

STATE OF TENNESSEE DEPARTMENT OF TRANSPORTATION

STRATEGIC TRANSPORTATION INVESTMENTS DIVISION

SUITE 1000, JAMES K. POLK BUILDING 505 DEADERICK STREET NASHVILLE, TN 37243 (615) 741-2208

JOHN C. SCHROER COMMISSIONER BILL HASLAM

MEMORANDUM

TO:

Mr. Paul Degges, Deputy Commissioner and Chief Engineer

FROM:

Mr. Steve Allen, Strategic Transportation Investments Director

DATE:

December 5, 2013

SUBJECT:

Road Safety Audit Review (RSAR), State Route 2 (Cummings Highway)

From Interstate 24 WB Ramps (Log Mile 2.09) to State Route 38

(Birmingham Highway, Log Mile 2.32) Hamilton County, PIN 118452.00

This project was identified through the Tennessee Department of Transportation (TDOT) safety needs planning process. The section of State Route (SR) 2 from log mile 2.09 to log mile 2.32 is a four (4) lane divide urban minor arterial, with twelve (12) feet lane widths and shoulder widths varying from two (2) to ten (10) feet. SR 2 appears on the Highway Safety Improvement Program (HSIP) list and qualifies for Hazard Elimination Safety Program (HESP) funds, because the crash ratio (actual crash rate divided by critical crash rate) was 5.88 which is greater than 3.50, the current minimum threshold.

The total estimated cost of improvements listed in the report is \$2,551,000. Right-of-way acquisition is required. A lighting maintenance agreement is required (City of Chattanooga). No local match is required. These improvements will be let to contract.

If you should need any further information, please contact me at (615) 741-2208 or e-mail me at Steve.Allen@tn.gov.

SA/cb

Attachment

CC: Adetokunbo Omishakin, Jim Moore, Ray Rucker, Brad Freeze, Jerry Hatcher, Brian Hurst, Alan Wolfe, Karen Rennich (Chattanooga TPO), file

ROAD SAFETY AUDIT REPORT

STATE ROUTE 2 (CUMMINGS HIGHWAY)
From Interstate 24 WB Ramps LM 2.09 to
State Route 38 (Birmingham Highway, Log Mile 2.32)
HAMILTON COUNTY
PIN 118452.00



PREPARED BY

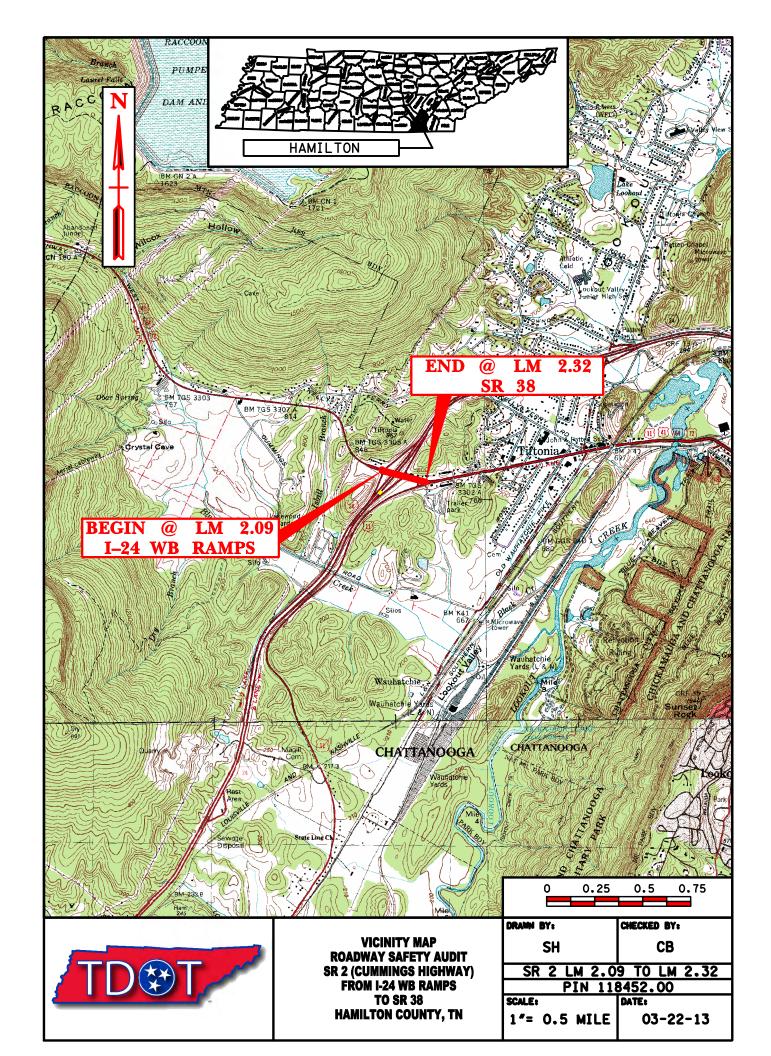
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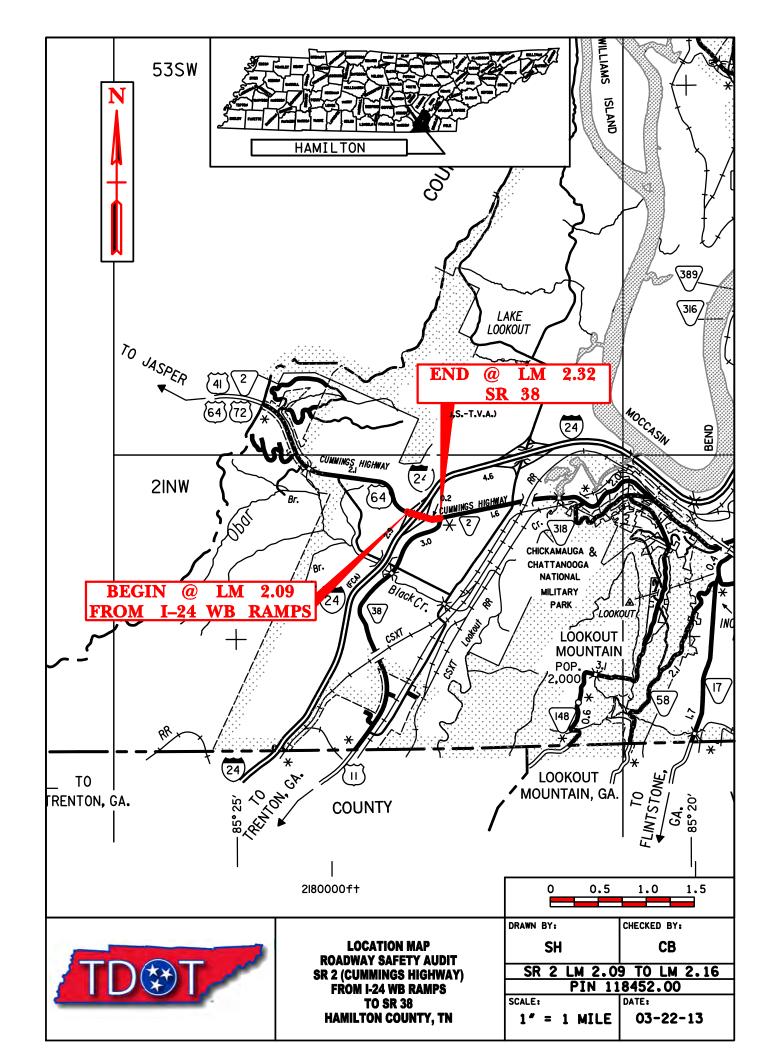
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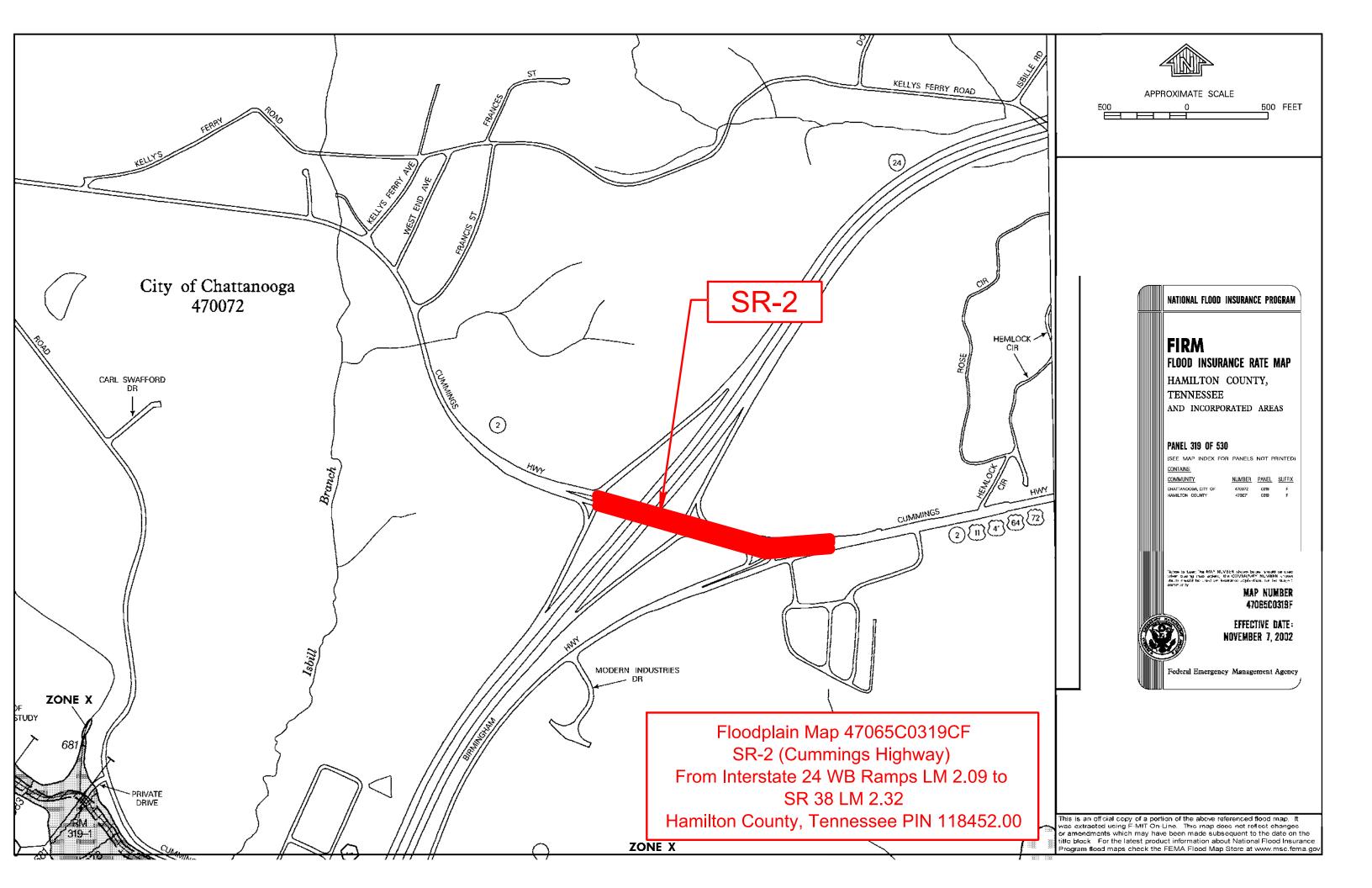
TENNESSEE DEPARTMENT OF TRANSPORTATION

STRATEGIC TRANSPORTATION INVESTMENTS DIVISION

Approved by:	Signature	DATE
DIRECTOR Strategic Transportation Investments Division	Sue Ola	12-9-13







Hamilton County

SR 2 (Cummings Highway)

From Interstate 24 WB Ramps LM 2.09 to SR 38 (Birmingham Highway, LM 2.32) 118452.00

Road Safety Audit Review

Date of Review: March 28, 2013

Project Description and Background

This project was identified through the Tennessee Department of Transportation (TDOT) safety needs planning process. The section of State Route (SR) 2 from log mile 2.09 to log mile 2.32 is a four (4) lane divided urban minor arterial, with twelve (12) feet lane widths and shoulder widths varying from two (2) to ten (10) feet. SR 2 appears on the Highway Safety Improvement Program (HSIP) list and qualifies for Hazard Elimination Safety Program (HESP) funds, because the crash ratio (actual crash rate divided by critical crash rate) was 5.88 which is greater than 3.50, the current minimum threshold.

The RSAR team members are provided below.

RSAR Team Members

Name	Organization	Title	Phone #	Email
Glenda Tyus	TDOT-Strategic Transportation Investments Div.	Transportation Planner 4	615-741- 1816	Glenda.Tyus@tn.gov
David Duncan	TDOT- Strategic Transportation Investments Div.	Roadway Specialist 2	615-532- 6131	<u>David.A.Duncan@tn.gov</u>
Mike Gilbert	TDOT- Strategic Transportation Investments Div.	Roadway Specialist 2	615-741- 0772	Michael.Gilbert@tn.gov
Terrance Hill	TDOT-UT	Special Projects Coordinator	615-532- 5824	Terrance.Hill@tn.gov
Alan Wolfe	TDOT-Traffic Region 2	Regional Traffic Manager	423-510- 1139	Alan.Wolfe@tn.gov
Landon Castleberry	TDOT-Traffic Region 2	Ops. Specialist 2	423-510- 1208	Landon.T.Castleberry@tn.gov
Amber Thornton	TDOT-Project Management Region 2	Transportation Project Manager 2	423-510- 1225	Amber.Thornton@tn.gov
Karen Anderson	ARCADIS	Project Engineer	423-756- 7193	<u>Karen.Anderson@arcadis-us.com</u>
Harvey McKaig	ARCADIS	Transportation Specialist	423-756- 7193	Harvey.McKaig@arcadis- us.com
Jason Yakimowich	City of Chattanooga	Traffic Engineer	423-643- 5950	Yakimowich J@chattanooga. gov

Hamilton County

SR 2 (Cummings Highway)

From Interstate 24 WB Ramps LM 2.09 to SR 38 (Birmingham Highway, LM 2.32) 118452.00

Information Used in the Review

- TDOT Hamilton County Highway Map
- United States Geological Survey (USGS) Map
- Aerial Photography
- TRIMS Traffic Report
- TRIMS Geometric Report
- TRIMS Road Segment Report
- TRIMS Route Feature Description Listing
- TRIMS Crash Data (2007, 2008, and 2009)
- TRIMS Crash Summary Report
- Crash Reports
- Crash Summary Sheets

Pre-Brief Summary

The RSAR pre-brief meeting was held on Tuesday, March 26, 2013 at 9:00 AM CDT via teleconference.

An overview of the crash data observations and findings for the section of State Route 2 were reviewed with the team members. From January 01, 2007 to December 31, 2009 a total of fifty-three (53) crashes occurred on this section with thirty-seven (37) property damage crashes, twelve (12) non-incapacitating injury crashes and four (4) incapacitating injury crashes.

Discussions among pre-brief meeting attendees included:

• The Waffle House parking lot was selected as the meeting location. It is located in the northwest corner of the intersection of SR 2 and Interstate 24 WB Ramp. Meeting time was set at 9:00 AM CDT.

Observations

The following observations were made utilizing existing data and information collected during the on-site visit conducted on Thursday, March 28, 2013:

- Following the field review, the project limits were extended to include the intersections of I-24 EB Ramps at SR 2 and SR 38 (Birmingham Highway) at SR 2. This will provide a coordinated review of the control at all three intersections.
- SR 2 is a four (4) lane divided urban minor arterial route with an asphalt pavement surface.
- The pavement is in good condition.
- A heavy left turning truck volume was observed travelling eastbound from the Interstate 24 WB ramp.
- Centerline raised pavement markers were present along SR 2.
- SR 2 has edge line rumble stripes.

Hamilton County

SR 2 (Cummings Highway)

From Interstate 24 WB Ramps LM 2.09 to SR 38 (Birmingham Highway, LM 2.32) 118452.00

- Shoulder widths vary from two (2) to ten (10) feet. Under Interstate 24 only, five (5) feet sidewalks are present on both sides of SR 2.
- TRIMS indicated a 2011 Annual Average Daily Traffic (AADT) volume of 3,540 for this section and 11,010 for SR 2 east of Interstate 24.
- TDOT provided Turning Movement Counts (TMC) at the Interstate 24 WB intersection for Signal Warrant Analysis and Roundabout Analysis.
- The Interstate 24 WB intersection does not meet Signal Warrant. (Results provided in the appendix.)
- Peak hour traffic counts were collected on Thursday October 10, 2013 along State Route 2 at the intersections of Interstate 24 eastbound ramps, westbound ramps and State Route 38 (Birmingham Highway). Counts are provided in the Appendix.
- Analysis of the existing geometrics with existing traffic volumes indicate that the stop controlled intersection of SR 2 and the Interstate 24 westbound ramps operate at an LOS E in the AM peak hour and an LOS F in the PM peak hour. The stop controlled intersection of SR 2 and the Interstate 24 eastbound ramps operate at an LOS B in the AM and PM peak hours. Analysis of the existing geometrics and existing traffic volumes at the signalized intersection of SR 38 and SR 2 operate at an LOS B for both the AM peak hour and a LOS C in PM peak hour.
- Analysis of the proposed geometrics with existing traffic volumes indicate that the two (2) Interstate 24 ramp intersection roundabouts will operate at an LOS A during both the AM and PM peak hours. Analysis of the proposed geometrics and existing traffic volumes at the SR 38 and SR 2 signalized intersection indicate that it will operate at an LOS B in the AM peak hour and an LOS C in the PM peak hour. The maximum expected northbound queue along SR 38 at SR 2 is 111 feet. The storage between the two intersections is approximately 400 feet. Therefore, the SR 38 at SR 2 signal queue will not impact the roundabout operations. (Traffic Counts and Results are provided in the appendix.)

Construction of the roundabout at the Interstate 24 westbound ramps will improve the operations from an LOS E to an LOS A in the AM peak and from an LOS F to an LOS A in the PM peak. The Interstate 24 eastbound roundabout will improve the operations from an LOS B to an LOS A for both the AM and PM peaks. LOS at the signalized intersection of SR 38 and SR 2 will remain an LOS B for both the AM and PM peaks.

Recommended improvements and guidance are on the following aerial figures.

The total estimated cost of improvements listed in the report is \$2,551,000. Right-of-way acquisition is required. A lighting maintenance agreement is required (City of Chattanooga). No local match is required. These improvements will be let to contract.



SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County

Date Photos Taken: March 28, 2013 October 30, 2013

Photograph 1

Taken: March 28, 2013 View looking west along SR 2 near Interstate 24 WB Off Ramp



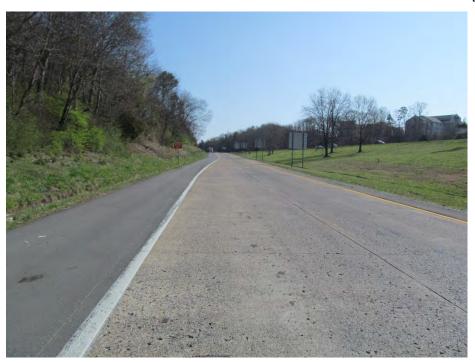
Photograph 2

Taken: March 28, 2013 View looking east along SR 2 from the Interstate 24 WB Off Ramp

SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County Date Photos Taken: March 28, 2013 October 30, 2013



Taken: March 28, 2013 View looking north along Interstate 24 WB Off Ramp



Photograph 4

Taken: March 28, 2013 View looking south along Interstate 24 WB Off Ramp toward SR 2

SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County Date Photos Taken: March 28, 2013

October 30, 2013



Taken: March 28, 2013 View looking east along SR 2 toward Interstate 24 WB Ramps.





Photograph 6

Taken: March 28, 2013 View looking west along SR 2 toward Interstate 24

SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County Date Photos Taken: March 28, 2013 October 30, 2013



Taken: October 30, 2013 View looking north from Interstate 24 EB off ramp.



Photograph 8

Taken: October 30, 2013 View looking south at I-24 EB off ramp.

SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County Date Photos Taken: March 28, 2013

October 30, 2013



Taken: October 30, 2013 View looking north at I-24

EB on ramp.



Photograph 10

Taken: October 30, 2013 View looking west at the SR 2 and SR 38 intersection.

SR 2 (Cummings Highway) From Interstate 24 WB Ramp (LM 2.09) To SR 38 (Birmingham Highway LM 2.32) Hamilton County Date Photos Taken: March 28, 2013

October 30, 2013



Photograph 11

Taken: October 30, 2013 View looking south along SR 38 (Birmingham Hwy).



Photograph 12

Taken: October 30, 2013 View looking north along SR 38 at the SR 2 intersection. Route: SR 2 (Cummings Highway)

Description: From Interstate 24 WB Ramps (LM 2.09) to SR 38 (LM 2.32)

County: Hamilton County

Length: 0.23 Miles

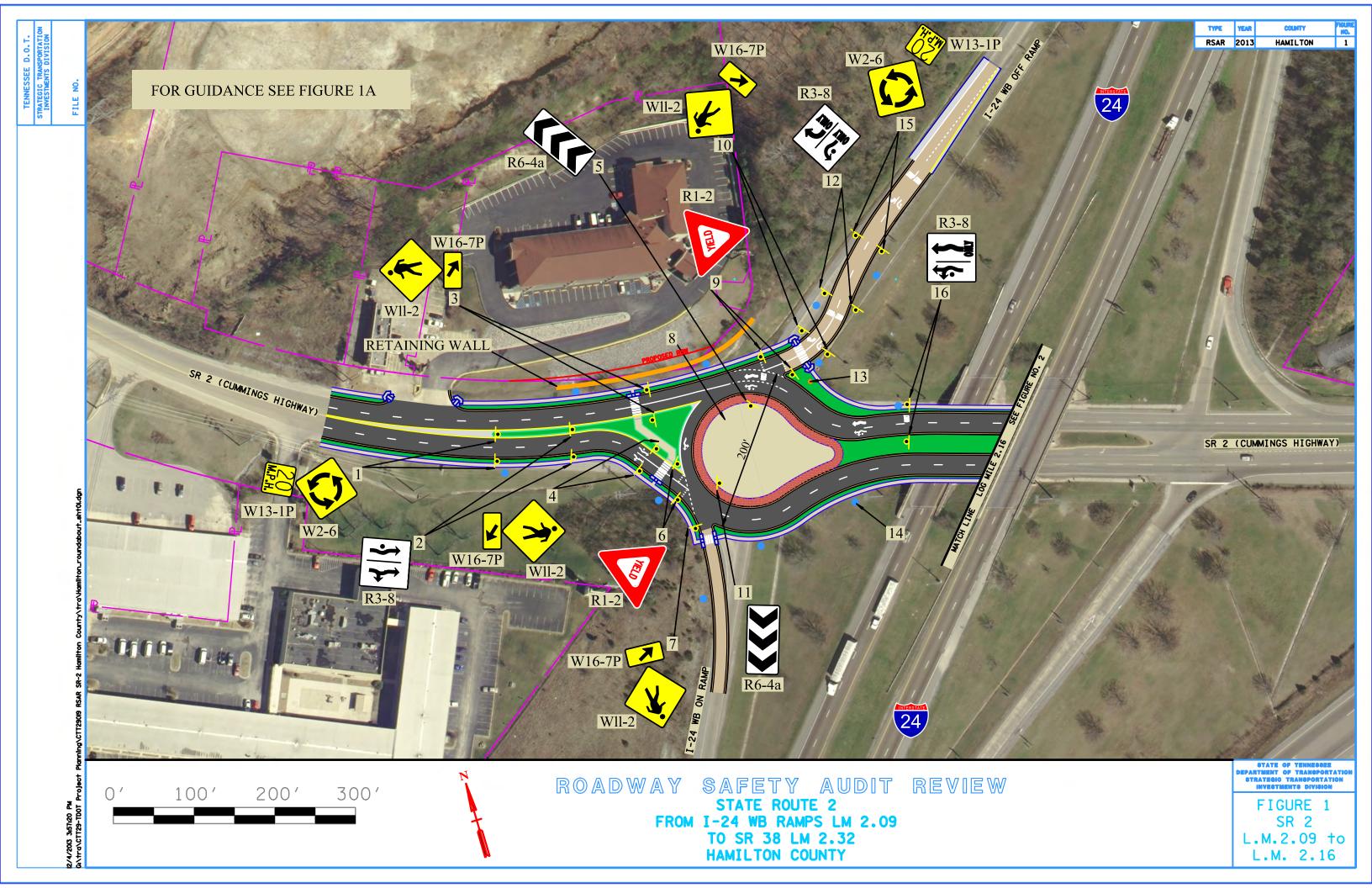
Date: December 5, 2013

<u>DESCRIPTION</u>	LOCAL	STATE	<u>FEDERAL</u>	TOTAL
Right-of-Way	\$ -	\$ -	\$ 40,000	\$ 40,000
Clearing and Grubbing	\$ -	\$ -	\$ 10,000	\$ 10,000
Earthwork	\$ -	\$ -	\$ 375,000	\$ 375,000
Railroad Crossing or Separation	\$ -	\$ -	\$ -	\$ -
Drainage	\$ -	\$ -	\$ 110,000	\$ 110,000
Utilities	\$ -	\$ -	\$ 60,000	\$ 60,000
Structures	\$ -	\$ -	\$ -	\$ -
Pavement Removal	\$ -	\$ -	\$ 54,400	\$ 54,400
Paving	\$ -	\$ -	\$ 548,000	\$ 548,000
Roadway and Pavement Appurtenances	\$ -	\$ -	\$ 189,700	\$ 189,700
Retaining Walls	\$ -	\$ -	\$ 200,300	\$ 200,300
Topsoil	\$ -	\$ -	\$ -	\$ -
Seeding	\$ -	\$ -	\$ -	\$ -
Sodding	\$ -	\$ -	\$ 24,400	\$ 24,400
Rip-Rap or Slope Protection	\$ -	\$ -	\$ -	\$ -
Fencing	\$ -	\$ -	\$ -	\$ -
Signing ¹	\$ -	\$ -	\$ 15,200	\$ 15,200
Pavement Markings ¹	\$ -	\$ -	\$ 18,900	\$ 18,900
Lighting ¹	\$ -	\$ -	\$ 70,000	\$ 70,000
Signalization ¹	\$ -	\$ -	\$ -	\$ -
Guardrail ¹	\$ -	\$ -	\$ 3,500	\$ 3,500
Pay Item Quantity Adjustment (15%) ²	\$ -	\$ -	\$ 257,900	\$ 257,900
Maintenance of Traffic	\$ -	\$ -	\$ 30,000	\$ 30,000
Mobilization (5%)	\$ -	\$ -	\$ 100,400	\$ 100,400
CONSTRUCTION COST (rounded)	\$ -	\$ -	\$ 2,107,700	\$ 2,107,700
Engineering and Contingency (10%)	\$ -	\$ -	\$ 210,800	\$ 210,800
TOTAL CONSTRUCTION COST (rounded)	\$ -	\$ -	\$ 2,319,000	\$ 2,319,000
Preliminary Engineering (10%)	\$ -	\$ -	\$ 232,000	\$ 232,000
PROJECT COST ³ (rounded)	\$ -	\$ -	\$2,551,000	\$2,551,000

¹ This safety item is 100% eligible and does not require a 10% funding match by the local agency.

² For estimating purposes pay items are adjusted for fluxuation of cost based on quantity.

³ For estimating future project costs, a compounded inflation rate of 10% should be applied from the date of this estimate.



TENNESSEE D.O.T.
STRATEGIC TRANSPORTATION
INVESTMENTS DIVISION

FILE

TYPE YEAR COUNTY 100.

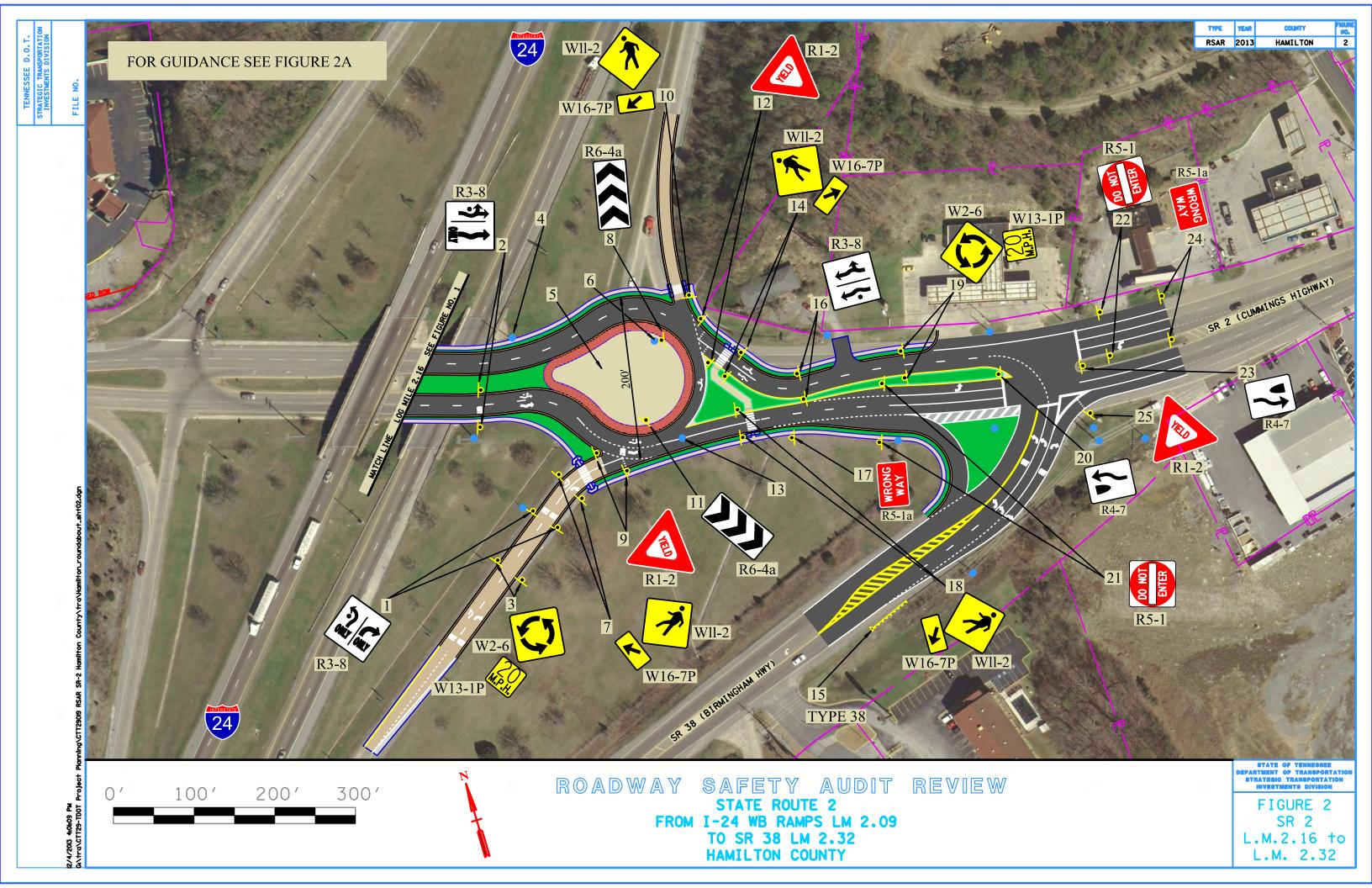
RSAR 2013 HAMILTON 1A

GUIDANCE

- 1. Install one (1) Roundabout Warning (W2-6) (36"x36") sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque and one (1) supplemental Roundabout Warning (W2-6) sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque, southbound on SR 2 approximately 300 feet north of Interstate 24 WB Ramps.
- 2. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, southbound on SR 2 approximately 200 feet north of Interstate 24 WB Ramps.
- 3. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, northbound on SR 2 approximately 20 feet south of pedestrian crosswalk.
- 4. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, southbound on SR 2 approximately 20 feet north of pedestrian crosswalk.
- 5. Install one (1) Roundabout Directional Chevron (R6-4a) (48"x24") sign, inside roundabout as shown facing the Interstate 24 WB Off Ramp approach at SR 2.
- 6. Install one (1) Yield (R1-2) (48"x48"x48") sign and one (1) supplemental Yield (R1-2) sign, southbound on SR 2 at Interstate 24 WB Ramps. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 7. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, westbound on I-24 WB on ramp approximately 20 feet east of pedestrian crosswalk.
- 8. Improve the intersection of SR 2 and Interstate 24 WB Ramps to provide a two (2) lane roundabout, curb and gutter, 5' sidewalks and handicap ramps in accordance with TDOT Standard Drawing RD01-TS-10. The intersection shall have all pavement markings and appropriate signing laid out according to TDOT design guidelines. All striping shall be Enhanced Flatline Thermoplastic.
- 9. Install one (1) Yield (R1-2) (48"x48"x48") sign and one (1) supplemental Yield (R1-2) sign, westbound on Interstate 24 WB Off Ramp at SR 2. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 10. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, westbound on Interstate 24 WB Off Ramp approximately 20 feet east of pedestrian crosswalk.
- 11. Install one (1) Roundabout Directional Chevron (R6-4a) (48"x24") sign, inside roundabout as shown facing the SR 2 southbound approach at Interstate 24 WB Ramps.
- 12. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, westbound on Interstate 24 WB Off Ramp approximately 200 feet east of SR 2.
- 13. Remove and relocate one (1) utility pole.
- 14. Install roadway lighting according to the TDOT design guidelines (Including a lighting plan).
- 15. Install one (1) Roundabout Warning (W2-6) (36"x36") sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque and one(1) supplemental Roundabout Warning (W2-6) sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque, westbound on Interstate 24 WB Off Ramp approximately 300 feet east of SR 2.
- 16. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, northbound on SR 2 approximately 200 feet south of Interstate 24 WB Ramps.

ROADWAY SAFETY AUDIT REVIEW

FROM I-24 WB RAMPS LM 2.09
TO SR 38 LM 2.32
HAMILTON COUNTY



Mamning/CTT29019 RSAR SR-2 Hamilton County/tra/Hamilton_roundabout_sht02A.dgn

GUIDANCE

- 1. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, eastbound on Interstate 24 EB Off Ramp approximately 200 feet west of SR 2.
- 2. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, southbound on SR 2 approximately 200 feet north of Interstate 24 EB Ramps.
- 3. Install one (1) Roundabout Warning (W2-6) (36"x36") sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque and one(1) supplemental Roundabout Warning (W2-6) sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque, eastbound on Interstate 24 Off Ramp approximately 300 feet west of SR 2.
- 4. Remove and relocate one (1) utility pole.
- 5. Improve the intersection of SR 2 and Interstate 24 EB Ramps to provide a two (2) lane roundabout, curb and gutter, 5' sidewalks and handicap ramps in accordance with TDOT Standard Drawing RD01-TS-10. The intersection shall have all pavement markings and appropriate signing laid out according to TDOT design guidelines. All striping shall be Enhanced Flatline Thermoplastic.
- 6. Remove and relocate one (1) utility pole.
- 7. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, eastbound on Interstate 24 EB Off Ramp approximately 20 feet west of pedestrian crosswalk.
- 8. Install one (1) Roundabout Directional Chevron (R6-4a) (48"x24") sign, inside roundabout as shown facing the Interstate 24 EB Off Ramp approach at SR 2.
- 9. Install one (1) Yield (R1-2) (48"x48"x48") sign and one (1) supplemental Yield (R1-2) sign, eastbound on Interstate 24 EB Off Ramp at SR 2. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 10. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, northbound on SR 2 approximately 20 feet south of pedestrian crosswalk.
- 11. Install one (1) Roundabout Directional Chevron (R6-4a) (48"x24") sign, inside roundabout as shown facing the Interstate 24 EB Off Ramp approach at SR 2.
- 12. Install one (1) Yield (R1-2) (48"x48"x48") sign and one (1) supplemental Yield (R1-2) sign, northbound on SR 2 at Interstate 24 EB Ramps. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 13. Remove and relocate one (1) utility pole.
- 14. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, northbound on SR 2 approximately 20 feet south of pedestrian crosswalk.
- 15. Remove existing guardrail end terminal and install one (1) Type 38 guardrail end terminal on SR 38 eastbound approximately 400 feet west of SR 2.
- 16. Install one (1) Advanced Intersection Lane Control (R3-8) (48"x36") sign and one (1) supplemental Advanced Intersection Lane Control (R3-8) sign, northbound on SR 2 approximately 200 feet south of Interstate 24 EB Ramps.
- 17. Install one (1) Wrong Way (R5-1a) (36"x24") sign , northbound on SR 2 approximately 300 feet north of SR 38. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 18. Install one (1) Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque and one (1) supplemental Pedestrians (W11-2) (36"x36") sign with one (1) Downward Diagonal Arrow (W16-7P) plaque, southbound on SR 2 approximately 20 feet north of pedestrian crosswalk.

TYPE	YEAR	COUNTY	FIGUR NO.
RSAR	2013	HAMILTON	2A

- 19. Install one (1) Roundabout Warning (W2-6) (36"x36") sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque and one (1) supplemental Roundabout Warning (W2-6) sign with one (1) 20 MPH Speed Advisory (W13-1P) Plaque, northbound on SR 2 approximately 300 feet south of Interstate 24 EB Ramps.
- 20. Install one (1) Keep Right (R4-7) (24"x30") sign , northbound on SR 2 in the center island at the intersection of SR 38.
- 21. Install one (1) Do Not Enter (R5-1) (30"x30") sign one (1) supplemental Do Not Enter (R5-1) (30"x30") sign, southbound on SR 2 approximately 200 feet north of SR 38. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 22. Install one (1) Do Not Enter (R5-1) (30"x30") sign one (1) supplemental Do Not Enter (R5-1) (30"x30") sign, northbound on SR 2 approximately 100 feet south of SR 38. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 23. Install one (1) Keep Right (R4-7) (24"x30") sign , southbound on SR 2. in the center island at the intersection of SR 38.
- 24. Install one (1) Wrong Way (R5-1a) (36"x24") sign one (1) supplemental Wrong Way (R5-1a) (36"x24") sign, southbound on SR 2 approximately 300 feet south of SR 38. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.
- 25. Install one (1) Yield (R1-2) (48"x48"x48") sign ,northbound on SR 38 at SR 2. Install 2" min. width red reflective strip on sign posts. See Detail 1, Figure 3.

ROADWAY SAFETY AUDIT REVIEW

STATE ROUTE 2
FROM I-24 WB RAMPS LM 2.09
TO SR 38 LM 2.32
HAMILTON COUNTY

ROADWAY SAFETY AUDIT REVIEW

STATE ROUTE 2
FROM I-24 WB RAMPS LM 2.09
TO SR 38 LM 2.32
HAMILTON COUNTY

Hamilton County SR 2 (Cummings Highway) From Interstate 24 WB Ramps LM 2.09 to SR 38 (Birmingham Highway, LM 2.32) 118452.00

APPENDIX

TDOT PAY ITEM	TDOT DESCRIPTION	UNIT	QUANTITY	UNIT CO	ST '	TOT	TAL COST
-	Right-of-Way	LS	LS	\$ 40,000	.00	\$	40,000
		RIGHT	-OF-WAY TOTA	L (ROUNDE	D)	\$	40,000
201-01	Clearing and Grubbing	LS	LS	\$ 10,000	.00	\$	10,000
	CLE	AR AND G	RUBBING TOTA	L (ROUNDE		\$	10,000
203-01	Road & Drainage Excavation (Unclassified)		15000	\$ 5	.00	\$	75,000
203-03	Borrow Excavation (Unclassified)	CY	20000			\$	300,000
		EAR	THWORK TOTA	L (ROUNDE	-	\$	375,000
202-03.01	Removal of Asphalt Pavement	SY	7,800			\$	39,000
415-01.02	Cold Planning Bituminous Pavement	SY VEMENT D	4,400			\$	15,400
207.00.00			REMOVAL TOTA	<u> </u>		\$	54,400
607-03.02 611-12.02	18" Conc. Pipe Culvert (Class III) Catch Basin, Type 12, 4'-8' Depth	LF EA	2000 12	\$ 40 \$ 2,500		\$ \$	80,000 30,000
011-12.02	Catch Dasin, Type 12, 4-0 Depth		RAINAGE TOTA			\$	110,000
_	Utility Pole Relocation	LS	4	\$ 15,000		\$	60,000
	Other Followship		JTILITIES TOTA			\$	60,000
						-	00,000
			JCTURES TOTA			\$	•
	RAILROAD CROSSIN					\$	-
303-01	Mineral Aggregate, TY A Base, Grading D	TON	4994.7	-		\$	99,894
307-01.01	Asp. Conc. Mix(PG64-22) (BPMB-HM) Gr. A	TON	918.0	•		\$	59,671
307-01.08 313-03	Asphalt Conc Mx(PG64-22)(BPMB-HM)Gr B-M2 Treated Permeable Base	TON SY	867.8 1736.8	•		\$ \$	60,744 31,262
402-01	Bituminous Material for Prime Coat (PC)	TON	10.7	\$ 18 \$ 400		Φ \$	4,275
402-02	Aggregate for Cover Material (PC)	TON	42.3			\$	1,354
411-01.07	ACS Mix (PG64-22) Grading E Shoulder	TON	410.8			\$	32,862
411-01.10	ACS Mix(PG64-22) Grading D	TON	643.2			\$	51,452
415-01.02	Cold Planing Bituminous Pavement	SY	4,391			\$	15,369
501-01.03	Portland Cem Concrete Pvmt (Plain) 10"	SY	1736.8	•		\$	107,680
604-03.07	Class A Concrete	CY	190.0	\$ 400		\$	76,000
701-03 411-12.01	Concrete Median Pavement Scoring shoulders (cont. 16 IN)	CY L.M.	13.8 0.2	\$ 500 \$ 500		\$ \$	6,907 100
411-12.01	Scotting shoulders (cont. 10 my)	L.IVI.	PAVING TOTA			\$	548,000
701-01.01	Concrete Sidewalk (4")	SF	16180	•		\$	56,630
701-01.01	Concrete Handicap Ramp	SF	800		.00	\$	12,000
702-03	Concrete Combined Curb & Gutter	CY	440.00		.00		121,000
	ROADWAY AND PAVEMENT	T APPURTE	ENANCES TOTA	L (ROUNDE	D)	\$	189,700
604-07.01	Retaining Wall	SF	1335	\$ 150		\$	200,250
			G WALLS TOTA			\$	200,300
712-01	Traffic Control	LS		\$ 30,000		\$	30,000
	MAINTEI	NANCE OF	TRAFFIC TOTA	L (ROUNDE	D)	\$	30,000
			TOPSOIL TOTA	L (ROUNDE	D)	\$	-
			SEEDING TOTA	L (ROUNDE	D)	\$	-
803-01	Sodding (New Sod)	SY	6,100	\$ 4	.00	\$	24,400
			SODDING TOTA	L (ROUNDE	D)	\$	24,400
713-11.01	"U" Section Steel Posts	LB	1,026			\$	5,130
713-02.21	Sign Post Delineation Enhancement	LF	62		.00	\$	310
713-13.04 713-13.14	Flat Sheet Aluminum Signs (0.100" Thick) Florescent Yellow Sign Sheeting	SF SF	483 244			\$ \$	7,245 1,244
713-15.14	Removal & Relocation of Sign & Support	EA	16			\$	1,244
			SIGNING TOTA			\$	15,200
				,	•		,

TDOT PAY ITEM	TDOT DESCRIPTION	UNIT	QUANTITY	U	NIT COST	тот	AL COST
716-02.06	Plastic Pavement Marking (Turn Lane Arrow)	EA	10	\$	120.00	\$	1,200
716-02.08	Plastic Pavement Marking (8" Dotted Line)	LF	1,200	\$	2.50	\$	3,000
716-02.09	Plastic Pavement Marking (Longitudinal Cross-Walk)	LF	140	\$	20.00	\$	2,800
716-03.01	Plastic Word Pavement Marking (Only)	EA	6	\$	135.00	\$	810
716-04.01	Plastic Pavement Marking (Straight-Turn Arrow)	EA	10	\$	200.00	\$	2,000
716-04.05	Plastic Pavement Marking (Straight Arrow)	EA	8	\$	110.00	\$	880
716-04.12	Plastic Pavement Marking (Yield Line)	SF	216	\$	10.00	\$	2,160
716-12.01	Enhanced Flatline Thermo Pvmt Mrkng (4" Line)	LM	2	\$	3,000.00	\$	6,000
	PAVE	MENT MA	RKINGS TOTA	L (R	OUNDED)	\$	18,900
714-01.36	Roadway Lighting	LS	LS	\$	70,000.00	\$	70,000
		L	IGHTING TOTA	L (R	OUNDED)	\$	70,000
		SIGNAL	IZATION TOTA	L (R	OUNDED)	\$	-
			FENCE TOTA	L (R	OUNDED)	\$	-
705-04.07	Guardrail Terminal (Type 38)	EA	1	\$	3,350.00	\$	3,350
706-01	Guardrail Removed	LF	50	\$	3.00	\$	150
		GUA	ARDRAIL TOTA	L (R	OUNDED)	\$	3,500
	RIP-RAP OR SLO	PE PRO	TECTION TOTA	L (R	OUNDED)	\$	-

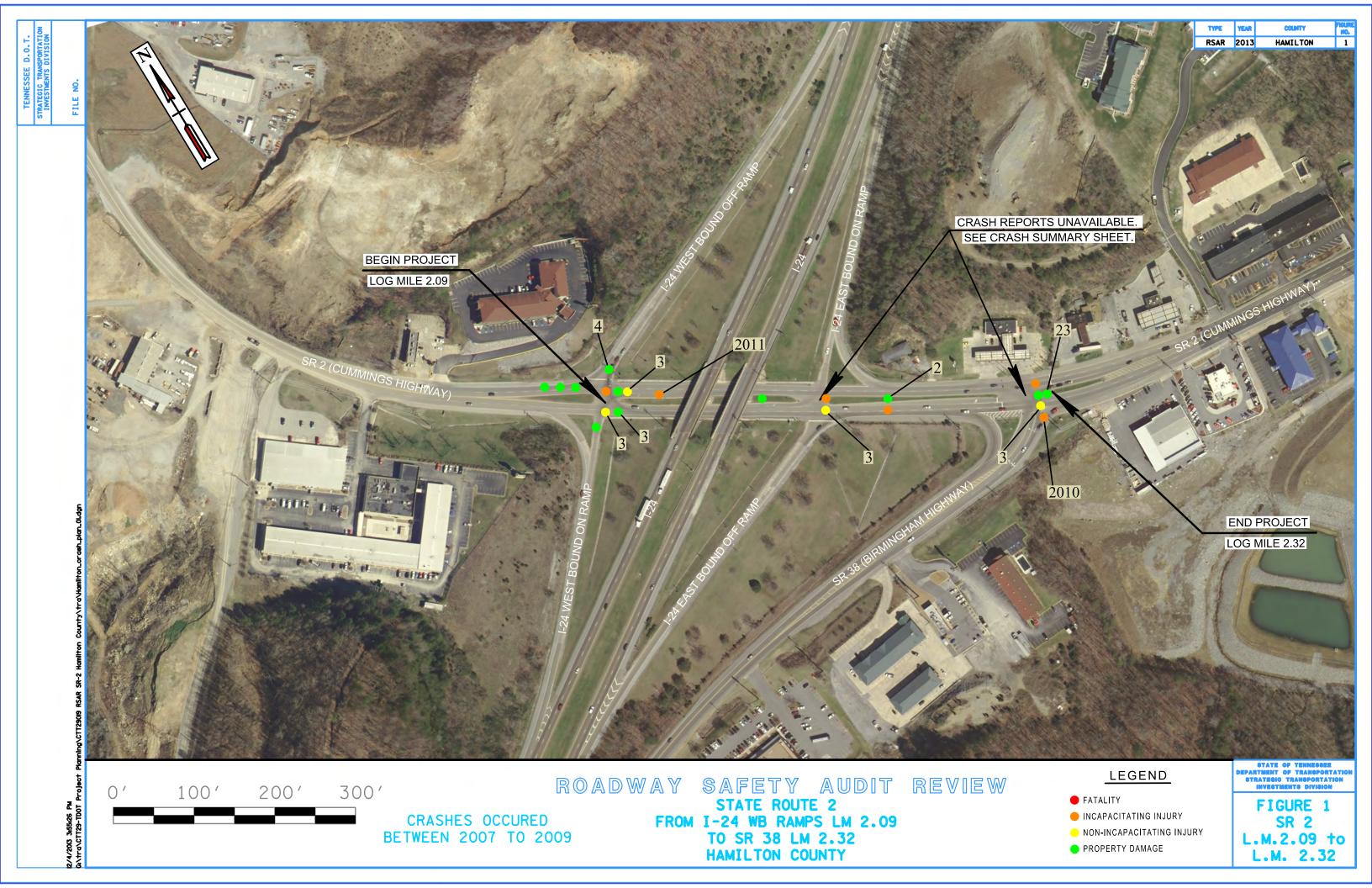
SR-2 (Cummings Highway) RSAR

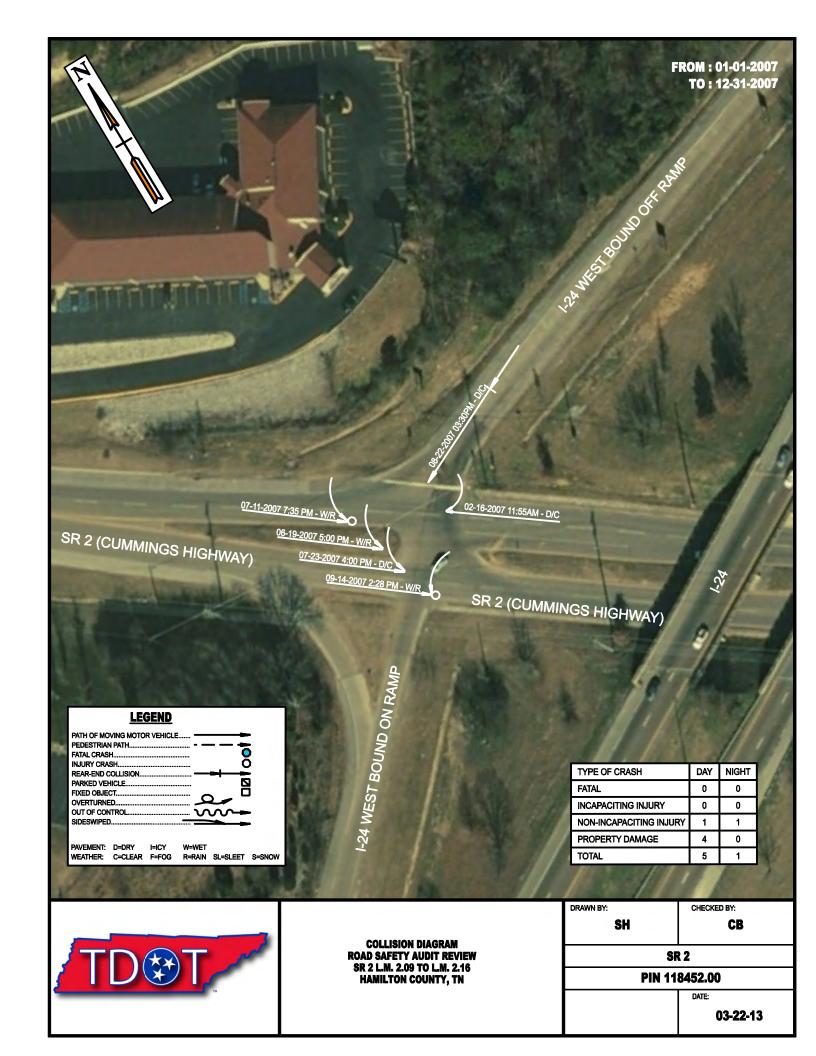
Designation	Legend or Description	No. of Signs	Size (in.)	Size (sq. ft.)	Total Sq.Ft.
R1-2	YIELD	9	48" x 48" x 48"	7.00	63.00
R3-8	ADVANCED INTERSECTION LANE CONTROL	12	48" x 36"	12.00	144.00
R6-4a	ROUNDABOUT DIRECTIONAL CHEVRON	4	48" x 24"	8.00	32.00
W2-6	ADVANCED ROUNDABOUT WARNING	8	36" x 36"	9.00	72.00
W11-2	ADVANCED PEDESTRIAN WARNING	14	36" x 36"	9.00	126.00
W13-1P	ADVISORY SPEED PLAQUE	8	18" x 18"	2.25	18.00
W16-7P	DOWNWARD DIAGONAL ARROW PLAQUE	14	24" x 12"	2.00	28.00
Totals		69			483.00

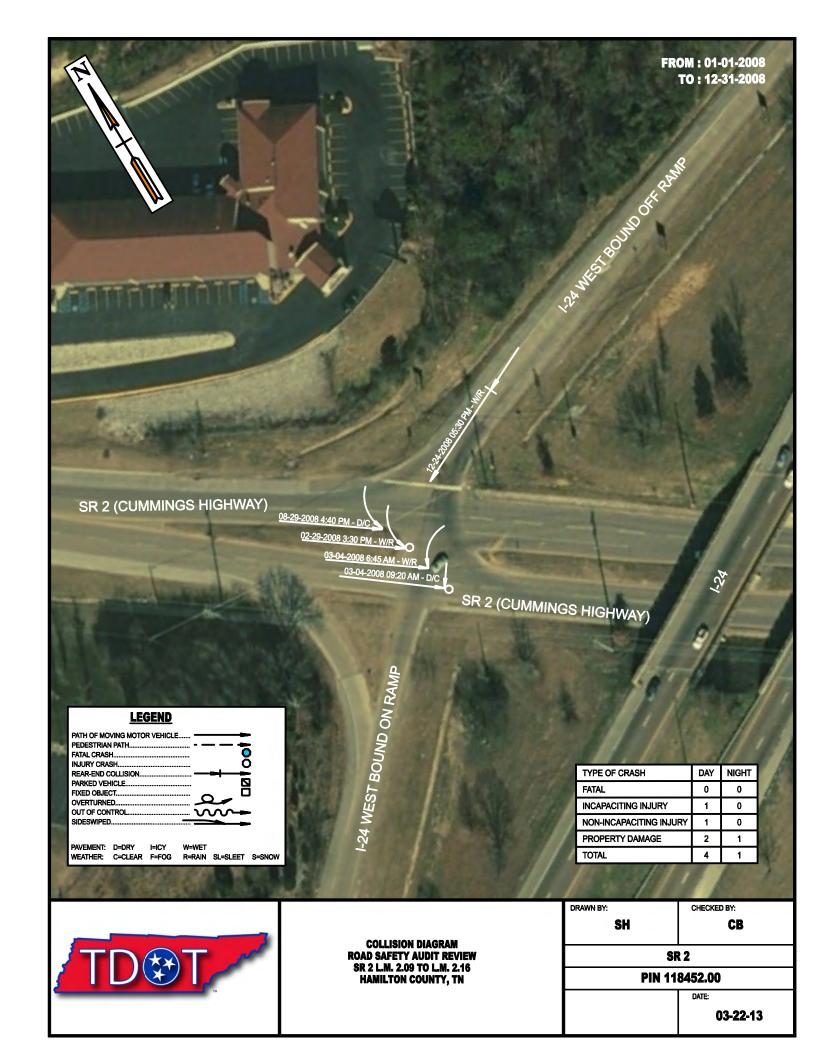
COUNTY = Hamilton 11/4/2013 Date: Route = SR 2 (Cummings Highway) Location = I-24 WB Ramps (L.M. 2.09) to SR 38 (L.M. 2.32) **Highway Type** = 4 LANE SECTION FUNCTIONAL CLASS= SR 2 (Cummings Highway) is a Urban Minor Arterial **DATA YEARS** = 2007 to 2009 TRIMS Crash Data ADT YEARS USED= 2011 TRIMS & ADAM COMMENTS = 4 Incapacitating Injury Crashes and no Fatal Crashes ANALYZED BY = SECTION = MORE THAN 0.10 MILE / SPOT = LESS THAN 0.10 MILE **ELM** VMT **BLM** Length Average AADT 2.09 2.16 0.07 3,540 248 2.16 2.32 0.16 1,762 11,010 0.00 0.00 0.00 0 0 0.00 0.00 0.00 0 0 0.00 0.00 0.00 0 0 0.00 0.00 0 0 0.00 0.00 0.00 0 0 0.00 0.23 8,737 2,009 INTERSECTION Leg Traffic AADT Log Mile North East South = 0 West 0 Entering AADT = 2011 Trims & Adam **4 Lane Section** 2007 To 2009 Trims Crash Data Other *Severe **Total Fatal** Injury Incap. Injury Crashes No. of Crashes **53** 0 12 No. of Years 3 SW avg. rate 0.426 1.777 0.009 0.048 0.057 08-10 S/W Rates 2.2003 Exposure (E) 24.088 5.454 Crash Rate (A) 0.000 1.818 1.818 4.095 Critical Rate (C) 0.3774 Severity Index (SI) **Actual Rate/SW Average** 13.56 0.00 37.87 31.89 12.80 Ratio of A/C 5.88 Severe Crashes are the sum of fatal and incapacitating injury crashes Revised 4/3/2007

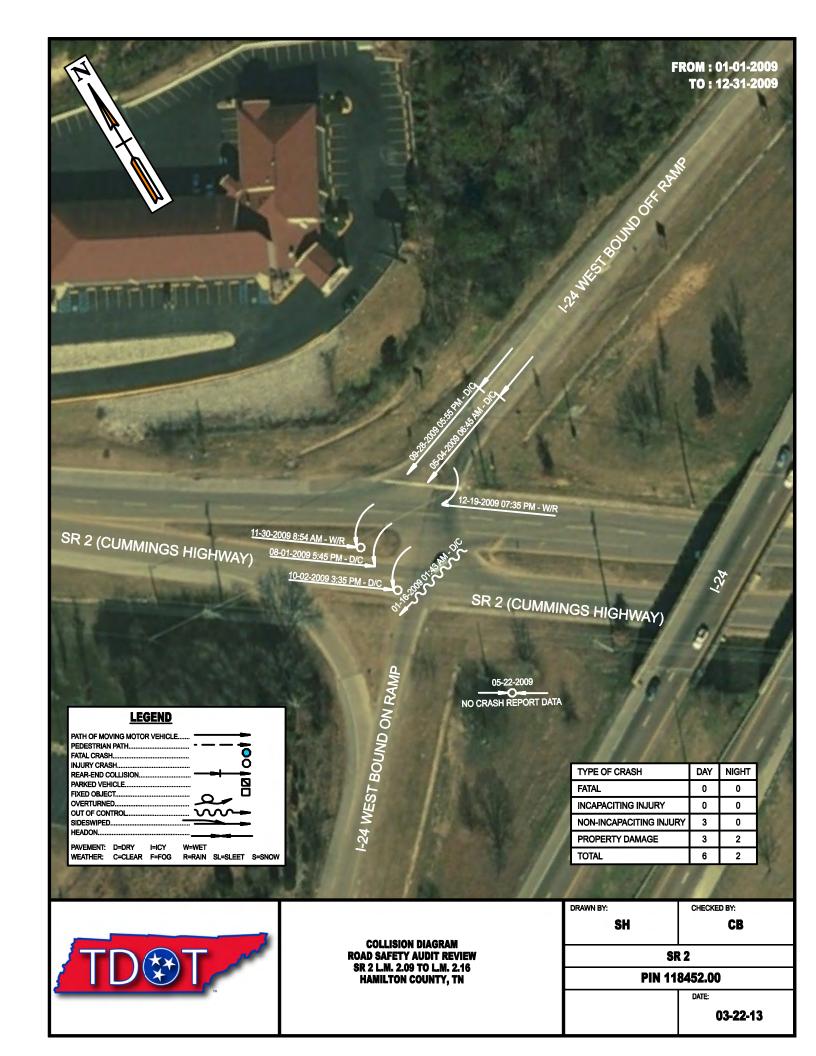
T.D.O.T. STRATEGIC TRANSPORTATION INVESTMENTS DIVISION (SAFETY PLANNING SECTION)

Cb









RSAR – SR 2 (CUMMINGS HIGHWAY) HAMILTON COUNTY CRASH REPORT SUMMARY (2007-2009) INCAPACITING INJURY CRASH (NO FATAL CRASHES)

L.M. 2.11 02/29/2008: Vehicle on I-24 Westbound Exit Ramp was attempting to turn left onto SR 2 (Cummings Highway) and was struck by a westbound vehicle on SR 2 (Cummings Highway). Left turning vehicle failed to yield right-of-way. Daylight, rain. One (1) incapacitating injury.

L.M. 2.21 07/07/2007: Angle crash. Daylight, clear. Two (2) incapacitating injuries. Crash report was not provided.

L.M. 2.24 05/27/2007: Angle crash. Daylight, clear. One (1) incapacitating injury. Crash report was not provided.

L.M. 2.32 11/27/2007: Head-on crash. Daylight, clear. Two (2) incapacitating injuries. Crash report was not provided.

CRASH SUMMARY SHEET RSAR: SR 2 (Cummings Hwy) at Interstate 24 Eastbound Ramps (LM 2.21)

1/1/07 to 12/31/09 Crash Data Observations

Crash Summary (1/1/07 to 12/31/09)

Total Number of Crashes = 8

Total Number of Fatal Crashes = 0

Total Number of Incapacitating Injury Crashes = 2

Overall Corridor Crash Types (2 or more Crashes)

- ✓ 75% (6) were Angle Crashes
- ✓ 25% (2) were Rear End Crashes

Overall Corridor Contributing Factors

- √ 13% (1) occurred at Dark-Lighted
- √ 87% (7) occurred during Daylight
- √ 13% (1) were due to Wet Conditions
- √ 87% (7) occurred in Dry Conditions

CRASH SUMMARY SHEET RSAR: SR 2 (Cummings Hwy) at SR 38 (Birmingham Hwy, LM 2.32)

1/1/07 to 12/31/09 Crash Data Observations

Crash Summary (1/1/07 to 12/31/09)

Total Number of Crashes = 26

Total Number of Fatal Crashes = 0

Total Number of Incapacitating Injury Crashes = 1

Overall Corridor Crash Types (2 or more Crashes)

- √ 38% (10) were Angle Crashes
- ✓ 23% (6) were Rear End Crashes
- √ 19% (5) were Sideswipe Crashes
- ✓ 12% (3) were No Collision w/ Vehicle
- √ 8% (2) were Head-On Crashes

Overall Corridor Contributing Factors

- √ 15% (4) occurred at Dark-Lighted
- √ 81% (21) occurred during Daylight
- √ 1% (1) occurred at Dawn
- √ 31% (8) were due to Wet Conditions
- √ 69% (18) occurred in Dry Conditions

One Incap Injury in 2010, Rear End, Wet Conditions

TRAFFIC SIGNAL WARRANT ANALYSIS - VOLUME WARRANTS

		Major	Street		Minor	Street	Warra Min Vehicu	ant 1A ılar Volume		ant 1B ont Traffic	Warra 80% of W	ant 1C	111	ant 1D Varrant 1B		rrant 2 our Volume		rrant 3 our Volume
Time	A	ctual Volun		Adjusted Total	Actual Volume	Adjusted Total	Percent o		Percent of		Percent o		Percent of		Warrant	Percent of	Warrant	Percent of
Beginning	App #1	App #2	Total	Volume		Volume	Major	Minor	Major 	Minor	Major 	Minor	Major 	Minor	Volume	Warrant	Volume	Warrant
6:00 am 7:00 8:00	110 232 181	106 147 163	216 379 344	216 379 344	70 112 59	70 112 59	51 90 82	50 80 42	34 60 55	100 160 84	45 79 72	44 70 37	30 53 48	88 140 74	0 310 0	***** 36 *****	0 0 0	*****
9:00 am 10:00 11:00	158 163 172	185 190 208	343 353 380	343 353 380	42 43 41	42 43 41	82 84 90	30 31 29	54 56 60	60 61 59	71 74 79	26 27 26	48 49 53	53 54 51	0 330 310	***** 13 13	0 0 0	*****
12:00 noon 1:00 2:00	197 196 152	254 340 251	451 536 403	451 536 403	65 69 45	65 69 45	107 128 96	46 49 32	72 85 64	93 99 64	94 112 84	41 43 28	63 74 56	81 86 56	260 210 290	25 33 16	400 340 0	16 20 *****
3:00 pm 4:00 5:00	234 188 231	503 383 608	737 571 839	737 571 839	100 90 94	100 90 94	175 136 200	71 64 67	117 91 133	143 129 134	154 119 175	63 56 59	102 79 117	125 113 118	120 190 90	83 47 104	240 320 200	42 28 47
6:00 pm 7:00 8:00			0 0 0	0 0 0		0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	**** **** ****	0 0 0	****
N Note:	lo adjustme	ent made.					Warrantin 420	g Volumes 140	Warrantin 630	g Volumes 70	Warrantin	g Volumes 160	Warrantin 720	ig Volumes 80		ng Volumes TCD Fig. 4-8	Warranti From MU	ng Volume
W a N	pproach vo lumber of h	lume ours showr	is the mi	oach exists u	ng the MUT(CD		rs Meeting		rs Meeting 2	Total Hou Warrant = Warrant Me	rs Meeting		irs Meeting	Total Ho Warrant = Warrant M	urs Meeting 1 et No	Total Ho Warrant = Warrant M	urs Meetir
		s. Additiona JTCD speci		utside of the ne levels.	count period	may		Warrar	nt 1 Met				No		-	ajor Street volun Minor Street war		t no
Comments:	(includo	any informa	tion which	n may be use	ful to the rev	iowor)												
Johnnerits.	,	geometrics.	uon wind	Tillay be use	ui to the rev	iewei)												

Intersection Intersection Delay, s/veh	17.8										
,											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL		NBR	SWL	SWF
Vol, veh/h	0	286	8	95	61	0	0		0	0	6
Conflicting Peds, #/hr	0	0	1	1	0	0	0		0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop		Stop	Stop	Sto
RT Channelized	-	-	Yeild	-	-	None	-		-	-	None
Storage Length	-	-	-	115	-	-	-		-	0	
Veh in Median Storage, #	-	0	-	-	0	-	0		-	0	
Grade, %	-	0	-	-	0	-	0		-	0	
Peak Hour Factor	92	91	50	74	80	92	92		92	92	60
Heavy Vehicles, %	2	2	2	11	7	2	2		2	2	Ĺ
Mvmt Flow	0	314	16	128	76	0	0		0	0	107
Major/Minor	Major1			Major2						Minor2	
Conflicting Flow All	76	0	0	314	0	0				490	39
Stage 1	-	-	-	-	-	-				333	
Stage 2	-	-	-	-	-	-				157	
Follow-up Headway	2.22	-	-	2.31	-	-				3.57	3.3
Pot Capacity-1 Maneuver	1521	-	-	1180	-	-				495	1014
Stage 1	-	-	-	-	-	-				684	
Stage 2	-	-	-	-	-	-				840	
Time blocked-Platoon, %		-	-		-	-					
Mov Capacity-1 Maneuver	1520	-	-	1179	-	-				441	1013
Mov Capacity-2 Maneuver	-	-	-	-	-	-				441	
Stage 1	-	-	-	-	-	-				610	
Stage 2	-	-	-	-	-	-				840	
Approach	EB			WB						SW	
HCM Control Delay, s	0			5.3						37.6	
HCM LOS	U			5.5						37.0 E	
HOW EOS										L	
Minor Lane / Major Mvmt		EBL	EBT	EBR	WBL	WBT	WBR S	SWLn1			
Capacity (veh/h)		1520	-	-	1179	-	-	514			
HCM Lane V/C Ratio		-	-	-	0.109	-	-	0.83			
HCM Control Delay (s)		0	-	-	8.426	-		37.6			
HCM Lane LOS		A			Α			E			
HCM 95th %tile Q(veh)		0	-	-	0.366	-	-	8.306			
Notes											
~ : Volume Exceeds Capaci	ty; \$: Dela	y Exceeds	s 300 Sec	onds; Er	ror : Com	putation N	Not Define	ed .			

Intersection Delay, s/veh	4.3										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL		SBR	NEL	NEF
Vol, veh/h	175	372	0	0	149	156	0		0	0	214
Conflicting Peds, #/hr	1	0	0	0	0	1	0		0	0	(
Sign Control	Free	Free	Free	Free	Free	Free	Stop		Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-		-	-	None
Storage Length	90	-	-	-	-	-	-		-	0	
Veh in Median Storage, #	-	0	-	-	0	-	0		-	0	
Grade, %	-	0	-	-	0	-	0		-	0	-
Peak Hour Factor	95	84	92	92	87	95	92		92	92	88
Heavy Vehicles, %	3	6	2	2	8	14	2		2	2	4
Mvmt Flow	184	443	0	0	171	164	0		0	0	243
	N4 1 4									N# 4	
Major/Minor	Major1			Major2						Minor1	
Conflicting Flow All	335	0	0	443	0	0				897	222
Stage 1	-	-	-	-	-	-				811	-
Stage 2	-	-	-	-	-	-				86	-
Follow-up Headway	2.23	-	-	2.22	-	-				3.75	3.34
Pot Capacity-1 Maneuver	1214	-	-	1113	-	-				240	775
Stage 1	-	-	-	-	-	-				344	-
Stage 2	-	-	-	-	-	-				863	-
Time blocked-Platoon, %		-	-		-	-					
Mov Capacity-1 Maneuver	1213	-	-	1112	-	-				203	774
Mov Capacity-2 Maneuver	-	-	-	-	-	-				203	-
Stage 1	-	-	-	-	-	-				292	-
Stage 2	-	-	-	-	-	-				862	-
Approach	EB			WB						NE	
HCM Control Delay, s	2.5			0						14	
HCM LOS	2.0			U						В	
TIOW EOS										U U	
Minor Lane / Major Mvmt		NELn1	EBL	EBT	EBR	WBL	WBT	WBR			
Capacity (veh/h)		659	1213	-	-	1112	-	-			
HCM Lane V/C Ratio		0.393	0.152	-	-	-	-	-			
HCM Control Delay (s)		14	8.499	-	-	0	-	-			
HCM Lane LOS		В	А			Α					
HCM 95th %tile Q(veh)		1.874	0.535	-	-	0	-	-			
Notes											

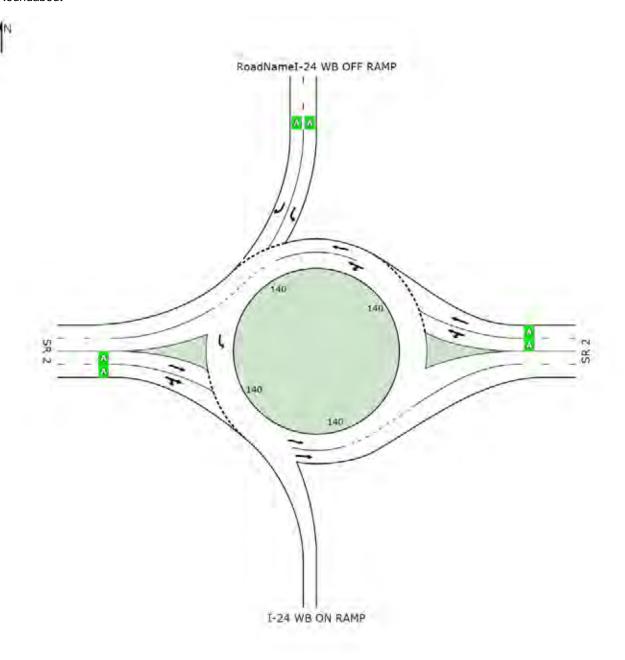
	٠	→	•	•	←	•	4	†	<i>></i>	>	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	↑ 1>		ሻ	∱ 1>		ሻ	4			4	
Volume (vph)	22	278	282	115	169	17	97	11	120	2	5	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
Lane Util. Factor	1.00	0.95		1.00	0.95		0.95	0.95			1.00	
Frt	1.00	0.93		1.00	0.98		1.00	0.87			0.89	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)	1719	3184		1641	3235		1517	1350			1655	
Flt Permitted	0.61	1.00		0.25	1.00		0.72	0.99			0.99	
Satd. Flow (perm)	1106	3184		437	3235		1153	1338			1644	
Peak-hour factor, PHF	0.79	0.78	0.81	0.61	0.86	0.61	0.87	0.69	0.75	1.00	0.63	0.80
Adj. Flow (vph)	28	356	348	189	197	28	111	16	160	2	8	44
RTOR Reduction (vph)	0	257	0	0	15	0	0	101	0	0	28	0
Lane Group Flow (vph)	28	447	0	189	210	0	100	86	0	0	26	0
Heavy Vehicles (%)	5%	4%	6%	10%	10%	6%	13%	0%	18%	2%	2%	2%
Turn Type	Perm	NA		pm+pt	NA		Perm	NA		Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)	11.8	11.8		20.3	20.3		16.6	16.6			16.6	
Effective Green, g (s)	11.8	11.8		20.3	20.3		16.6	16.6			16.6	
Actuated g/C Ratio	0.26	0.26		0.45	0.45		0.37	0.37			0.37	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0			3.0	
Lane Grp Cap (vph)	290	836		318	1462		426	494			607	
v/s Ratio Prot		0.14		c0.06	0.06							
v/s Ratio Perm	0.03			c0.21			c0.09	0.06			0.02	
v/c Ratio	0.10	0.54		0.59	0.14		0.23	0.17			0.04	
Uniform Delay, d1	12.5	14.2		8.6	7.2		9.8	9.5			9.1	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2	0.1	0.7		3.0	0.0		1.3	0.8			0.1	
Delay (s)	12.7	14.9		11.6	7.3		11.1	10.3			9.2	
Level of Service	В	В		В	Α		В	В			А	
Approach Delay (s)		14.8			9.2			10.6			9.2	
Approach LOS		В			А			В			Α	
Intersection Summary												
HCM 2000 Control Delay			12.2	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	acity ratio		0.46									
Actuated Cycle Length (s)	-		44.9	S	um of lost	time (s)			12.0			
Intersection Capacity Utiliza	ation		46.4%	IC	CU Level	of Service	2		А			
Analysis Period (min)			15									

Intersection											
Intersection Delay, s/veh	160.3										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SWL	SWR	
Vol, veh/h	0	202	15	342	174	0	0	0	0	119	
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	
RT Channelized	-	-	Yeild	-	-	None	-	-	-	None	
Storage Length	-	-	-	115	-	-	-	-	0	-	
Veh in Median Storage, #	-	0	-	-	0	-	0	-	0	-	
Grade, %	-	0	-	-	0	-	0	-	0	-	
Peak Hour Factor	92	79	63	86	82	92	92	92	92	80	
Heavy Vehicles, %	2	1	0	4	2	2	2	2	2	6	
Mvmt Flow	0	256	24	398	212	0	0	0	0	149	
Major/Minor	Major1			Major2					Minor2		
Conflicting Flow All	212	0	0	256	0	0			1136	107	
Stage 1	-	-	-	-	-	-			1008	-	
Stage 2	-	-	-	-	-	-			128	-	
Follow-up Headway	2.22	-	-	2.24	-	-			3.58	3.36	
Pot Capacity-1 Maneuver	1356	-	-	1291	-	-			# 187	914	
Stage 1	-	-	-	-	-	-			300	-	
Stage 2	-	-	-	-	-	-			866	-	
Time blocked-Platoon, %		-	-		-	-					
Mov Capacity-1 Maneuver	1355	-	-	1290	-	-			# 129	913	
Mov Capacity-2 Maneuver	-	-	-	-	-	-			# 129	-	
Stage 1	-	-	-	-	-	-			# 207	-	
Stage 2	-	-	-	-	-	-			866	-	
Approach	EB			WB					SW		
HCM Control Delay, s	0			5.9					\$ 518.2		
HCM LOS									F		
Minor Lane / Major Mvmt		EBL	EBT	EBR	WBL	WBT	WBR SWL	n1			
Capacity (veh/h)		1355		-	1290	-		92			
HCM Lane V/C Ratio		-	_	-	0.308	_	- 2.0				
HCM Control Delay (s)		0	_	-	9.029	_	- \$ 518				
HCM Lane LOS		A			Α		ΨΟΙ	F			
HCM 95th %tile Q(veh)		0		-	1.321		- 29.4				
							27.1				
Notes											
~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined											

Intersection											
Intersection Delay, s/veh	2.7										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL		SBR	NEL	NER
Vol, veh/h	96	307	0	0	494	296	0		0	0	179
Conflicting Peds, #/hr	0	0	0	0	0	0	0		0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop		Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-		-	-	None
Storage Length	90	-	-	-	-	-	-		-	0	-
Veh in Median Storage, #	-	0	-	-	0	-	0		-	0	-
Grade, %	-	0	-	-	0	-	0		-	0	-
Peak Hour Factor	80	75	92	92	89	81	92		92	92	84
Heavy Vehicles, %	2	4	2	2	3	5	2		2	2	9
Mvmt Flow	120	409	0	0	555	365	0		0	0	213
Major/Minor	Major1			Major2						Minor1	
Conflicting Flow All	920	0	0	409	0	0				927	205
Stage 1	-	-	-	-	-	-				649	-
Stage 2	-	-	-	-	-	-				278	-
Follow-up Headway	2.22	-	-	2.22	-	-				3.63	3.39
Pot Capacity-1 Maneuver	738	-	-	1146	-	-				248	780
Stage 1	-	-	-	-	-	-				453	-
Stage 2	-	-	-	-	-	-				712	-
Time blocked-Platoon, %		-	-		-	-					
Mov Capacity-1 Maneuver	738	-	-	1146	-	-				208	780
Mov Capacity-2 Maneuver	-	-	-	-	-	-				208	-
Stage 1	-	-	-	-	-	-				379	-
Stage 2	-	-	-	-	-	-				712	-
Approach	EB			WB						NE	
HCM Control Delay, s	2.5			0						14	
HCM LOS										В	
Minor Lane / Major Mvmt		NELn1	EBL	EBT	EBR	WBL	WBT	WBR			
Capacity (veh/h)		631	738	_	-	1146		-			
HCM Lane V/C Ratio		0.369	0.163	_	_	-	_	_			
HCM Control Delay (s)		14	10.823	-	-	0	-	-			
HCM Lane LOS		В	В			A					
HCM 95th %tile Q(veh)		1.699	0.578	-	-	0	-	-			
Notes Volume Exceeds Capaci											
~ . Volume Exceeds Capaci	~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined										

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑ 1>		ሻ	∱ ∱		ሻ	4			4	
Volume (vph)	35	337	119	237	475	26	245	14	229	4	4	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
Lane Util. Factor	1.00	0.95		1.00	0.95		0.95	0.95			1.00	
Frpb, ped/bikes	1.00	0.99		1.00	1.00		1.00	0.99			1.00	
Flpb, ped/bikes	1.00	1.00		1.00	1.00		1.00	1.00			1.00	
Frt	1.00	0.96		1.00	0.99		1.00	0.87			0.88	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)	1805	3289		1735	3513		1618	1491			1648	
Flt Permitted	0.44	1.00		0.21	1.00		0.95	1.00			1.00	
Satd. Flow (perm)	835	3289		387	3513		1618	1491			1648	
Peak-hour factor, PHF	0.88	0.83	0.87	0.87	0.89	0.81	0.72	0.70	0.74	1.00	0.50	0.94
Adj. Flow (vph)	40	406	137	272	534	32	340	20	309	4	8	72
RTOR Reduction (vph)	0	45	0	0	6	0	0	234	0	0	56	0
Lane Group Flow (vph)	40	498	0	272	560	0	306	129	0	0	28	0
Confl. Peds. (#/hr)			1	1					1	1		
Heavy Vehicles (%)	0%	5%	5%	4%	2%	0%	6%	14%	3%	0%	0%	2%
Turn Type	Perm	NA		pm+pt	NA		Split	NA		Split	NA	
Protected Phases		4		3	8		2	2		6	6	
Permitted Phases	4			8								
Actuated Green, G (s)	14.9	14.9		27.9	27.9		18.0	18.0			16.0	
Effective Green, g (s)	14.9	14.9		27.9	27.9		18.0	18.0			16.0	
Actuated g/C Ratio	0.20	0.20		0.38	0.38		0.24	0.24			0.22	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0			4.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0			3.0	
Lane Grp Cap (vph)	168	663		310	1326		394	363			356	
v/s Ratio Prot		0.15		c0.11	0.16		c0.19	0.09			c0.02	
v/s Ratio Perm	0.05			c0.22								
v/c Ratio	0.24	0.75		0.88	0.42		0.78	0.36			0.08	
Uniform Delay, d1	24.7	27.8		18.2	17.0		26.1	23.2			23.1	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2	0.7	4.8		23.2	0.2		13.9	2.7			0.4	
Delay (s)	25.5	32.6		41.3	17.3		40.0	25.9			23.5	
Level of Service	С	С		D	В		D	С			С	
Approach Delay (s)		32.1			25.1			32.3			23.5	
Approach LOS		С			С			С			С	
Intersection Summary												
HCM 2000 Control Delay			29.1	H	CM 2000	Level of S	service		С			
HCM 2000 Volume to Capac	ity ratio		0.67		6.1	11 / 1			44.0			
Actuated Cycle Length (s)			73.9		um of lost				16.0			
Intersection Capacity Utilizat	ion		57.2%	IC	U Level o	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

Paired Roundabout - Intersection 1 (varying number of approach & circul. lanes) MUTCD (FHWA 2009) example number: C-13 Roundabout Guide (TRB 2010) example number: A-14 Roundabout



	South	East	North	West	Intersection
LOS	NA	Α	Α	Α	Α

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Sign Control.

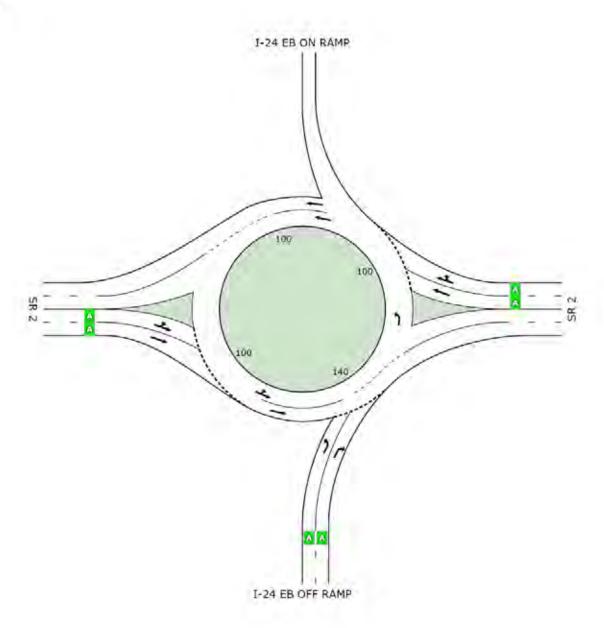
Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Paired Roundabout - Intersection 2 (varying number of approach & circul. lanes) MUTCD (FHWA 2009) example number: C-13 Roundabout Guide (TRB 2010) example number: A-14 Roundabout





	South	East	North	West	Intersection
LOS	Α	Α	NA	Α	Α

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

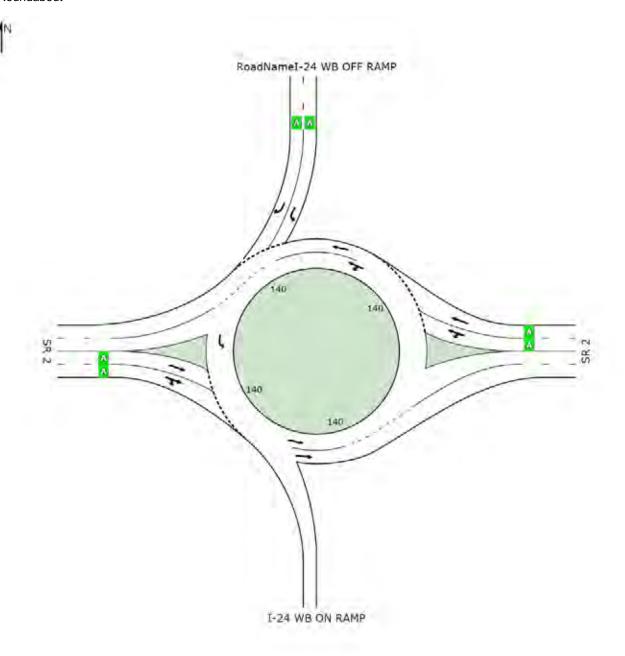
LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	44	7	ሻ	† 1>		**	4	7		4	
Volume (vph)	22	278	282	115	169	17	97	11	120	2	5	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0		4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		1.00	
Frt	1.00	1.00	0.85	1.00	0.98		1.00	1.00	0.85		0.89	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00		1.00	
Satd. Flow (prot)	1719	3471	1524	1641	3235		1517	1585	1369		1655	
Flt Permitted	0.61	1.00	1.00	0.40	1.00		0.72	0.82	1.00		0.99	
Satd. Flow (perm)	1106	3471	1524	691	3235		1153	1349	1369		1649	
Peak-hour factor, PHF	0.79	0.78	0.81	0.61	0.86	0.61	0.87	0.69	0.75	1.00	0.63	0.80
Adj. Flow (vph)	28	356	348	189	197	28	111	16	160	2	8	44
RTOR Reduction (vph)	0	0	258	0	15	0	0	0	101	0	28	0
Lane Group Flow (vph)	28	356	90	189	210	0	63	64	59	0	26	0
Heavy Vehicles (%)	5%	4%	6%	10%	10%	6%	13%	0%	18%	2%	2%	2%
Turn Type	Perm	NA	Perm	pm+pt	NA		Perm	NA	Perm	Perm	NA	
Protected Phases		4		3	8			2			6	
Permitted Phases	4		4	8			2		2	6		
Actuated Green, G (s)	11.5	11.5	11.5	20.0	20.0		16.5	16.5	16.5		16.5	
Effective Green, g (s)	11.5	11.5	11.5	20.0	20.0		16.5	16.5	16.5		16.5	
Actuated g/C Ratio	0.26	0.26	0.26	0.45	0.45		0.37	0.37	0.37		0.37	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0		4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	285	897	393	406	1453		427	500	507		611	
v/s Ratio Prot		0.10		c0.05	0.06							
v/s Ratio Perm	0.03		0.06	c0.16			c0.05	0.05	0.04		0.02	
v/c Ratio	0.10	0.40	0.23	0.47	0.14		0.15	0.13	0.12		0.04	
Uniform Delay, d1	12.6	13.6	13.0	7.8	7.2		9.3	9.2	9.2		9.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00		1.00	
Incremental Delay, d2	0.2	0.3	0.3	0.8	0.0		0.7	0.5	0.5		0.1	
Delay (s)	12.7	13.9	13.3	8.7	7.3		10.0	9.8	9.7		9.1	
Level of Service	В	В	В	А	Α		В	Α	А		Α	
Approach Delay (s)		13.6			7.9			9.8			9.1	
Approach LOS		В			А			Α			А	
Intersection Summary												
HCM 2000 Control Delay			11.1	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capa	acity ratio		0.35									
Actuated Cycle Length (s)			44.5		um of lost	` '			12.0			
Intersection Capacity Utiliza	ation		37.2%	IC	CU Level	of Service			Α			
Analysis Period (min)			15									
o Critical Lana Croup												

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Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBT
Lane Group Flow (vph)	28	356	348	189	225	63	64	160	54
v/c Ratio	0.10	0.39	0.53	0.44	0.16	0.14	0.13	0.26	0.08
Control Delay	13.2	14.8	5.3	10.4	6.0	12.7	12.3	4.2	6.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	13.2	14.8	5.3	10.4	6.0	12.7	12.3	4.2	6.0
Queue Length 50th (ft)	6	40	0	27	13	11	11	0	2
Queue Length 95th (ft)	17	57	31	34	25	35	27	19	11
Internal Link Dist (ft)		408			592		651		11
Turn Bay Length (ft)	90		75	125				125	
Base Capacity (vph)	417	1310	791	434	1994	435	509	616	649
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.27	0.44	0.44	0.11	0.14	0.13	0.26	0.08
Intersection Summary									

Paired Roundabout - Intersection 1 (varying number of approach & circul. lanes) MUTCD (FHWA 2009) example number: C-13 Roundabout Guide (TRB 2010) example number: A-14 Roundabout



	South	East	North	West	Intersection
LOS	NA	Α	Α	Α	Α

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Sign Control.

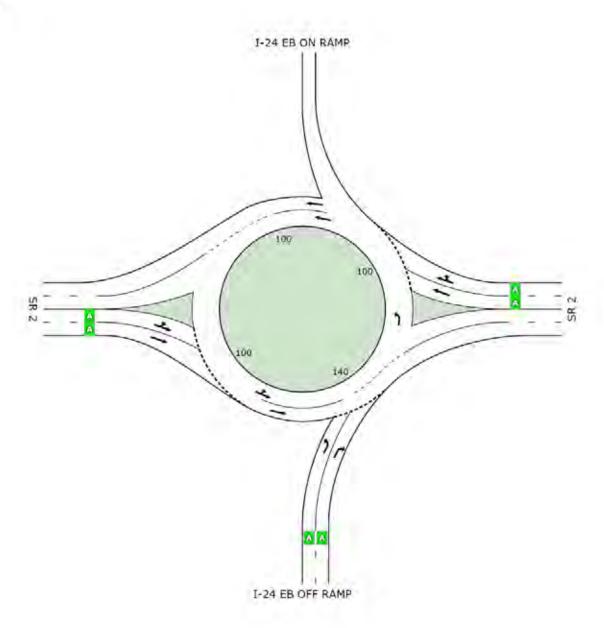
Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Paired Roundabout - Intersection 2 (varying number of approach & circul. lanes) MUTCD (FHWA 2009) example number: C-13 Roundabout Guide (TRB 2010) example number: A-14 Roundabout





	South	East	North	West	Intersection
LOS	Α	Α	NA	Α	Α

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Sign Control.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	† †	7	ሻ	† Ъ		*	र्स	7	022	4	00.1
Volume (vph)	35	337	119	237	475	26	245	14	229	4	4	68
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0		4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.95	0.95	1.00		1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00		1.00	1.00	0.99		1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00		1.00	
Frt	1.00	1.00	0.85	1.00	0.99		1.00	1.00	0.85		0.88	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00		1.00	
Satd. Flow (prot)	1805	3438	1504	1735	3513		1618	1617	1547		1648	
Flt Permitted	0.44	1.00	1.00	0.30	1.00		0.95	0.96	1.00		1.00	
Satd. Flow (perm)	835	3438	1504	543	3513		1618	1617	1547		1648	
Peak-hour factor, PHF	0.88	0.83	0.87	0.87	0.89	0.81	0.72	0.70	0.74	1.00	0.50	0.94
Adj. Flow (vph)	40	406	137	272	534	32	340	20	309	4	8	72
RTOR Reduction (vph)	0	0	110	0	7	0	0	0	236	0	55	0
Lane Group Flow (vph)	40	406	27	272	559	0	180	180	73	0	29	0
Confl. Peds. (#/hr)			1	1					1	1		
Heavy Vehicles (%)	0%	5%	5%	4%	2%	0%	6%	14%	3%	0%	0%	2%
Turn Type	Perm	NA	Perm	pm+pt	NA		Split	NA	Perm	Split	NA	
Protected Phases		4		3	8		2	2		6	6	
Permitted Phases	4		4	8					2			
Actuated Green, G (s)	13.3	13.3	13.3	23.3	23.3		16.0	16.0	16.0		16.0	
Effective Green, g (s)	13.3	13.3	13.3	23.3	23.3		16.0	16.0	16.0		16.0	
Actuated g/C Ratio	0.20	0.20	0.20	0.35	0.35		0.24	0.24	0.24		0.24	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0		4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0		3.0	
Lane Grp Cap (vph)	165	679	297	294	1216		384	384	367		391	
v/s Ratio Prot		0.12		c0.08	0.16		0.11	c0.11			c0.02	
v/s Ratio Perm	0.05		0.02	c0.24					0.05			
v/c Ratio	0.24	0.60	0.09	0.93	0.46		0.47	0.47	0.20		0.07	
Uniform Delay, d1	22.8	24.6	22.1	19.6	17.1		22.0	22.0	20.5		19.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00		1.00	
Incremental Delay, d2	0.8	1.4	0.1	33.2	0.3		4.1	4.1	1.2		0.4	
Delay (s)	23.5	26.0	22.2	52.8	17.4		26.1	26.1	21.8		20.3	
Level of Service	С	С	С	D	В		С	С	С		С	
Approach Delay (s)		24.9			28.9			24.1			20.3	
Approach LOS		С			С			С			С	
Intersection Summary												
HCM 2000 Control Delay			26.0	H	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	city ratio		0.57									
Actuated Cycle Length (s)			67.3		um of lost				16.0			
Intersection Capacity Utiliza	tion		46.4%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									
c Critical Lane Group												

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Lane Group EBL EBT EBR WBL WBT NBL NBT NBF	SBT
Lane Group Flow (vph) 40 406 137 272 566 180 180 309	84
v/c Ratio 0.24 0.60 0.34 0.93 0.46 0.47 0.47 0.5	0.19
Control Delay 26.5 28.5 7.0 58.9 18.1 27.5 27.5 6.6	9.0
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0
Total Delay 26.5 28.5 7.0 58.9 18.1 27.5 27.5 6.6	9.0
Queue Length 50th (ft) 14 81 0 85 91 68 68	4
Queue Length 95th (ft) 38 111 35 #196 131 100 97 26	8
Internal Link Dist (ft) 408 592 651	11
Turn Bay Length (ft) 90 75 125 100	
Base Capacity (vph) 198 818 464 294 1365 384 384 603	447
Starvation Cap Reductn 0 0 0 0 0 0 0	0
Spillback Cap Reductn 0 0 0 0 0 0 0	0
Storage Cap Reductn 0 0 0 0 0 0 0	0
Reduced v/c Ratio 0.20 0.50 0.30 0.93 0.41 0.47 0.47 0.5	0.19

Intersection Summary

^{# 95}th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

TENNESSEE DEPARTMENT OF TRANSPORTATION PROJECT PLANNING DIVISION

		33007-1205-9	4			ROUTE:	S.R. 2				
COUNTY		<u>IAMILTON</u>				_ CITY:	CHATT	ANOOGA	1		
PROJECT											
PROJECT	DESCRI	PHON: S.	R. 2 (a) 1-2	4 WES	STBOUN.	D RAMPS.		, <u></u>			
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							DESIGN DESIGN ROADWAY AVERAGE				
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AADT	YEAR	AADT	DHV	%	YEAR	DIR.DIST.	DHV	AADT		LOADS	
10,330	2016	13,950	1,383	10	2016	55-45	5	8 8	FLEX	RIGID	
				10	2010	33 13					
				 						 	
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REQUEST	ED BY:	NAME	GLENE	A TY	US			DATE	3/26/13		
		DIVISION	PLANN	ING						_	
		ADDRESS			LK BUIL						
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THIS TRAFFIC IS BASED ON 2011 & 2012 CYCLE AND RAMP COUNTS AND [1] 12-HOUR TURNING MOVEMENT COUNT [MAY 2013]. THE FUTURE TRAFFIC IS BASED ON THE AVERAGE GROWTH RATE FROM THE CHATTANOOGA TPO COMPUTER ASSIGNMENT MODEL. AADT's & DHV's ARE INCLUDED.

DHV'S ARE NOT REQUIRED FOR SIDE ROADS LESS THAN 1000 AADT. NOTE: FOR BRIDGE REPLACEMENT PROJECTS, ADLS ARE NOT REQUIRED FOR AADT'S OF 1000 OR LESS AND PERCENTAGE OF TRUCKS OF 7% OR LESS.

SEE ATTACHMENTS FOR TURNING MOVEMENTS AND/OR OTHER DETAILS.

