## SR 6 TPR EXECUTIVE SUMMARY

This Transportation Planning Report (TPR) examines the improvements listed in the Nashville Area Metropolitan Planning Organization's (MPO) Long Range Transportation Plan (LRTP) for 10.87 miles of State Route 6 (SR 6) from the City of Spring Hill to the southern limits of the City of Franklin in south-central Tennessee. The project limits also include the Town of Thompson's Station. Widening SR 6 through the City of Spring Hill's CBD or through Thompson's Station Town Limits is not supported by local officials. This TPR was initiated by request of the MPO on behalf of Williamson County. Coordination with local agencies, including the Nashville Area MPO, Williamson County, the City of Spring Hill, the Town of Thompson's Station, and the City of Franklin was conducted throughout the development of this report.

## Purpose for Improvements

The purpose for improvements to SR 6 is to provide a transportation facility that will improve regional mobility, reduce congestion, meet the changing social demands of the area, and improve safety by addressing existing roadway deficiencies. The following options are included in the TPR:

## No Build Option

The No Build Option provides no improvements and serves as a baseline option against which all other options are compared. No improvement costs are associated with the No Build Option. All local officials contacted for this TPR acknowledge a need for improvements and therefore do not support a No Build Option.

## Widen Along the Existing Alignment Option

This option is similar to the improvements listed in the MPO's LRTP and widens the existing route from two (2) travel lanes to four (4) travel lanes. The cost for corridor wide improvements is estimated to range between $\$ 90$ and $\$ 103$ million in year 2014 dollars.

## Additional Options

Due to the lack of support by local officials for the Widen Along the Existing Alignment Option, several additional options were studied, including bypass options, reduced travel lane options, and spot improvements. Many of these options, including the bypass options, are significantly different from the improvements listed in the LRTP. These options need additional coordination with the MPO's planning process. Furthermore, the additional options, in their current form, do not meet the purpose and need of the project. Further refinement of these additional options through the MPO's planning process could lead to them becoming viable improvement options. The additional improvement options studied are assessed in the Options Requiring Additional Study Appendix.


# TRANSPORTATION PLANNING REPORT 

STATE ROUTE 6 (US 31)<br>FROM KEDRON ROAD TO MACK HATCHER PARKWAY MAURY AND WILLIAMSON COUNTIES PIN 111040.00



| Recommended by: | Signature |
| :--- | :--- | :--- |
| CHIEF OF <br> ENVIRONMENT <br> AND PLANNING |  |
| TRANSPORTATION DIRECTOR <br> PROJECT PLANNING DIVISION | DRATE |
| TRANSPORTATION MANAGER 2 <br> PROJECT PLANNING DIVISION | 6.14 .10 |

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SR 6 (US 31) TPR
Maury and Williamson Counties
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### 1.0 PURPOSE OF THE TPR

This Transportation Planning Report (TPR) examines the improvements listed in the Nashville Area Metropolitan Planning Organization's (MPO) Long Range Transportation Plan (LRTP) for 10.87 miles of State Route 6 (SR 6) from the City of Spring Hill to the southern limits of the City of Franklin in south-central Tennessee. The improvements widen the existing route from two (2) travel lanes to four (4) travel lanes. A No Build Option was also studied. This TPR was initiated by request of the MPO on behalf of Williamson County. Coordination with local agencies, including the Nashville Area MPO, Williamson County, the City of Spring Hill, the Town of Thompson's Station, and the City of Franklin was conducted throughout the development of this report.

It should be noted that the options presented in this TPR are part of a planning process, and not a design process. The corridor improvement options are presented as 2000 foot wide study areas to allow flexibility when specific alignments are developed in the future. Specific alignments will be developed in future planning documents as the National Environmental Policy Act (NEPA) process continues.

### 2.0 HISTORY \& BACKGROUND

### 2.1 PROJECT STUDY AREA

The project study area extends from the City of Spring Hill to the southern limits of the City of Franklin, in south-central Tennessee. Please refer to Figure 2.1.1 Area Vicinity Map, Figure 2.1.2 Project Location Map, or the Conceptual Plan Sheets for visual representations of the study area.


Figure 2.1.1 Vicinity Map


Figure 2.1.2 Project Location Map (1 of 4)


Figure 2.1.2 Project Location Map (2 of 4)

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Figure 2.1.2 Project Location Map (3 OF 4)


Figure 2.1.2 Project Location Map (4 OF 4)

### 2.2 PROJECT HISTORY

This TPR is a continuation of several previous studies and meetings. A summary of previous activity concerning this project is provided in Table 2.2 Project History. Correspondence concerning this project, including minutes from the previous meetings, is provided in the Appendix.

Table 2.2 Project History

| Date | Activity |
| :---: | :--- |
| April 4, 2008 | Mr. Michael Skipper, Executive Director for the Nashville Area <br> Metropolitan Planning Organization (MPO), issued a letter to <br> TDOT requesting a Transportation Planning Report (TPR) be <br> completed for SR 6 from Old Kedron Road to Mack Hatcher <br> Memorial Parkway. Improvements along SR 6 are listed in <br> the MPO's 2030 Long Range Transportation Plan Update with <br> a horizon year of 2016. A copy of the letter is provided in the <br> Appendix. |
| May 2008 | Florence \& Hutcheson, Inc. was retained by TDOT's Project <br> Planning Division to produce a Transportation Planning <br> Report (TPR) to examine improvements along SR 6 from Old <br> Kedron Road in Spring Hill to Mack Hatcher Memorial <br> Parkway in Franklin. |
| August 29, 2008 | A site visit was held to gain input from local stakeholders <br> along the route. Thirteen (13) people were in attendance <br> representing the City of Franklin, the Nashville Area MPO, <br> TDOT, the Town of Thompson's Station, and Williamson <br> County. The meeting began with a short discussion of the <br> project limits and the status of the study. The meeting was <br> then opened up for questions, comments, recommendations <br> for improvements (including spot improvements), and closed <br> with a field reconnaissance. Minutes of this meeting are <br> provided in the Appendix. |

Table 2.2 Project History (Cont.)

| Date | Activity |
| :---: | :--- |
| September 10, 2008 | A meeting was held with the City of Spring Hill to gain input <br> from local stakeholders. Six (6) people were in attendance. <br> The meeting began with a discussion of what occurred at the <br> Site Visit on 8/29/08. The meeting was then opened up for <br> questions, comments, and recommendations for <br> improvements (including spot improvements). A copy of the <br>  <br> Hutcheson. Minutes of this meeting are provided in the <br> Appendix. |
| January 28, 2009 | A meeting was held with TDOT and the MPO to discuss the <br> SR 6 TPR Draft. The primary focus of the meeting was to <br> determine if the options included in the TPR should be strictly <br> limited by what is listed in the MPO's LRTP, or if additional <br> options requested by local agencies should also be included. <br> The Draft SR 6 TPR at that time included bypass options and <br> spot improvements, in addition to an option to widen along the <br> existing alignment. The LRTP only lists widening along the <br> existing alignment. Additional discrepancies between the |
| LRTP and the options presented in the TPR were discussed. |  |
| It was determined to proceed with submitting the SR 6 TPR |  |
| with the Bypass Options. Minutes of this meeting are |  |
| provided in the Appendix. |  |

Table 2.2 Project History (Cont.)

| Date | Activity |
| :---: | :--- |
| March 4, 2010 | A meeting was held with TDOT to discuss the revised Draft <br> SR 6 TPR. At this meeting, it was decided to limit the TPR to <br> a No Build Option and an option resembling the improvements <br> listed in the LRTP. This was decided due to the additional <br> options studied not meeting the Purpose and Need for the <br> project. The additional options are also significant alterations <br> to the LRTP, and have not been included in the MPO's <br> planning process. The additional options investigated are <br> included in the Appendix, as they may be modified with <br> further study to meet the Purpose and Need of the project. <br> Significant alterations to the project should originate through <br> the MPO Planning Process. |

### 2.3 PLANNED AND RECENT IMPROVEMENTS

### 2.3.1 Planned Improvements

Williamson and part of Maury County are part of the Nashville Area Metropolitan Planning Organization (MPO). The Nashville Area MPO leads in the development of the region's Long Range Transportation Plan (LRTP) and short-range Transportation Improvement Program (TIP). The goal of the Nashville Area MPO is to provide leadership in planning a comprehensive, multi-modal regional transportation system that promotes economic vitality and encourages sustainable land development to protect community and natural resources. The current TIP is for fiscal years 2008 2011. The current LRTP Update (amended November 14, 2007) covers a twenty five (25) year planning horizon through the year 2030.

Numerous transportation improvements are planned within the study area that will directly affect traffic operations along SR 6. Tables 2.3.1 MPO 20082011 TIP Projects and 2.3.2 MPO 2030 LRTP
Projects provide a list of the proposed improvements in their respective current plans. The projects listed in these tables are either along SR 6, intersect SR 6, or provide improvements to a parallel facility. Improvements along a parallel facility are listed because utilization of these routes may decrease congestion along SR 6.

MPO's are established through federal legislation and exist throughout the United States in urbanized areas of more than 50,000 people. It is through MPOs that local communities prioritize state and federal funds for various transportation projects and programs. Each MPO is comprised of representatives from:

- City and County governments
- Their respective State Department of Transportation
- The Federal Highway Administration
- The Federal Transit Administration
- Other local or regional transportation-related agencies such as transit authorities and airports.

The improvements for SR 6 listed in the Nashville Area MPO's 2030 LRTP are to widen SR 6 from the existing two (2) travel lanes to four (4) travel lanes. Specifically, the LRTP promotes widening SR 6 from the existing two (2) lanes to five (5) lanes between Old Kedron Road and Buckner Road (MPO LRTP Project \#6001, Horizon Year 2016). The LRTP also specifies widening SR 6 from the existing two (2) lanes to four (4) or five (5) lanes between Buckner Road and Henpeck Lane (MPO LRTP Project \#6022, Horizon Year 2016). The LRTP specifies SR 6 will be five (5) lanes within Spring Hill's City Limits and four (4) lanes in Williamson County. Spring Hill's City Limits end approximately one-quarter ( $1 / 4$ ) of a mile north of Buckner Road. Thompson's Station's Town Limits are not mentioned in the LRTP. Thompson's Station's Town Limits border Spring Hill's City Limits and end approximately 3.2 miles before Franklin's City Limits begin. Also of note, Old Kedron Road does not intersect SR 6. Kedron Road intersects SR 6. The existing five (5) lane section of SR 6 south of Spring Hill transitions to three (3) lanes at Kedron Road. The logical terminus of Kedron Road is the southern limit of this TPR's study area.

It should be noted that the northern terminus for improvements to SR 6 listed in the LRTP extend only to Henpeck Lane. The study limits of this TPR extend from Kedron Road to Mack Hatcher Parkway. Therefore, improvements to the 1.25 mile section of SR 6 from Henpeck Lane to Mack Hatcher Parkway are not in the LRTP. In discussions with a representative of the MPO, it was noted that this section will need to be adopted into the LRTP. As part of the Nashville Area MPO's planning process, a Tier 2 report will also need to be performed by the MPO for this segment of SR 6 . Tier 2 reports are necessary to comply with the Nashville Area MPO's Congestion Management Process. Ending the project at Henpeck Lane is not a logical terminus. The City of Franklin is improving SR 6 to five (5) lanes north of Mack Hatcher. Terminating this project at Henpeck Lane would leave a two (2) travel lane segment between these four (4) travel lane segments.

Other than improvements to SR 6, significant highway improvements planned in the area include widening Interstate 65 and completion of the southern loop of SR 840. Interstate 65 is planned to be improved from four (4) travel lanes to eight (8) travel lanes (including high occupancy vehicle lanes) between SR 840 and SR 96.

SR 840 was initiated in 1986 as part of the state's Better Roads Program to provide economic growth by improving access to communities in Middle Tennessee. On the 78 mile route, 57.1 miles are open to traffic, with 20.9 miles remaining in some phase of construction or development. The entire route is scheduled to be open to traffic by 2012. Completion of SR 840 can be expected to impact traffic patterns within the study area.

The traffic data utilized in this report was generated in conjunction with the MPO's travel demand model. This model incorporates future improvements as programmed in the LRTP. Therefore, the traffic data utilized in this report incorporates the effects on travel patterns created by the highway improvements listed in the LRTP and TIP. The traffic generation calculations are provided in the Appendix.

In addition to the MPO planning data, the Major Thoroughfare Plans of the City of Spring Hill and the City of Franklin were examined. The City of Franklin is currently in the planning stages to widen SR 6 to five (5) lanes north of Mack Hatcher Parkway, at the northern terminus of this TPR. This will provide a logical terminus for the improvements discussed in this TPR.

The Town of Thompson's Station was consulted for this TPR. The town is developing a greenway plan. The greenway locations are not currently known. However, it is anticipated
greenways will eventually cross SR 6. The town has requested that the greenways and SR 6 be grade separated. This could be accomplished by providing oversized culverts or bridge spans where these structures are needed. The design of improvements along SR 6 should therefore be coordinated with the Town, especially concerning the development of their greenway plan.

Table 2.3.1 MPO 2008-2011 TIP Projects

| Description | Horizon <br> Year | Cost |
| :--- | :---: | :---: |
| Widen SR 247 from SR 6 to near Interstate 65 from two (2) <br> lanes to three (3) lanes. | 2008 (PE) | $\$ 15,580,000$ |
| Construct new four (4) lane segment of SR 840 from west of <br> Bending Chestnut Road to east of Thompson's Station Road | 2008 (ROW) | $\$ 75,000,000$ |
| Coordinate the signal timing of six (6) existing intersections <br> along SR 6 between Stephen P. Yokich Parkway and <br> Buckner Road. | 2008 | $\$ 540,000$ |
| Construct new four (4) lane segment of SR 840 from <br> southeast of SR 100 to west of Bending Chestnut Road | 2008 | $72,500,000$ |
| Construct new four (4) lane segment of SR 840 from west of <br> Bending Chestnut Road to west of Leipers Creek Road (SR <br> 46) | 2009 | $\$ 70,000,000$ |
| Construct new four (4) lane segment of SR 840 from west of <br> Leipers Creek Road (SR 46) to west of Carters Creek Pike <br> (SR 246) | 2009 | $\$ 41,900,000$ |
| Improve the intersection of SR 6 at Duplex/Beechcroft Road. | 2011 | $\$ 740,000$ |
| Widen Interstate 65 from south of SR 840 to SR 96, including <br> the interchange with Peytonsville Rd, from four (4) lanes to <br> eight (8) lanes. The interchange may be improved to a single <br> point urban design. | 2011 | $\$ 84,549,392$ |
| Total Improvements: | $\$ 360,809,392$ |  |

TABLE 2.3.2 MPO 2030 LRTP PROJECTS

| Description | Horizon <br> Year | Cost |
| :--- | :---: | :---: |
| Widen SR 6 from Buckner Lane to Henpeck Lane from two <br> (2) lanes to four (4) lanes in the County and from two (2) <br> lanes to five (5) lanes through the City of Spring Hill. Project <br> \#6022. | 2016 | $\$ 29,000,000$ |
| Widen SR 6 from Old Kedron Road to Buckner Road from <br> two (2) lanes to five (5) lanes. Project \#6001. | 2016 | $\$ 7,200,000$ |
| Widen SR 6 from Mack Hatcher Parkway (SR 397) to Downs <br> Boulevard from three (3) lanes to five (5) lanes, with bike <br> lanes, sidewalk, and curb and gutter. Project \#6027 | 2016 | $\$ 9,251,500$ |
| Widen Goose Creek Bypass (SR 248) from SR 106 <br> (Lewisburg Pike) to Interstate 65 from two (2) lanes to four <br> (4) lanes with a median and bike lanes. Project \#6034. | 2016 | $\$ 2,450,000$ |
| Widen SR 106 (Lewisburg Pike) from Henpeck Lane to Mack <br> Hatcher Parkway (SR 397) from two (2) lanes to four (4) <br> lanes with bike lanes. Project \#6032. | 2016 | $\$ 15,000,000$ |
| Widen SR 106 (Lewisburg Pike) from Critz Lane to Goose <br> Creek Bypass (SR 248) from two (2) lanes to four (4) lanes. <br> Project \#6021 | 2020 | $\$ 15,000,000$ |
| Widen Goose Creek Bypass (SR 248) from SR 6 to SR 106 <br> (Lewisburg Pike) from two (2) lanes to three (3) lanes. <br> Project \#6038. | 2025 | $\$ 11,000,000$ |
| Construct an interchange at Interstate 65 and Thompson's <br> Station Road. Project \#6046. | 2030 | $\$ 6,500,000$ |
| Total Improvements: | $\$ 95,401,500$ |  |

### 2.3.2 Recent Improvements

In addition to the planned improvements previously discussed, the City of Spring Hill has recently constructed Miles Johnson Parkway. The southern terminus of Miles Johnson Parkway is located at an intersection with Duplex Road (SR 247) approximately 0.4 miles east of SR 6. The City's Major Thoroughfare Plan lists a project to extend Miles Johnson Parkway south approximately 0.2 miles to Old Kedron Road. Miles Johnson Parkway's northern terminus is at an intersection with SR 6 approximately 0.6 miles north of Spring Hill's Central Business District (CBD). Miles Johnson Parkway should reduce traffic in the CBD by providing access between numerous new subdivisions located along Duplex Road and destinations to the north. Additionally, when the extension to Kedron Road is complete, Miles Johnson Parkway will provide improved access to an interchange on Saturn Parkway (SR 396), which will further enhance this route as an alternate to SR 6 through the CBD. It is unknown when funding for the 0.2 mile long Miles Johnson Parkway extension will be available.


Figure 2.3 Miles Johnson Parkway Detail

### 3.0 PURPOSE AND NEED FOR IMPROVEMENTS

### 3.1 PURPOSE FOR IMPROVEMENTS

The purpose for improvements to SR 6 is to provide a transportation facility that will improve regional mobility, reduce congestion, meet the changing social demands of the area, and improve safety by addressing existing roadway deficiencies. This TPR was initiated by request of the Nashville Area MPO on behalf of Williamson County. The MPO's 2030 Long Range Transportation Plan (LRTP) calls for widening SR 6 from the existing two (2) travel lanes to four (4) travel lanes.

### 3.2 NEED FOR IMPROVEMENTS

### 3.2.1 Improve Regional Mobility

Improvements to the transportation system are needed to improve regional mobility. SR 6 is a critical arterial in south-central Tennessee. Within the study limits of this TPR, SR 6 runs parallel to Interstate 65 and serves regional travel demand between Columbia, Spring Hill, Thompson's Station, and Franklin. Considerable commuter driving patterns occur between these cities, creating congestion during peak travel times. The Nashville Area MPO reports that twelve percent (12\%) of Maury County residents commute to work in Williamson County and nine percent (9\%) commute to Davidson County. SR 6, along with Interstate 65 and SR 106 (Lewisburg Pike) are the primary north-south routes in the region. When traffic incidents occur on Interstate 65, SR 6 is a primary detour route.

### 3.2.2 Reduce Congestion

Improvements to the transportation system are needed to reduce congestion. As mandated by federal transportation regulations, a Congestion Management Process (CMP) has been implemented in the Nashville MPO Area. A CMP is required due to the MPO Area having a population over 200,000 people and therefore being a Transportation Management Area (TMA). Projects must comply with the CMP prior to approval in the LRTP. The MPO's 2030 LRTP calls for widening SR 6 from the existing two (2) travel lanes to four (4) travel lanes. Therefore, the Nashville Area MPO's planning process indicates SR 6 is congested and that travel demand reduction strategies outlined in the CMP Document can not address the congestion. Local officials and field observations verify congestion limits mobility along SR 6 during AM and PM peak travel times.

### 3.2.3 Meet Changing Social Demands

Improvements to the transportation system are needed to meet the changing social demands of the area. The population of Williamson County grew by $56.3 \%$ from 1990 to 2000, and was the fastest growing county in Tennessee. The population of Maury County grew by $26.8 \%$ from 1990 to 2000. The City of Spring Hill has seen extraordinary growth since 2000. There are several major regional employers located within or near the study limits of this TPR, including the newly relocated North American Headquarters of Nissan in the Cool Springs area of

Franklin. This formerly rural area has seen extensive changes in population and land use in recent years and the existing transportation system does not meet the demand.

### 3.2.4 Improve Safety

Improvements to SR 6 are needed to improve safety by addressing existing roadway deficiencies. The segment of SR 6 through the City of Spring Hill's CBD has a crash rate that is approximately one and a half (1.5) times higher than the statewide rate for similar roadways. High crash rates are also reported between Campbell Station Parkway and Thompson's Station Road, at the Goose Creek Bypass Intersection, and at the Mack Hatcher Parkway Intersection. The primary deficiency along the route is poor traffic operations.

### 4.0 EXISTING CONDITIONS

### 4.1 COMMUNITY DESCRIPTION

Over ninety percent (90\%) of the project's limits are located in Williamson County, with the remainder being located in Maury County. Williamson and Maury Counties are located in Southern Middle Tennessee. Williamson and part of Maury County are part of the Nashville Area MPO. There are ninety-five (95) counties in Tennessee. Williamson and Maury County are the eleventh ( $11^{\text {th }}$ ) and seventeenth ( $17^{\text {th }}$ ) largest counties by population, with populations of 126,638 and 69,498 respectively (2000 US Census). The population of Williamson County grew by 56.3\% from 1990 to 2000, and was the fastest growing county in Tennessee. The population of Maury County grew by 26.8\% from 1990 to 2000.

Williamson County is:

- The wealthiest county in TN
- The fastest growing county in TN
- Has numerous historic sites

Williamson County is the sixteenth ( $\left.16^{\text {th }}\right)$ largest County by land area in Tennessee with an area of 583 square miles. Williamson County is the wealthiest county in Tennessee and ranked as one of the wealthiest counties in the country. In 2004, the median household income was $\$ 79,692$ with a per capita income of $\$ 44,298$. Williamson County also had the lowest unemployment rate in the State in 2006 at $3.5 \%$. This is below the 2006 unemployment rate for Tennessee of $5.2 \%$.

Maury County is the tenth $\left(10^{\text {th }}\right)$ largest County by land area in Tennessee with an area of 613 square miles. In 2004, the median household income for the county was $\$ 43,116$ with a per capita income of $\$ 28,126$. Maury County had the twenty-ninth ( $29^{\text {th }}$ ) highest unemployment rate in the state in 2006 at $5.4 \%$. The Nashville Area MPO reports that twelve percent (12\%) of Maury County residents commute to work in Williamson County and nine percent (9\%) commute to Davidson County. SR 6 is a primary commuting route between Maury County and southern Williamson County.

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The Project corridor
crosses the following
communities:
- Spring Hill
- Thompson's Station
- Franklin
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specifies a maximum residential building density of one (1) residential dwelling unit per acre, with no zoning for commercial development. The terrain in the study area is rolling. A map of Williamson County's land use zones is provided in Figure 4.1.1 Land Use Zones.

The City of Franklin is the county seat of Williamson County and is the most populous city with a population of 41,842 (2000 US Census). Franklin is located approximately nineteen (19) miles south of Nashville. The median household income in Franklin is $\$ 65,506$ with a per capita income of $\$ 32,160$. The northern terminus of the study corridor is located at Franklin's southern city limits. The land use in this area has a moderate building density with residential, public, and commercial developments.

Thompson's Station is located in Williamson County and is approximately twenty-five (25) miles south of Nashville, just south of the City of Franklin. Thompson's Station has a population of 1,283 (2000 US Census). The median household income is $\$ 66,875$ with a per capita income of $\$ 24,143$.

The portion of Thompson's Station adjacent to the study area has a moderate building density. The land use is generally residential, with some minor commercial development, including a service station. The Town's land use and zoning policy is designed to spur development on the east side of town, between SR 6 and Interstate 65. The Town's zoning calls for more condensed development along SR 6 , residential development east of SR 6, and agricultural development west of the CSX Corporation rail line. Development along Critz Lane, which extends from SR 6 east to SR 106 (Lewisburg Avenue), is expected to see significant residential growth in the near future. Several large residential developments were zoned in this area, but are currently stalled due to current economic conditions. It is anticipated these areas will see growth when the economy rebounds.

The City of Spring Hill is located along the Maury and Williamson county border approximately thirty (30) miles south of Nashville. The City had a population of 7,715 in the 2000 US Census. The City has seen extraordinary growth since 2000 . The median household income is $\$ 60,872$, with a per capita income of $\$ 21,688$.

The CBD of Spring Hill has a compact building density and includes numerous historic properties. SR 6 bisects this urban environment with a narrow sixty (60) feet of existing right-ofway. Further north, the land use within Spring Hill's City Limits is predominantly residential or commercial with varying land use densities. Numerous commercial properties are located along SR 6. Many of these developments have features, including parking lots, retaining walls, and detention ponds, located adjacent to the existing right-of-way.

There are several major regional employers located within or near the study limits of this TPR. Industries that employee fifty (50) or more employees within the study area are listed in Table 4.1.1. These industries are mapped in Figure 4.1.2.

After the economic data discussed above was collected, the nation experienced an economic recession. During this economic downturn, Tennessee's unemployment rate increased and the GM (Saturn) automotive plant was idled. This plant is located just south of the study corridor in Spring Hill. There are currently no known plans to resume the assembly of automobiles at the plant. Until the production of automobiles is resumed there will be a considerable negative economic impact on Spring Hill and the adjacent communities.

Several Utilities are located within the study area. The utility service providers are listed in Table 4.1.2.


Figure 4.1.1 Land Use Zones

TAble 4.1.1 MAJOR Industries

| ID | Name | Product | Number of Employees |
| :---: | :---: | :---: | :---: |
| 11* | GM - Saturn* | Automobile Manufacturer* | 4,295* |
| 12 | Nissan - North American | Automobile Manufacturer Headquarters | 1,600 |
| 13 | Healthways | Corporate Headquarters | 1,000 |
| 14 | AIM Healthcare | Healthcare Industry | 815 |
| 15 | Progeny Marketing Innovations | Marketing and Insurance | 550 |
| 16 | Civil Constructors, Inc. | Construction Contractor | 500 |
| 17 | Mars Petcare US | Pet Food Manufacturer Headquarters | 500 |
| 18 | Affinion Group | Marketing | 470 |
| 19 | LDM Technologies | Automotive Supplier | 400 |
| 110 | CIMplify | Medical Computer Information Management | 370 |
| 111 | Lasko Metal Products | Fans, Humidifiers, and Heaters | 350 |
| 112 | DA Vita | Dialysis Treatments | 350 |
| 113 | Apcom, Inc. | Divisional Headquarters | 350 |
| 114 | Plastech Engineered Products, Inc. | Plastic Injection Molding | 230 |
| 115 | Essex Group, Inc. | Magnet Wire | 209 |
| 116 | Husky Truss \& Building Supplies | Wooden Floor Trusses | 100 |
| 117 | Premier Mfg. Support Services | Janitorial Services | 80 |
| 118 | US Engineering Company | Mechanical Contractors | 79 |
| 119 | Spandeck, Inc. | Hydraulic Cranes | 75 |
| 120 | Pioneer Manufacturing | Loading Dock Equipment | 60 |
| 121 | Phoenix Metals | Specialty Metals | 54 |
| 122 | Haley Tool \& Stamping, Inc. | Metal Stamping, Tool \& Die | 50 |
| 123 | Prime Colorants, Inc | Additives for Plastics | 50 |

* The GM automobile plant was idled in 2009. There are currently no known plans to resume the assembly of automobiles at the plant.

SR 6 (US 31) TPR
Maury and Williamson Counties


Figure 4.1.2 Major Industries Map

## Table 4.1.2 Utility Service Providers

| Name | Utility Service | Source/Company |
| :--- | :--- | :--- |
| Middle Tennessee <br> Electric Membership <br> Corporation | Electricity | TVA |
| Duck River Electric <br> Membership <br> Corporation | Electricity | TVA |
| Columbia Power <br> Systems | Electricity | TVA |
| City of Franklin | Water | Harpeth Valley Utility District/Harpeth <br> River |
| City of Spring Hill <br> Harpeth Valley Utility <br> District | Water | Duck River |
| Columbia Water <br> Systems | Water | Harpeth River |
|  <br> Thompson's Station <br> (H.B. \& T.S.) Utilities | Water | Duck River |
| City of Franklin | Sewer | Duck and Cumberland Rivers |
| City of Spring Hill | Sewer | n/a |
| Harpeth Valley Utility <br> District | Sewer | n/a |
| Columbia <br> Wastewater Systems | Sewer | n/a |
| Town of Thompson's <br> Station | Sewer | n/a |
| Atmos Energy | Natural Gas | Natural \& Columbia Gulf Transmission, <br> Texas Eastern, Columbia Gulf |
| AT\&T | Telephone | n/a |
| Comcast | n/a |  |

### 4.2 ENVIRONMENTAL SITES

Williamson and Maury County are rich in history and were the location of several major Civil War battles including the Battles of Brentwood, Spring Hill, Thompson's Station, and Franklin. Williamson County has more properties on the National Register of Historic Places than any other County outside of the State of Virginia. Therefore, several environmentally sensitive locations and community resources are located within the study area. In addition to historic places, these areas include churches, cemeteries, schools, environmental sites, parks, major streams, utilities, and railroads. The environmentally sensitive locations were determined through an Early Environmental Screening and a Desktop Environmental Scan. The known environmentally sensitive locations along the study corridor are listed in Table 4.2
Environmental and Community Resources and mapped in Figure 4.2 Environmentally
Sensitive Locations. Additionally, the locations are labeled in the Conceptual Plans.

### 4.2.1 Early Environmental Screening (EES)

In preparation of Transportation Planning Reports (TPR), the Tennessee Department of Transportation (TDOT) has introduced an Early Environmental Screening (EES) process for the project study area. By screening the latest available Geographic Information Systems (GIS) environmental data during the early stages of project planning, TDOT and the public will be better prepared to anticipate potential environmental issues and mitigation requirements. This screening process involves using GIS to assess environmental data as it spatially relates to the project's Area of Potential Effect (APE). In broad terms, the GIS environmental data reviewed in this TPR include the following layers:

- Archaeological/Historic Architecture - Historic properties and cemetery sites;
- Community Impacts - Sensitive community populations;
- Ecology - Scenic Waterways, Natural Areas, large wetlands, protected species (bat, aquatic, terrestrial, plants);
- Hazardous Substances/Geology - Hazardous substance sites, pyritic rock/geotechnical, caves; and,
- Parks \& Public Land - parks (federal/state/local), public land/buildings, railroads, wildlife management areas.
- As of the publication of this document, the GIS data within each layer was up to date relevant to date of its publication. This data will be updated as part of the ongoing project development process.
The TDOT EES Maps are provided in the Appendix.


### 4.2.2 Desktop Environmental Scan

In addition to TDOT's GIS based EES, a desktop environmental/community resources scan was performed as part of this TPR. Sources utilized in this environmental/community resources scan include the following: Environmental Protection Agency Envirofacts, United States Geological Survey Topographic Mapping (Spring Hill, Leipers Fork, Carters Creek, Bethesda, and Franklin) aerial photography, TDOT maps, and the National Register of Historic Places.

Table 4.2 Environmental and Community Resources

| Churches |  |
| :--- | :--- |
| Name | Location |
| Spring Hill Church of Christ | 5351 Main Street, Spring Hill, TN, 37174 |
| Spring Hill Presbyterian <br> Church | 5344 Main Street, Spring Hill, TN 37174 |
| Grace Episcopal Church | 5291 Main Street, Spring Hill TN 37174 |
| Spring Hill United Methodist <br> Church | 5286 Main Street, Spring Hill, TN 37174 |
| Mt. Hope Baptist Church | 5276 Main Street, Spring Hill, TN 37174 |
| Wesley Chapel United <br> Methodist Church | 511 McClemore Avenue, Spring Hill, TN 37174 |
| New Town Church of Christ | 2615 Duplex Road, Spring Hill, TN 37174 |
| First Baptist Church | 5219 Main Street, Spring Hill, TN 37174 |
| Jehovah's Witness Church | 4002 O'Hallorn Drive, Spring Hill, TN 37174 |
| St. Mark United Primitive <br> Baptist Church | 518 Maury Hill Street, Spring Hill, TN 37174 |
| Faith Lutheran Church | 4738 Columbia Pike, Thompson's Station, TN 37179 |
| Thompson's Station Baptist <br> Church | 2604 Thompson's Station Road, Thompson's Station, <br> TN 37179 |
| Thompson's Station Church of <br> Christ | 4721 Columbia Pike, Thompson's Station, TN 37179 |
| Spring Meadow Baptist <br> Church | 4256 Columbia Pike, Franklin, TN 37064 |
| New Birth Seventh Day <br> Adventist Church | 1336 Coleman Road, Franklin, TN 37064 |
| West Harpeth Primitive <br> Baptist Church | 4141 Columbia Pike, Franklin, TN 37064 |


| Cemeteries |  |
| :--- | :--- |
| Name | Location |
| Spring Hill Memorial Park | 5239 Main Street, Spring Hill, TN 37174 |
| Williamson Memorial Gardens | 3009 Columbia Pike, Franklin TN, 37064 |
| Patton Cemetery | Thompson's Station Road W, Thompson's Station, TN <br> 37179 |
| Evergreen Cemetery | Evergreen Road, Spring Hill, TN 37174 |
| Buford Cemetery | Columbia Pike, Thompson's Station, TN 37179 (south <br> of Critz Lane) |
| Dodson Cemetery | Clayton Arnold Road, Thompson's Station, TN 37179 |
| Baugh Cemetery | Columbia Pike, Franklin, TN 37064 |
| Harrison Cemetery | Columbia Pike, Franklin, TN 37064 |
| Hood Cemetery | Snowbird Hollow Road, Franklin, TN 37064 |
| Turner Cemetery | Coleman Road, Franklin, TN 37064 |
| Harrison Cemetery | Coleman Road, Franklin, TN 37064 |
| Mathis Cemetery | Henpeck Lane, Franklin, TN 37064 |
| Sweeney Cemetery | Columbia Pike, Franklin, TN 37064 |


| Schools |  |
| :--- | :--- |
| Name | Location |
| Spring Hill Elementary | 5359 Main Street, Spring Hill TN, 37174 |
| Heritage Middle School | 4803 Columbia Pike, Thompson's Station, TN 37179 |
| Heritage Elementary School | 4801 Columbia Pike, Thompson's Station, TN 37179 |
| Independence High School | 1776 Declaration Way, Thompson's Station, TN 37179 |
| Winstead Elementary School | 4080 Columbia Pike, Franklin, TN 37064 |


| Environmental Sites |  |
| :--- | :--- |
| Name | Location |
| Ryder Truck Rental (EPA <br> Hazardous Waste) | 3639 Royal Park Boulevard, Spring Hill, TN 37174 |
| Spring Hill Water Treatment <br> Plant (EPA Water Discharger) | 199 Town Center Parkway, Spring Hill, TN 37174 |
| Jimmy's Custom Cleaners <br> and Alterations (EPA Air <br> Emissions) | 5317 Main Street, Spring Hill, TN 37174 |
| CVS (EPA Hazardous Waste) | 4805 Columbia Pike, Thompson's Station, TN 37179 |
| Henry P. Minton Family <br> Trust/Batey Property (EPA <br> Hazardous Waste) | 2762 Critz Lane, Thompson's Station Road, <br> Thompson's Station, TN 37179 |


| Parks \& Recreation |  |
| :--- | :--- |
| Name | Location |
| Thompson's Station Park | Thompson's Station Road W, Thompson's Station, TN <br> 37179 |
| Winstead Hill Park | US 31, Franklin, TN 37064 (across from Mack Hatcher <br> Parkway) |


| Historic Places |  |
| :--- | :--- |
| Name | Location |
| Ferguson Hall (aka <br> Tennessee Orphan's Home, <br> Martin Cheairs House, <br> Branham and Hughes Military <br> Academy) |  |
| Spring Hill High School | Spring Hill |
| Cleburne Jersey Farm (aka <br> Campbell Farm) | 2319 Sugar Ridge Road, Spring Hill, TN 37174 Spring Hill, TN 37174 |
| Ewell Farm | Depot Lane, Spring Hill, TN 37174 (1 mile west of US <br> $31)$ |
| Grace Episcopal Church | 5291 Main Street, Spring Hill, TN 37174 |
| Rippavilla | 5700 Main Street, Spring Hill, TN 37174 |
| Ritter-Morton House | McLemore Ave, Spring Hill, TN 37174 |
| Spring Hill Battlefield | Junction of Kedron and Old Kedron Roads, Spring Hill, <br> TN 37174 |
| Spring Hill Presbyterian <br> Church | 5344 Main Street, Spring Hill, TN 37174 |
| St. Mark United Primitive <br> Baptist Church | 518 Maury Hill Street, Spring Hill, TN 37174 <br> White Hall |
| Buford Spencer House | US 31, Thompson's Station, TN 37179 (1/2 mile south <br> of Critz Lane) |
| Thomas Critz House | Critz Lane, Thompson's Station, TN 37179 (1 mile east <br> of US 31) |
| Franklin Battlefield | South of Franklin along US 31 <br> HS 31, Franklin TN, 37064 (between railroad and Mack <br> Hatcher, west side, with a red roof) |
| Homestead Manor | US 31, Thompson's Station, TN 37179 (south of SR <br> $840 ~ w h e r e ~ t h e ~ r a i l r o a d s ~ s p l i t s ~ f r o m ~ U S ~ 31) ~$ |
| James P Johnson House | US 31, Thompson's Station, TN 37179 (3/10 of a mile <br> south of W. Harpeth Road) |
| Henry Pointer House | US 31, Thompson's Station, TN 37179 |
| Thompson's Station Bank | Thompson' Station Road, Thompson's Station, TN <br> 37179 |
| James Giddens House | Thompson's Station, TN 37179 |
| Winstead Hill | US, Franklin, TN 37064 |
|  |  |


| Utilities | Location |
| :--- | :--- |
| Name | Crosses US 31 south of Franklin |
| Columbia Gulf Pipeline |  |



Figure 4.2 Environmentally Sensitive Locations

### 4.3 EXISTING TRANSPORTATION CONDITIONS

SR 6 is a critical arterial in south central Tennessee. Within the study limits of this TPR, SR 6 runs parallel to Interstate 65 and serves regional travel demand between Columbia, Spring Hill, Thompson's Station, and Franklin. Considerable commuter driving patterns occur between these cities, creating congestion during peak travel times. SR 6 , along with Interstate 65 and SR 106 (Lewisburg Pike) are the primary north-south routes in the region. When traffic incidents occur on Interstate 65, SR 6 is a primary detour route.

SR 6 is the primary north-south arterial through the City of Spring Hill and Town of Thompson's Station, and one of the primary routes through the City of Franklin. The speed limit varies between 35 and 55 mph and the cross section varies from two (2) lanes to five (5) lanes along the route. There are nine (9) traffic signals and four (4) school zones located within the study area. The primary deficiency along the route is congested traffic operations. The existing geometric conditions and deficiencies of SR 6 are discussed in more detail on the following pages. The roadway segments are discussed from south to north and are based upon the cross section and functional classification of SR 6 within the study area. Photos along the route are provided in Figures 4.3.1 through 4.3.6. The existing geometric conditions of SR 6, along with the Segment ID's, are mapped in Figure 4.3.7 Existing Geometric Conditions. The segment ID's are also labeled in the Conceptual Plan Sheets.

Segment ID \#1, Three (3) Lane Urban Other Principal Arterial, L.M. 32.47 to L.M. 33.31 Maury County

The City of Spring Hill's CBD is located in this 0.84 mile long segment of SR 6. This segment extends from Kedron Road to the Williamson/Maury County Line, near Witt Hill Drive. The speed limit of this segment is 35 mph . The 15 mph Spring Hill Elementary School Zone is located in this segment. Three (3) traffic signals are located in this segment. The entire segment is three (3) lanes wide and includes two (2) travel lanes and a center two way left turn lane (TWLTL).

The school zone and numerous private and commercial entrances adversely impact traffic operations in this area. The offset intersection created by SR 6 with Duplex Road (SR 247) and Beechcroft Road (SR 247) also affects traffic operations in this segment.


Figure 4.3.1 Segment ID \#1 Рhotos (July 2007)

Segment ID \#2, Two (2) Lane Urban Other Principal Arterial, L.M. 0 to L.M. 2.49 Williamson County

This 2.49 mile long segment of SR 6 is within Spring Hill's City Limits and includes rural and dense suburban development, including commercial development. This segment extends from the Williamson/Maury County Line, near Witt Hill Drive, to Thompson's Station's City Limits, north of Buckner Road. The speed limit of this segment is 45 mph . The 20 mph Heritage Elementary and Middle School Zone is located in this segment. Two (2) traffic signals are located in this segment. The cross section is primarily two (2) lanes wide, with left turn lanes at most major intersections or developments.
The 0.9 mile-long section between Cemetery Road/O'Hallorn Drive and Buckner Road is effectively a three (3) lane section, as the left turn lanes are continuous through this commercial area.

The school zone adversely impacts traffic operations in this area. Representatives of the City of Spring Hill also noted that the intersections of SR 6 with Miles Johnson Parkway and Bellshire Way need to be signalized. It was also noted that turn lane improvements were needed at the intersections of SR 6 with Campbell Station Parkway and Buckner Road.

L.M. 0.43 SR 6

L.M. 1.35 SR 6 Near Campbell Sta. Pkwy.

L.M. 1.79

Figure 4.3.2 Segment ID \#2 Photos (July 2007)

Segment ID \#3, Two (2) Lane Rural Minor Arterial, L.M. 2.5 to L.M. 4.75 Williamson County
This 2.25 mile-long segment of SR 6 is within the Town of Thompson's Station's Urban Growth Boundary. This segment extends from Thompson's Station's City Limits, north of Buckner Road, to south of SR 840, north of Critz Lane. The development along the route is primarily rural, with small commercial developments located adjacent to the intersection with Thompson's Station Road. The speed limit of this section is 45 mph . One (1) traffic signal is located in this segment. The cross section is two (2) lanes wide, with left turn lanes located at the intersection of SR 6 with Thompson's Station Road.

Representatives of the Town of Thompson's Station noted that turn lane improvements are needed at the intersection of SR 6 with Thompson's Station Road to improve traffic operations. Another geometric deficiency noted by the Town is the intersection of SR 6 with Critz Lane. Critz Lane meets SR 6 at a sharp skew. It is desired to realign Critz Lane to improve the skew.

L.M. 3.04 SR 6 at Thompson's Station Rd.

L.M. 4.41 SR 6

Figure 4.3.3 Segment ID \#3 Рhotos (July 2007)

This 0.79 mile-long segment of SR 6 primarily consists of the functional area of the interchange of SR 6 with SR 840.
This segment extends from south of SR 840, north of Critz Lane, to Tollgate Road. The development along the route is a mix of rural and high density residential. The speed limit of this section is 45 mph . The 20 mph Independence High School Zone is located in this segment. One (1) traffic signal is located in this segment. The cross section is five (5) lanes wide, with a two way center left turn lane (TWLTL). There are no observed geometric deficiencies in this segment. However, the school zone does adversely impact traffic operations during AM peak periods.

L.M. 4.97 SR 6 at SR 840

Figure 4.3.4 Segment ID \#4 Рноto (July 2007)

## Segment ID \#5, Two (2) Lane Rural Minor Arterial, L.M. 5.56 to L.M. 8.67 Williamson County

This 3.11 mile-long segment of SR 6 is rural with low density residential developments. This segment extends from Tollgate Road to south of Coleman Road. The Goose Creek Bypass Intersection is located in this segment. The northern end of this segment is at Nashville's urban boundary near Franklin's City Limits. The speed limit of this section varies from 45 to 55 miles per hour. No traffic signals are located in this segment. The cross section is two (2) lanes wide.

Local officials noted the bridge over the West Harpeth River has limited sight distance. This bridge is located just south of the Goose Creek Bypass Intersection with SR 6. Local officials also noted the odd configuration of the Goose Creek Bypass intersection with SR 6 adversely affects safety and traffic operations in the area.

L.M. 5.79 SR 6 at Goose Creek Bypass

L.M. 6.69 SR 6

Figure 4.3.5 Segment ID \#5 Photos (July 2007)

Segment ID \#6, Two (2) Lane Urban Other Principal Arterial, L.M. 8.68 to L.M. 10.03 Williamson County

This 1.35 mile-long segment of SR 6 is primarily rural with low density residential developments. This segment extends from south of Coleman Road to Mack Hatcher Parkway. The speed limit of this section is 40 miles per hour. The 20 mph Winstead Elementary School Zone is located in this segment. One (1) traffic signal is located in this segment. The cross section is two (2) lanes wide. SR 6 bridges over the CSX Railroad tracks in this segment. This bridge has a sharp vertical curve and limited shoulders.

The school zone adversely impacts traffic operations in this area during AM peak periods. The offset intersection created by SR 6 with Coleman Road and Henpeck Lane also affects traffic operations in this segment. It was noted by local officials that the right turn lane along SR 6 northbound approaching Mack Hatcher Parkway is deficient in length.

L.M. 8.79 SR 6 over CSX Railroad

L.M. 9.96 SR 6 at Mack Hatcher Pkwy.

Figure 4.3.6 Segment ID \#6 Рhotos (June 2008)

SR 6 (US 31) TPR
Maury and Williamson Counties


Figure 4.3.7 Existing Geometric Conditions

### 4.4 CRASH HISTORY

SR 6 crash rates were provided by TDOT from crash data for the years 2002 through 2006.
Table 4.4 SR 6 Crash Data for 2002-2006 tabulates the crash data and Figure 4.4 SR 6
Cumulative Crashes vs. Location provides a visual representation of where the crashes are occurring. SR 6 is classified as an Urban Principal Arterial and as a Rural Minor Arterial within the study area. The crash data is summarized based upon the same roadway classification and cross section characteristics identified in Section 4.3 Existing Transportation Conditions, and compared to the statewide crash rates for similar roadway segments.

As can be seen in Table 4.4, the actual crash rate along SR 6 from Kedron Road to the Williamson County Line is higher than the statewide crash rate, indicating a safety concern. This segment includes the central business district (CBD) of Spring Hill. While the remaining segments of the study area have actual crash rates below the statewide crash rates for similar roadway segments, Figure 4.4 indicates areas within those segments with safety concerns. The limits of these areas are as follows:

The crash rate in Spring Hill's CBD is 1.5 times higher than the statewide average rate.
$61 \%$ of the crashes along the study corridor are rear end crashes, which may indicate congestion as a cause of many crashes

- Campbell Station Parkway to Thompson's Station Road
- The Goose Creek Bypass Area
- The intersection of SR 6 with Mack Hatcher Parkway

The crash data shows that $61 \%$ of the crashes along SR 6 are rear end crashes. This may indicate congestion as a cause of many crashes due to stop and go traffic. No other significant crash patterns were observed. Pavement conditions do not appear to have a significant impact on crashes, as $87 \%$ of the crashes occurred in fair weather conditions.

Table 4.4 SR 6 CRASH DAta For 2002-2006

| ID | Location/ Description | Roadway Classification | State- <br> Wide <br> Crash <br> Rate | Actual Crash Rate |
| :---: | :---: | :---: | :---: | :---: |
| 1 | From Kedron Rd. to Maury/Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 2.65 | 4.02 |
| 2 | From Maury/Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 2.34 | 1.77 |
| 3 | From Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 1.70 | 1.24 |
| 4 | From south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 1.05 | 0.95 |
| 5 | From Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 1.70 | 1.26 |
| 6 | From Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 2.34 | 2.06 |



TAbLE 4.4 SR 6 CRASH DATA FOR 2002-2006 (Continued)

| ID | Location/ Description | Length of Segment (Miles) | Total Crashes | Fatal Crashes | Injury <br> Crashes | Property Damage Only Crashes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | From Kedron Rd. to Maury/Williamson County Line (L.M. 32.47 to 33.31) | 0.84 | 103 | 1 | 25 | 77 |
| 2 | From Maury/Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2.49 | 151 | 0 | 51 | 100 |
| 3 | From Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2.25 | 102 | 3 | 19 | 80 |
| 4 | From south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 0.79 | 28 | 0 | 10 | 18 |
| 5 | From Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 3.11 | 110 | 1 | 40 | 69 |
| 6 | From Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 1.35 | 88 | 1 | 31 | 56 |
| Total |  |  | 582 | 6 | 176 | 400 |
| Percentage |  |  |  | 1\% | 30\% | 69\% |

TAbLE 4.4 SR 6 CRASH DATA FOR 2002-2006 (CONTINUED)



Figure 4.4 SR 6 Crash Data for 2002-2006

### 4.5 MULTIMODAL FACILITIES

Sidewalks are present along SR 6 in Spring Hill's CBD. Sidewalks are not present elsewhere along SR 6 in the study area. Bike lanes are not present along SR 6 . Thompson's Station and Spring Hill are developing greenways near the study area.

Several transit agencies and other modal options offer services in or near the study area. The services are described below.

Franklin Transit Authority (FTA): Trolley services are offered in Franklin's CBD between 8 am and 6 pm Monday through Friday and Saturday from 9 am to $6: 30 \mathrm{pm}$. Fares are typically one dollar (\$1) per trip.

Transportation Management Association (TMA) Group: The TMA Group is a public-private partnership committed to providing environmentally friendly, affordable, reliable, and safe mobility options for Middle Tennesseans. Services offered include a van pool program.

Regional Transportation Authority (RTA): The mission of RTA is to offer the citizens of Middle Tennessee choices and alternatives on how they commute to work each day and to coordinate local regional transportation services. Services offered include rideshare and vanpool programs and commuter bus service from Spring Hill and Franklin to Nashville.

South Central TN Development District (SCTDD) Rural Public Transportation: SCTDD offers rural public transportation in Maury County.

Mid-Cumberland Human Resource Agency (MCHRA) Rural Transit System: MCHRA offers curb-to-curb rural public transportation service between Williamson and Davidson County for a fare of six dollars (\$6) each way.

CSX Corporation Rail: CSX Corporation railroad tracks are located parallel to SR 6 throughout the study corridor. The line offers freight service. It is desired by local officials for the rail line to eventually provide commuter rail service. There are currently no plans for this to occur. The Radnor Yard railroad yard, located in south Nashville, creates an impediment to commuter rail utilizing this track to Nashville's CBD.

Airports: The Maury Regional Airport is located approximately twenty (20) miles southwest of the southern terminus of the study area in Mount Pleasant, TN. Charter services for both passengers and freight are available at this airport. The nearest commercial airline services are available at Nashville International Airport, approximately thirty (30) miles northeast of the northern terminus of the study area.

### 5.0 PROPOSED IMPROVEMENTS

Two improvement options are included in this TPR, the No Build Option and the Widen Along the Existing Alignment Option. The No Build Option provides no improvements and serves as a baseline option against which other options are compared. The Widen Along the Existing Alignment Option will improve the existing route from two (2) travel lanes to four (4) travel lanes. A center two way left turn lane (TWLTL) is anticipated along the majority of the route. This option resembles the improvements listed in the Nashville Area MPO's Long Range Transportation Plan (LRTP).

In discussions with the MPO and local officials, it is not desired for the exact improvements along SR 6 listed in the LRTP to be constructed. Different median options, travel lane options, and termini should be considered to limit impacts to environmentally sensitive areas along the route and to achieve logical termini. The MPO noted that new options are welcome. Several additional options were therefore studied as part of this TPR, including bypass options, reduced travel lane options, and spot improvements. Many of these options, including the bypass options, are significantly different from the improvements listed in the LRTP. These options need additional coordination with the MPO's planning process, which may include being subject to public review and comment. Furthermore, the additional options, in their current form, do not meet the purpose and need of the project. Further refinement of these additional options through the MPO's planning process could lead to them becoming viable improvement options. The additional improvement options studied are assessed in the Options Requiring Additional Study Appendix.

### 5.1 MEASURES OF EFFECTIVENESS

Several measures of effectiveness are utilized in this TPR to assess the operational conditions of SR 6 for both the existing and proposed conditions. These measures of effectiveness are level of service, volume to capacity ratio, average travel speed, and travel time. A definition of these measures is provided in the following text.

### 5.1.1 Level of Service

Level of Service (LOS) is a qualitative measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience. LOS range from A to $F$, with LOS A representing the best operating conditions and LOS F the worst. Each LOS represents a range of operating conditions and the driver's perception of those conditions. Please refer to Table 5.1 LOS Table for a description of each LOS.

The LOS of SR 6 was assessed utilizing the methodology outlined in the Highway Capacity Manual 2000 (HCM) Urban Streets, Two-Lane Highways, and Multilane Highways Chapters. The calculations were performed with the Highway

It should be noted that the future use of the GM (Saturn) automotive plant will significantly alter traffic volumes in the area. The plant, which is located just south of the study corridor, was idled at the end of 2009. The traffic projections utilized in this report were performed prior to the opening of Miles Johnson Parkway and the idling of the GM plant.

Capacity Software (HCS+T7F Version 5.3). HCS+ is developed and maintained as an implementation of the HCM procedures. HCS+ calculations assign a LOS along route segments with similar geometric and traffic characteristics.

The Arterials Planning Level Analysis routine in the HCS+ software was utilized to analyze SR 6 between Kedron Road and Thompson's Station's City Limits (Segments 1 and 2). This routine was chosen for these segments due to their urban nature, their low posted speed limit, and the presence of several traffic signals. The Two-Lane Highway or Multilane Highway Operational Analysis routines were utilized to analyze the remainder of the route (Segments 3 through 6). These highway routines were chosen for these remaining segments due to their more rural nature, higher posted speed limits, and fewer traffic signals. The segments are labeled in the Conceptual Plan Sheets. The Arterials and Highway routines have different inputs, including the existence of turn lanes and passing zones, and assign LOS differently. Therefore, the LOS are not consistent between Segments 2 and 3 because of the different HCS+ routines utilized. This is not an error and can be attributed to the HCS+ assigning different driver expectancies between the urban and more rural areas.

Table 5.1 LOS Table

| LOS | Traffic Flow Conditions | Representative Photo |
| :--- | :--- | :--- |
| A | Free flow operations. Vehicles are almost <br> completely unimpeded in their ability to <br> maneuver with the traffic stream. The <br> general level of physical and psychological <br> comfort provided to the driver is high. |  |
| B | Reasonable free flow operations. The ability <br> to maneuver within the traffic stream is only <br> slightly restricted and the general level of <br> physical and psychological comfort provided <br> to the driver is still high. |  |
| C | Flow with speeds at or near free flow speeds. <br> Freedom to maneuver within the traffic <br> stream is noticeably restricted and lane <br> changes require more vigilance on the part of <br> the driver. The driver notices an increase in <br> tension. | Speeds decline with increasing traffic. <br> Freedom to maneuver within the traffic <br> stream is more noticeably limited. The driver <br> experiences reduced physical and <br> psychological comfort levels. |
| F | F |  |
| At lower boundary, the facility is at capacity. |  |  |
| is little room to maneuver. The driver |  |  |
| experiences poor levels of physical and |  |  |
| psychological comfort. |  |  |
| Operations are volatile because there are |  |  |
| is little room to maneuver. The driver |  |  |
| experiences poor levels of physical and |  |  |
| psychological comfort. |  |  |

### 5.1.2 Volume to Capacity Ratio \& Congestion Reduction

Unlike LOS, which is a qualitative measure, the volume to capacity ratio $(\mathrm{v} / \mathrm{c})$ is a quantitative measure. The $\mathrm{v} / \mathrm{c}$ ratio is reported to demonstrate the magnitude of congestion for the options included in this TPR. The v/c ratio demonstrates how much reserve capacity along a roadway segment is available, or how much the segment is overcapacity. A v/c ratio near or above " 1 " indicates a roadway experiences congestion.

### 5.1.3 Average Travel Speed

Average travel speed is calculated in the LOS analysis. Speed, or its reciprocal travel time, is an important measure of congestion and the quality of the traffic service provided to the motorist.

### 5.1.4 Travel Time

The travel time along a route can be calculated by dividing the distance of the route by the average travel speed. As discussed above, travel time is an important measure of the quality of the traffic service provided to the motorist. In addition to the travel speed improvements associated with reduced congestion, travel time demonstrates the time savings of shorter route options.

### 5.2 NO BUILD OPTION

The No Build Option provides no improvements and serves as a baseline option against which all other options are compared. For a description of the geometric conditions associated with the No Build Option, please refer to Section 4.3 Existing Transportation Conditions. Because improvements are not being constructed, there is no cost associated with the No Build Option. All local officials contacted for this TPR along the route acknowledge a need for improvements and therefore do not support a No Build Option.

For the No Build Option, the HCS's analysis calculates LOS ranging from A to F along SR 6 through the year 2014 and from B to F through the year 2034. The LOS calculations do not take into effect the diminished traffic operations caused by the four (4) school zones along the route. A summary of the LOS calculations for the No Build Option is provided in schematic form in Figures 5.2.1 and 5.2.2 and in table form in Table 5.2. The LOS are reported for the years 2014 and 2034.

The analysis indicates that the existing one (1) travel lane in each direction is generally not adequate for the projected traffic volumes.

For the No Build Option in the year 2014, the volume to capacity ratio (v/c) of SR 6 is calculated to range from 0.40 to 1.02 , with a weighted average of 0.69 . In 2034, the v/c ranges from 0.57 to 1.58 with a weighted average of 0.91 . The average was weighted based upon the length of each segment analyzed. A v/c ratio near or above " 1 " indicates a roadway experiences congestion. Therefore, the volume to capacity ratios reported indicates that the existing one (1) travel lane in each direction is generally not adequate for the projected volumes. A summary of
the v/c calculations for the No Build Option is provided in table form in Table 5.2. The v/c are reported for the years 2014 and 2034.

The posted speed limit ranges from 35 to 55 mph along SR 6 within the study area. For the No Build Option in the year 2014, travel speeds along the route are calculated by the HCS to range from 14.6 mph to 46.0 mph , with a weighted average of 27.0 mph . In 2034, the travel speed ranges from 3.5 mph to 46.0 mph with a weighted average of 15.8 mph . The average was weighted based upon the length of each segment analyzed. The weighted average of the speed limit along the route is 45.5 mph . The calculated average route speed is $59 \%$ and $35 \%$ of the posted speed limit in the years 2014 and 2034, respectively. The Nashville Area MPO's Congestion Management Process notes that if the average route speed is $70 \%$ or less of the free flow speed, the roadway is congested. Therefore, for the No Build Option, SR 6 is congested in both 2014 and 2034. A summary of the travel speed calculations for the No Build Option is provided in table form in Table 5.2. The travel speeds are reported for the years 2014 and 2034.

The existing SR 6 Corridor (No Build Option) between Kedron Road to the south and Mack Hatcher Parkway to the north is 10.9 miles in length. For the No Build Option in the year 2014, the travel time along SR 6 is calculated to be 24.1 minutes. In 2034, the travel time is calculated to be 41.3 minutes.


Figure 5.2.1 No BuILD OPTION 2014 LOS


Figure 5.2.2 No Build Option 2034 LOS

## SR 6 (US 31) TPR

Maury and Williamson Counties
Table 5.2 No Build Option LOS Table

|  | From |  | To |  | Dist.(mi) | Roadway Data |  |  | 2014 |  |  |  |  | 2034 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ID | L.M. | Description | L.M. | Description |  | Posted <br> Speed <br> (mph) | \# of Signals | Cross- <br> Section Type | $\begin{aligned} & \text { ADT } \\ & \text { (vpd) } \end{aligned}$ | LOS | v/c | Calc. <br> Speed <br> (mph) | Travel <br> Time <br> (min.) | $\begin{aligned} & \text { ADT } \\ & \text { (vpd) } \end{aligned}$ | LOS | v/c | Calc. <br> Speed <br> (mph) | Travel <br> Time <br> (min.) |
| 1 | 32.47 | Kedron Road | 33.31 | County Line | 0.84 | 35 | 3 | 3-Lane | 22,400 | D | 0.92 | 14.6 | 3.5 | 36,700 | F | 1.50 | 3.5 | 14.4 |
| 2A | 0.00 | County Line | 1.30 | Cemetery Road | 1.30 | 45 | 2 | 2-Lane | 25,000 | C | 1.02 | 26.4 | 3.0 | 36,700 | F | 1.58 | 11.4 | 6.8 |
| 2B | 1.30 | Cemetery Road | 2.17 | Buckner Road | 0.87 | 45 |  | 3-Lane | 25,000 | A | 0.51 | 35.5 | 1.5 | 36,700 | B | 0.75 | 34.1 | 1.5 |
| 2C | 2.17 | Buckner Road | 2.49 | Thompson's Station City Limits | 0.32 | 45 |  | 2-Lane | 25,000 | A | 0.54 | 35.4 | 0.5 | 36,700 | B | 0.79 | 33.8 | 0.6 |
| 3 | 2.49 | Thompson's Station City Limits | 4.75 | Between Critz Lane and SR-840 | 2.26 | 45 | 1 | 2-Lane | 34,000 | F | 0.92 | 21.1 | 6.4 | 36,700 | F | 0.99 | 19.4 | 7.0 |
| 4 | 4.75 | Between Critz Lane and SR-840 | 5.55 | Tollgate Road | 0.80 | 45 | 1 | 5-Lane | 34,000 | C | 0.52 | 46.0 | 1.0 | 36,700 | C | 0.57 | 46.0 | 1.0 |
| 5A | 5.55 | Tollgate Road | 6.60 | near McLemore Road | 1.05 | 45 | 0 | 2-Lane | 24,000 | E | 0.65 | 29.5 | 2.1 | 27,600 | E | 0.74 | 27.1 | 2.3 |
| 5B | 6.60 | near McLemore Road | 8.67 | near Coleman Road | 2.07 | 55 | 0 | 2-Lane | 15,000 | D | 0.40 | 44.7 | 2.8 | 18,300 | E | 0.49 | 42.9 | 2.9 |
| 6 | 8.67 | near Coleman Road | 10.03 | Mack Hatcher Parkway | 1.36 | 40 | 1 | 2-Lane | 20,800 | E | 0.56 | 24.5 | 3.3 | 32,200 | F | 0.87 | 17.2 | 4.7 |
| 上 $=10.87$ |  |  |  |  |  |  |  |  | Avg= 0.69 |  |  |  | 24.1 | Avg= 0.91 |  |  | $\Sigma=$ | 41.3 |
| Notes: |  | Sections 1 \& 2 analyzed with HCS+ Arterials Planning Analysis Sections 3-6 analyzed with HCS+ Two Lane and Multi Lane Highway Analysis |  |  |  |  |  |  |  | Average Speed: <br> Avg. Post. Spd.: <br> \% Spd vs Post.: |  |  | $\begin{aligned} & 27.0 \\ & 45.5 \\ & 59 \% \end{aligned}$ |  | Avera Avg. \% Sp | ge Sp <br> Post. <br> d vS | peed: <br> Spd.: <br> Post.: | $\begin{aligned} & 15.8 \\ & 45.5 \\ & 35 \% \end{aligned}$ |

### 5.3 WIDEN ALONG THE EXISTING ALIGNMENT OPTION

Widening SR 6 through Spring Hill's CBD does not have the support of local officials due to how it would negatively impact:

- numerous religious properties
- historic properties
- commercial properties
- Impact their walkable community

This option will improve the existing route from two (2) travel lanes to four (4) travel lanes between Kedron Road and Mack Hatcher Parkway. A center two-way left turn lane (TWLTL) is desired along the majority of the route. This option is similar to the improvements listed in the Nashville Area MPO's LRTP. The termini are altered slightly from the LRTP's termini of Old Kedron Road to the south and Henpeck Lane to the north. The termini are altered for clarification to the south and to form a logical terminus at Mack Hatcher Parkway to the north. Additionally, the LRTP calls for a five (5) lane cross section within the City of Spring Hill's City Limits and a four (4) lane cross section outside of Spring Hill's City Limits. As noted previously, a center TWLTL is desired where feasible along the majority of the route by local officials.

Table 5.3.1 Widen Along the Existing Alignment Option vs. LRTP Improvements

| Feature | Widen Along the Existing Alignment Option | Improvements as listed in the LRTP (Project \#'s 6001 and 6022) | Reason for Modification |
| :---: | :---: | :---: | :---: |
| Northern Terminus | Mack Hatcher Parkway | Henpeck Lane | The LRTP's northern terminus at Henpeck Lane is not logical. It would create a 1.25 mile long two (2) travel lane cross section between Henpeck Lane and Mack Hatcher Parkway. |
| Southern Terminus | Kedron Road | Old Kedron Road | The LRTP's southern terminus of Old Kedron Road is likely a misprint. Old Kedron Road does not intersect SR 6. The existing four (4) travel lane cross section ends at Kedron Road, and is therefore a logical terminus. |
| Cross Section | Allow for flexibility in location of a TWLTL. | Notes five lanes inside Spring Hill's City Limits and four lanes outside Spring Hill's City Limits. | Modified to allow local stakeholders and the NEPA process to provide input on the location of a TWLTL. |

In addition to improving SR 6 from two (2) travel lanes to four (4) travel lanes, the following improvements are recommended to be incorporated into the Widen Along the Existing Alignment Option. These improvements were determined through coordination with local officials.

- Realign the following offset intersections with SR 6 to form traditional four (4) legged intersections:
- SR 247 (Duplex Road and Beechcroft Road) located in Spring Hill
- Coleman Road and Henpeck Lane located just south of Franklin
- Construct turn lane improvements at the following locations:
- Campbell Station Road located in Spring Hill
- Thompson's Station Road located in Thompson's Station
- Mack Hatcher Parkway located in Franklin
- Signalize warranted intersections along SR 6
- Realign Critz Lane at its intersection with SR-6 located in Thompson’s Station
- Improve sight distance along SR 6 at the bridge over the West Harpeth River located between SR 840 and the Goose Creek Bypass (SR 248). This can be accomplished by improving the shoulders on the bridge.
- Investigate a roundabout at the Goose Creek Bypass (SR 248) intersection with SR 6.
- Improve the grade and shoulders of the SR 6 Bridge over the CSX Corporation Railroad Tracks located just south of Franklin.

The Widen Along the Existing Alignment Option does not have the support of the City of Spring Hill due to the impact to their central business district (CBD). The existing R.O.W. within the CBD of Spring Hill is approximately sixty (60) feet wide. Improvements to SR 6 within the CBD will require impacts to numerous religious, historic, and commercial properties. Furthermore, the City wishes to revitalize their downtown and maintain their walkable community. They envision a future streetscape project that will maintain the existing three (3) lane cross section.

Example photographs of the development within Spring Hill's CBD are provided in Figures
5.3.1 and 5.3.2. A conceptual plan view of the CBD with a five (5) lane cross section is provided in Figure 5.3.3. The conceptual plan utilizes a five (5) lane cross section with twelve (12) foot wide lanes and five (5) foot wide sidewalks. This cross section is modeled after TDOT Standard Drawing RD01-TS-6A. This example cross section has no provisions for bicycle lanes and does not show the potential impacts of slope easements. The proposed R.O.W. is shown one (1) foot behind the sidewalks.


Figure 5.3.1 SR 6 ApPROACHING McLEMORE Ave./ Depot St.


Figure 5.3.2 SR 6 Approaching SR 247

Additionally, the Widen Along the Existing Alignment Option does not have the support of the Town of Thompson's Station. A representative of the Town notes that future improvements along Interstate 65 may alleviate congestion along SR 6 within their town limits, and therefore eliminate the need to widen SR 6 to four (4) travel lanes within the town limits. Concern was expressed that widening SR 6 without improvements to Interstate 65 would increase traffic volumes through the town.

The existing R.O.W. outside of Spring Hill's CBD is approximately sixty-six (66) feet wide, except where it expands at the SR 840 Interchange and Goose Creek Bypass. The narrow R.O.W. will create difficulties for improvements just north of the CBD of Spring Hill, where the land use is heavily developed commercial property, and wherever environmentally sensitive sites are located adjacent to the R.O.W.

The Widen Along the Existing Alignment Option is estimated to cost between \$90 and \$103 million in year 2014 dollars. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate summary is provided in Table 5.3.2. The cost estimate calculations are provided in the Appendix.

For the Widen Along the Existing Alignment Option, the HCS's analysis calculates LOS ranging from A to D along SR 6 through the year 2034. The LOS calculations do not take into effect the diminished traffic operations caused by the four (4) school zones along the route. The LOS calculations utilize the cross section specified in the LRTP, which includes a five (5) lane cross section within Spring Hill's City Limits and a four (4) lane cross section outside of Spring Hill's City Limits. As noted previously, a center TWLTL is desired where feasible along the majority of the route by local officials. Due to the uncertainty of where a TWLTL is feasible or appropriate, the LRTP Cross Section was utilized in the calculations. A summary of the LOS calculations for the Widen Along the Existing Alignment Option is provided in schematic form in Figures 5.3.4 and 5.3.5 and in table form in Table 5.3.3. The LOS are reported for the years 2014 and 2034.

For the Widen Along the Existing Alignment Option in the year 2014, the volume to capacity ratio (v/c) of SR 6 is calculated to range from 0.26 to 0.54 , with a weighted average of 0.40 . In 2034, the v/c ranges from 0.38 to 0.79 with a weighted average of 0.51 . The average was weighted based upon the length of each segment analyzed. The volume to capacity ratios reported indicates that the proposed two (2) travel lanes in each direction are generally
adequate for the projected volumes. A summary of the v/c calculations for the Widen Along the Existing Alignment Option is provided in table form in Table 5.3.3. The v/c are reported for the years 2014 and 2034.

The posted speed limit ranges from 35 to 55 mph along SR 6 within the study area. For the Widen Along the Existing Alignment Option in the year 2014, travel speeds along the route are calculated by the HCS to range from 17.7 mph to 54.5 mph , with a weighted average of 38.7 mph . In 2034, the travel speed ranges from 16.3 mph to 54.4 mph with a weighted average of 37.9 mph . The average was weighted based upon the length of each segment analyzed. The weighted average of the speed limit along the route is 45.5 mph . The calculated average route speed is $85 \%$ and $83 \%$ of the posted speed limit in the years 2014 and 2034, respectively. The Nashville Area MPO's Congestion Management Process notes that if the average route speed is $70 \%$ or less of the free flow speed, the roadway is congested. Therefore, for the Widen Along the Existing Alignment Option, SR 6 is not considered congested for both 2014 and 2034. A summary of the travel speed calculations for the Widen Along the Existing Alignment Option is provided in table form in Table 5.3.3. The travel speeds are reported for the years 2014 and 2034.

The existing SR 6 Corridor (Widen Along the Existing Alignment Option) between Kedron Road to the south and Mack Hatcher Parkway to the north is 10.9 miles in length. For the Widen along the Existing Alignment Option in the year 2014, the travel time along SR 6 is calculated to be 16.8 minutes. In 2034, the travel time is calculated to be 17.2 minutes.

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Figure 5.3.3 Spring Hill CBD Detail (1 of 2)


Figure 5.3.3 Spring Hill CBD Detail (2 of 2)

Table 5.3.2 Widen Along the Existing Alignment Cost Estimate

| ID | Location/ Description | Existing <br> Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Maury/Williamson County Line (L.M. 32.47 to 33.31 ) | 3-Lane Urban Other Principal Arterial | 4-Travel Lane Urban Other Principal Arterial | 0.84 | \$ 13,003,200 | \$ 16,405,200 |
| 2 | SR-6 from Maury/Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.49 | \$ 19,571,400 | \$ 23,630,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ - | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 89,550,000 | \$103,350,000 |

Notes: Low estimate utilizes 4-lane rural cross-section, high estimate utilizes 5-lane urban cross-section.
Total route length is equal to the construction length of 10.9 miles.


Figure 5.3.4 Widen Along the Existing Alignment Option 2014 LOS


Figure 5.3.5 Widen Along the Existing Alignment Option 2034 LOS

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Maury and Williamson Counties
Table 5.3.3 Widen Along the Existing Alignment Option LOS Table


### 6.0 ASSESSMENT OF OPTIONS

### 6.1 TDOT'S SEVEN GUIDING PRINCIPLES

The Tennessee Department of Transportation (TDOT) has adopted seven guiding principles against which all transportation projects are to be evaluated. These guiding principles address concerns for system management, mobility, economic growth, safety, community, environmental stewardship, and fiscal responsibility. These guiding principles are discussed in the following paragraphs as they relate to the options discussed Section 5.0 Proposed Improvements.

### 6.1.1 Guiding Principle 1: Preserve and Manage the Existing Transportation System

The Widen Along the Existing Alignment Option will reduce congestion and optimize service and operational efficiency along SR 6. It will improve the integration of the highway network in South Central Tennessee by providing an alternate route for Interstate 65 to serve sub-regional travel demand between Columbia, Spring Hill, Thompson's Station, and Franklin. Improvements along this route will preserve and manage the existing transportation system by providing a functional route with improved safety.

### 6.1.2 Guiding Principle 2: Move a Growing, Diverse, and Active Population

The Widen Along the Existing Alignment Option will reduce congestion and optimize service and operational efficiency along SR 6 . SR 6 is utilized by a diverse constituency, including commuters, industrial, agricultural, and commercial interests. Within the study limits of this TPR, SR 6 runs parallel to Interstate 65 and serves sub-regional travel demand between Columbia, Spring Hill, Thompson's Station, and Franklin. When traffic incidents occur on Interstate 65, SR 6 is a primary detour route. This interrelation with Interstate 65 supports transportation technology advances through the use of Intelligent Transportation Systems (ITS) by providing an alternate route to Interstate 65.

### 6.1.3 Guiding Principle 3: Support the State's Economy

Improvements to SR 6 are needed to meet the changing social demands of the area. The population of the study area has seen extraordinary growth since 1990. There are several major regional employers located within or near the study limits of this TPR, including the newly relocated North American Headquarters of Nissan in the Cool Springs area of Franklin. This formerly rural area has seen extensive changes in population and land use in recent years and the existing transportation system does not meet the demand.

### 6.1.4 Guiding Principle 4: Maximize Safety and Security

Improvements to SR 6 are needed to improve safety by addressing existing roadway deficiencies along the route. The segment of SR 6 through the City of Spring Hill's CBD has a crash rate that is approximately 1.5 times higher than the statewide rate for similar roadways. High crash rates are also reported between Campbell Station Parkway and Thompson's Station Road, at the Goose Creek Bypass Intersection, and at the Mack Hatcher Parkway Intersection. The crash data shows that $61 \%$ of the crashes along SR 6 are rear end crashes. This may
indicate congestion as a cause of many crashes due to stop and go traffic. Improving traffic flow through the corridor by providing additional travel lanes may therefore improve the safety of the route. Additionally, several locations noted by local officials should be improved while widening SR 6. These locations are discussed in Section 5.3 Widen Along the Existing Alignment Option and include realigning offset intersections, realigning skewed intersections, and improving sight distance at specific locations. Each of these measures should improve safety along SR 6.

### 6.1.5 Guiding Principle 5: Build Partnerships for Livable Communities

Throughout the development of this TPR, TDOT staff has coordinated with local leaders and the Nashville Area MPO to identify their concerns and objectives. Through this coordination it was determined that the improvements listed in the LRTP, which including widening SR 6 through the CBD of Spring Hill and the Town Limits of Thompson's Station, are not desired by local officials. Additional options to the improvements listed in the MPO's LRTP were therefore investigated to address local stakeholder's concerns. These additional options are included in the Appendix. Unfortunately, these additional options, in their current form, do not meet the purpose and need for improvements. Additional study is needed to determine a feasible option with limited impacts to environmentally sensitive sites that addresses local stakeholder's concerns. The public involvement process will continue as mandated by the provisions of the National Environmental Policy Act (NEPA).

### 6.1.6 Guiding Principle 6: Promote Stewardship of the Environment

Several environmentally sensitive locations and community resources within the study area should be avoided, if possible. Those that cannot be avoided should have their impact mitigated. These areas include communities, churches, cemeteries, environmental sites (including landfills), historic places, parks, schools, major streams, utilities, and railroads. The locations are discussed in Section 4.2 Environmental Sites. Additional Options to the improvements listed in the MPO's LRTP were investigated to address local stakeholder's concerns. Additional study is needed to determine a feasible option with limited impacts to environmentally sensitive sites that addresses local stakeholder's concerns.

### 6.1.7 Guiding Principle 7: Promote Financial Responsibility

The need for improvements along SR 6 was determined through Tennessee's statewide longrange multi-modal transportation planning process. This process includes extensive interaction with citizens, local government officials, and the Metropolitan Planning Organizations and the Rural Planning Organizations.

This TPR was initiated by request of the Nashville Area MPO on behalf of Williamson County. The need for improvements was developed through coordination with local agencies, including the Nashville Area MPO, Williamson County, the City of Spring Hill, the Town of Thompson's Station, and the City of Franklin in concurrence with the MPO's planning process. The planning process has demonstrated a need for improvements along this corridor. Preliminary construction cost estimates were prepared. Improvements along the corridor will be evaluated for cost, among other criteria, during the NEPA process. The costs are discussed in Sections 5.0 Proposed Improvements and 6.2 Summary of Proposed Options.

### 6.2 SUMMARY OF PROPOSED OPTIONS

Criteria for choosing an improvement option should incorporate the purpose and need discussed in Section 3.0 Purpose and Need for Improvements and should address TDOT's Seven Guiding Principles against which all transportation projects are to be evaluated. A summary of the No Build Option and Widen Along the Existing Alignment Option is provided in Table 6.2. Summary of Proposed Options. Key measures of effectiveness (MOE) for each option are listed. For a detailed discussion of each option, please refer to Section 5.0 Proposed Improvements. The cost for corridor wide improvements is estimated to range between $\$ 90$ and $\$ 103$ million in year 2014 dollars.

## Table 6.2 Summary of Proposed Options

| Criteria |  |  |
| :---: | :---: | :---: |
| TDOT's Seven Guiding Principles |  |  |
| Preserve and Manage the Existing Transportation System | No | Yes |
| Move a Growing, Diverse, and Active Population | No | Yes |
| Support the State's Economy | No | Yes |
| Maximize Safety and Security | No | Yes |
| Build Partnerships for Livable Communities | No | $\mathrm{No}^{1}$ |
| Promote Stewardship of the Environment | No | $\mathrm{No}^{1}$ |
| Promote Financial Responsibility | No | Yes |

## Purpose and Need Criteria

| Improve Regional Mobility | No | Yes |
| :--- | :---: | :---: |
| Reduce Congestion | No | Yes |
| Meet Changing Social Demands | No | Yes |
| Improve Safety | No | Yes |

Additional Criteria

| Length of route (miles) | 10.9 | 10.9 |
| :--- | :---: | :---: |
| Low estimated cost of improvements (millions) - year 2014 <br> dollars | $\$-$ | $\$ 90$ |
| High estimated cost of improvements (millions) - year 2014 <br> dollars | $\$-$ | $\$ 103$ |
| Year 2034 LOS range | B to F | A to D |
| Year 2034 average v/c ratio | 0.91 | 0.51 |
| Year 2034 average v/c ratio improvement or congestion <br> reduction (\%) | $\mathrm{n} / \mathrm{a}$ | $78 \%$ |
| Year 2034 average travel speed (mph) | 15.8 | 37.9 |
| Year 2034 travel time (minutes) | 41.3 | 17.2 |
| Year 2034 travel time (minutes) reduction (\%) | $\mathrm{n} / \mathrm{a}$ | $58 \%$ |
| Is the route considered congested in 2034? | Yes | No |

1. Throughout the development of this TPR, TDOT staff has coordinated with local leaders and the Nashville Area MPO to identify their concerns and objectives. Through this coordination it was determined that the improvements listed in the LRTP are not desired by local officials. Additional study is needed to determine a feasible option with limited impacts to environmentally sensitive sites that addresses local stakeholder's concerns.

## Checklist of Determinants for Location Study

## Location: SR 6, Maury and Williamson Counties

If preliminary field reviews indicate the presence of any of the following facilities or ESE categories, place an " X " in the blank opposite the item. Where more than one alternate is to be considered, place its letter designation in the blank.

1. Agricultural land usage............................................................ X
2. Airport (existing or proposed) $\qquad$
3. Commercial area, shopping center............................................ X
4. Floodplains
5. Forested Land.................................................................... X
6. Historical, archaeological, cultural, or natural landmark $\begin{aligned} & \text { or cemeteries....................................................................................... }\end{aligned}$
7. Industrial park, factory
8. Institutional usage's
a. School or other educational institution............................... X
b. Church or other religious institution................................. X
c. Hospital or other medical facility
d. Public building, e.g., fire station...................................... X
e. Defense Installation........................................................
9. Recreational Usage's
a. Park or recreational area, State Natural Area.................... X
b. Wildlife refuge or wildlife management area
10. Residential Establishment....................................................... X
11. Urban area, town, city or community....................................... $X$

Title 6, low income/minority community
12. Waterway, lake, pond, river, stream, spring, wetland................... X

Permit required: Coast Guard............................................
Section 404............................................. X
Section 10
TVA Section 26a review............................. X
NPDES................................................... X
Aquatic Resource Alteration Permit............ X
Class V Injection Wells...............................
13. Location coordinated with local officials..................................... X
14. Railroad Crossings
15. Hazardous Material Site

Underground Storage Tanks - U.S.T.)...................................... X
16. Other

Traffic Schematic


Tennessee Department of Transportation
Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 1, L.M. 32.47 to L.M. 33.31 | Region: | 3 |
| County: | Maury | Project \# |  |

## Location

| From: | Kedron Road |
| :--- | :--- |
| To: | Williamson County Line |


| Parameter |
| :--- |
| 2014 AADT 22,400 <br> 2034 AADT 36,700 <br> Percent Trucks (DHV) $3 \%$ <br> DHV (8\% AADT 2034) 2,940 <br> Functional Classification Urban Other Principal Arterial <br> Minimum Design Speed 40 mph <br> Access Control N/A <br> Minimum Radius 565 ft. <br> Maximum Grade $7 \%$ <br> Minimum Stopping Sight Distance 305 ft. <br> Surface Width To be determined in the NEPA Process <br> Number of Lanes To be determined in the NEPA Process - <br> between three and five total lanes. <br> Usable Shoulder Width To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. <br> Median Width To be determined in the NEPA Process - <br> dependent upon utilization of a center two- <br> way left turn lane. <br> Minimum R.O.W. To be determined in the NEPA Process - <br> dependent upon cross section selection <br> Signalization Yes |

Tennessee Department of Transportation
Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 2, L.M. 0 to 2.49 | Region: | 3 |
| County: | Williamson | Project \# |  |

## Location

| From: | Maury County Line |
| :--- | :--- |
| To: | Thompson's Station City Limits |

Parameter

| 2014 AADT | 22,400 |
| :--- | :--- |
| 2034 AADT | 36,700 |
| Percent Trucks (DHV) | $3 \%$ |
| DHV (8\% AADT 2034) | 2,940 |
| Functional Classification | Urban Other Principal Arterial |
| Minimum Design Speed | 45 mph/50 mph <br> To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. |
| Access Control | N/A |
| Minimum Radius | $730 \mathrm{ft} / 930 \mathrm{ft}$. |
| Maximum Grade | $6 \%$ |
| Minimum Stopping Sight Distance | 360 ft./425 ft. |
| Surface Width | To be determined in the NEPA Process |
| Number of Lanes | To be determined in the NEPA Process - <br> between three and five total lanes. |
| Usable Shoulder Width | To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. |
| Median Width | To be determined in the NEPA Process - <br> dependent upon utilization of a center two- <br> way left turn lane. |
| Minimum R.O.W. | To be determined in the NEPA Process - <br> dependent upon cross section selection |
| Signalization | Yes |

# Tennessee Department of Transportation 

Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 3, L.M. 2.5 to L.M. 4.75 | Region: | 3 |
| County: | Williamson | Project \# |  |

## Location

| From: | Thompson's Station City Limits |
| :--- | :--- |
| To: | South of SR 840 |

Parameter

| 2014 AADT | 34,000 |
| :--- | :--- |
| 2034 AADT | 36,700 |
| Percent Trucks (DHV) | $3 \%$ |
| DHV (8\% AADT 2034) | 2,940 |
| Functional Classification | Rural Minor Arterial |
| Minimum Design Speed | $45 \mathrm{mph} / 60$ mph <br> To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. |
| Access Control | N/A |
| Minimum Radius | $660 \mathrm{ft} / 1340 \mathrm{ft}$. |
| Maximum Grade | $5 \% / 4 \%$ |
| Minimum Stopping Sight Distance | 360 ft./570 ft. |
| Surface Width | To be determined in the NEPA Process |
| Number of Lanes | To be determined in the NEPA Process - <br> between three and five total lanes. |
| Usable Shoulder Width | To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. |
| Median Width | To be determined in the NEPA Process - <br> dependent upon utilization of a center two- <br> way left turn lane. |
| Minimum R.O.W. | To be determined in the NEPA Process - <br> dependent upon cross section selection |
| Signalization | Yes |

# Tennessee Department of Transportation 

Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 4, L.M. 4.76 to L.M. 5.55 | Region: | 3 |
| County: | Williamson | Project \# |  |

## Location

| From: | South of SR 840 |
| :--- | :--- |
| To: | Tollgate Road |


| Parameter |
| :--- |
| 2014 AADT 34,000 <br> 2034 AADT 36,700 <br> Percent Trucks (DHV) $3 \%$ <br> DHV (8\% AADT 2034) 2,940 <br> Functional Classification Rural Minor Arterial <br> Minimum Design Speed 60 mph <br>  Estimated - utilize existing <br> Access Control $\mathrm{N} / \mathrm{A}$ <br> Minimum Radius 1340 ft. <br> Maximum Grade $4 \%$ <br> Minimum Stopping Sight Distance 570 ft <br> Surface Width Varies - utilize existing <br> Number of Lanes Four travel lanes - utilize existing <br> Usable Shoulder Width Varies - utilize existing <br> Median Width Varies - utilize existing <br> Minimum R.O.W. Varies - utilize existing <br> Signalization Yes |

Note: No improvements needed in this section

# Tennessee Department of Transportation 

Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 5, L.M. 5.56 to L.M. 8.67 | Region: | 3 |
| County: | Williamson | Project \# |  |

## Location

| From: | Tollgate Road |
| :--- | :--- |
| To: | Nashville Urban Boundary |


| Parameter |
| :--- |
| 2014 AADT 24,000 <br> 2034 AADT 27,600 <br> Percent Trucks (DHV) $3 \%$ <br> DHV (8\% AADT 2034) 2,210 <br> Functional Classification Rural Minor Arterial <br> Minimum Design Speed 45 mph/60 mph <br> To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. <br> Access Control N/A <br> Minimum Radius 660 ft/1340 ft. <br> Maximum Grade $5 \% / 4 \%$ <br> Minimum Stopping Sight Distance 360 ft./570 ft. <br> Surface Width To be determined in the NEPA Process <br> Number of Lanes Four travel lanes <br> Usable Shoulder Width To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. <br> Median Width To be determined in the NEPA Process - <br> dependent upon utilization of a center two- <br> way left turn lane. <br> Minimum R.O.W. To be determined in the NEPA Process - <br> dependent upon cross section selection <br> Signalization N/A |

# Tennessee Department of Transportation 

Design Criteria for Location and Design Phase

| Route: | SR 6 | Option: | N/A |
| :--- | :--- | :--- | :--- |
| Section: | 6, L.M. 8.68 to L.M. 10.03 | Region: | 3 |
| County: | Williamson | Project \# |  |

## Location

| From: | Nashville Urban Boundary |
| :--- | :--- |
| To: | Mack Hatcher Parkway |


| Parameter |
| :--- |
| 2014 AADT 20,800 <br> 2034 AADT 32,200 <br> Percent Trucks (DHV) $3 \%$ <br> DHV (8\% AADT 2034) 2,580 <br> Functional Classification Urban Other Principal Arterial <br> Minimum Design Speed 45 mph <br> Access Control $\mathrm{N} / \mathrm{A}$ <br> Minimum Radius 730 ft. <br> Maximum Grade $6 \%$ <br> Minimum Stopping Sight Distance 360 ft. <br> Surface Width To be determined in the NEPA Process <br> Number of Lanes Four travel lanes <br> Usable Shoulder Width To be determined in the NEPA Process - <br> dependent upon rural or urban cross <br> section. <br> Median Width To be determined in the NEPA Process - <br> dependent upon utilization of a center two- <br> way left turn lane. <br> Minimum R.O.W. To be determined in the NEPA Process - <br> dependent upon cross section selection <br> Signalization Yes |














## APPENDIX

## CORRESPONDENCE

## OPTIONS REQUIRING ADDITIONAL STUDY

## COST ESTIMATES

Cost calculation summaries
Cost per mile calculations
Construction and engineering cost per mile calculations
Construction item cost per mile calculations
Structures calculations
Utility calculations
R.O.W. calculations

Paving cost calculations
TRAFFIC CALCULATIONS

## TDOT EES MAPS

## HCS+ CALCULATIONS

No Build Option
Widen Along the Existing Alignment Option
Three Lane Spring Hill Option
Three Lane Thompson's Station Option

## CORRESPONDENCE

800 Second Avenue South | Nashville, Tennessee 37201|(615) 862-7204 | wwv.nashvillempo.org Mayor Ernest Burgess, Chairman

April 4, 2008

Ms. Angie Midgett
Long Range Planning Division
Tennessee Department of Transportation
505 Deaderick Street, Suite 1000
James K. Polk State Office Building
Nashville, TN 37243-0334

Dear Ms. Midgett:
On behalf of Williamson County Government and the Executive Board of the Nashville Area MPO, I am requesting that the Tennessee Department of Transportation complete a Transportation Planning Report for State Route 6, from Old Kedron Road to Mack Hatcher Memorial Parkway. Per TDOT guidelines, our formal request is included as part of our FY 2008 Unified Planning Work Program, as amended on March 19, 2008. A copy of that request and other project-related information is enclosed with this letter.

Please contact me or Max Baker at the MPO if you need any additional information.

enclosures

CC: Mayor Ernest Burgess, MPO Executive Board Chair
Mr. Joe Horne, MPO Technical Coordinating Committee Chair Mayor Rogers Anderson, Williamson County

## APPENDIX

## TPR Requests

\#1. On behalf of Williamson County, the Nashville Area MPO requests that the Tennessee Department of Transportation complete a Transportation Planning Report (TPR) for the portion of State Route 6 from South Old Kedron Road to Mack Hatcher Memorial Parkway. Project details follow:

- LRTP\#: 6022, 6001
- TIP\#: 2008-64-068
- Horizon Year: 2016
- State Route: SR-6
- Termini: South Old Kedron Rd. to Mack Hatcher Memorial Pkwy.
- Length: 14.8 Miles
- Locally Used: US-31, Columbia Pike, Main St., Andrew Jackson
- Local Governments: Williamson County, Maury County Cities of Franklin, Thompson Station, and Spring Hill
- Description: Continued high rates of growth and the opening of SR-840 interchange have led to an AADT growth rate is at 7.7\% per annum. Thus, the LOS of SR-6 has changed from C to D .
Termini: South Old Kedron Rd. to Mack Hatcher Memorial Pkwy.

Length: $\mathbf{1 4 . 8}$ Miles

State Route: SR-6
Locally Used: US-31, Columbia Pike, Main St., Andrew Jackson

## LRTP: 6022, 6001

TIP: 2008-64-068

## Horizon Year: 2016

Local Governments: Williamson County, Maury County
Cities of Franklin, Thompson Station, and Spring Hill
Description: Continued high rates of growth and the opening
 of crashes occurred along the roadway.


Consulting Engineers

August 29, 2008
Mr. Chris Armstrong
Tennessee Department of Transportation
Project Planning Division
1000 JKP Building
505 Deaderick Street
Nashville, TN 37243-0334

## RE: SR-6 TPR Site Visit Meeting Minutes

Dear Mr. Armstrong,
A Site Visit was held Friday 8/29/08 at 10:00, meeting at Winstead Hill Park in Franklin, TN, concerning the SR-6 TPR in Maury and Williamson Counties. The study bisects Williamson County, a portion of Maury County, the City of Spring Hill, the Town of Thompson's Station, and the City of Franklin. The project limits are from Kedron Road to the South to Mack Hatcher Parkway to the north. A handout was provided to those in attendance that listed environmental constraints (including historic properties) along the route and mapped them on USGS Quad Maps. Crashes were also mapped along the route. Aerial photography of the route was included in the handout. The meeting began with a short discussion of the project limits and the status of the study. The meeting was then opened up for questions, comments, recommendations for improvements (including spot improvements), and closed by driving the project. Those in attendance are listed on below:

## Attendance List:

| Name |  | Agency | Title/Department | E-mail | Phone |
| :---: | :---: | :---: | :---: | :---: | :---: |
| William | Banks | City of Franklin | Engineering | william.banks@franklin-gov.com | 615-791-3218 |
| Eric | Gardner | City of Franklin | Engineering Dir. | ericg@franklin-gov.com | 615-791-3218 |
| Ben | Worley | City of Franklin | R.O.W. | ben.worley@franklin-gob.com | 615-791-3218 |
| Joe | York | City of Franklin | Streets Director | joey@franklin-gov.com | 615-791-3254 |
| Jon | Storey | Florence \& Hutcheson |  | istorey@flohut.com | 615-399-9090 |
| Matt | Meservy | Nashville MPO |  | meservy@nashvillempo.org | 615-862-6887 |
| Bob | Allen | TDOT | Environmental | bob.allen@state.tn.us | 615-253-2468 |
| Chris | Armstrong | TDOT | Project Planning | christopher.armstrong@state.tn.us | 615-741-3216 |
| Holly | Barnett | TDOT | Environmental | holly.barnett@state.tn.us | 615-253-2467 |
| Tyler | King | TDOT | Project Planning | tyler.king@state.tn.us | 615-253-2781 |
| Greg | Langeliers | Thompson's Station | Town Administrator | glangeliers@thompsonsstation.com | 615-794-4333 |
| Floyd | Hefflin | Williamson County | County Engineer | floydh@williamson-tn.org | 615-790-5731 |
| Joe | Horne | Williamson County | Community Dev. | joeh@williamson-tn.org | 615-790-5725 |

Consulting Engineers

## Items discussed concerning the SR-6 TPR included:

- No representative from the City of Spring Hill was present. Representatives from the City of Franklin called to see if Spring Hill would be in attendance prior to the meeting starting. Spring Hill had been contacted by e-mail and a voice message prior to the site visit.
- The MPO has a 5-lane cross section for the entire route in the LRTP, but is not opposed to other ideas.
- The Town of Thompson's Station is interested in improved access to I-65, along with improvements to I-65, if this option could reduce the cross section through their town. One idea discussed was a new interchange at Buckner Lane and I-65.
- It was asked if the improvements discussed above could be included in the MPO model for use in the TPR. "What If" scenarios could be included in the MPO model, but would be of limited benefit due to the regional nature of the model. Therefore, the improvements currently in the LRTP that provide improvements along I-65, along with a new interchange between Thompson's Station Road and I-65, are included in the traffic projections, but no further "what-if" scenarios will be performed at this time.
- It was agreed that the Town of Thompson's Station recommendation should be discussed in the TPR without a traffic analysis.
- The extent of what a "spot improvement" could be was discussed.
- All local officials present appeared in agreement that improvements to the Goose Creek bypass intersection with SR-6 should be included as part of a spot improvement.
- A roundabout was discussed at the Goose Creek Bypass intersection with SR-6. This intersection is within Franklin's growth boundary. Franklin was in favor of studying this improvement. They are comfortable with multilane roundabouts along reasonably high speed facilities. They cited a roundabout along Mack Hatcher as an example. Eric Gardner will provide F\&H with copies of the plans as a reference.
- Williamson County requested improvements to SR-6 be included in the LRTP to get options "on the table" and address the area's sub regional travel demands. The goal is to get input from Franklin, Thompson's Station, and Spring Hill to find a common solution to the region's traffic issue concerning SR-6.
- Historic properties will be an issue along this route. Williamson County noted that in the past historic groups have requested sizeable setbacks along road improvements to historic properties.
- It was discussed that input from interest groups (in this case historic societies) is generally not solicited during this early TPR stage of a roadway project. Input will be sought as the NEPA process continues and the project enters more formal stages.
- Thompson's Station has plans to realign Critz Lane at SR-6 and would welcome help as a spot improvement. R.O.W. has already been dedicated for this project. It was also noted that the intersection between SR-6 and Thompson's Station Road needs improvement.
- Thompson's Station is in agreement that the segment of SR-6 within their town growth boundary should be 5 lanes between Critz Lane and the Goose Creek Bypass.
- Improvements around Spring Hill are the most challenging segment of this project due to tight R.O.W. and extensive environmental locations.
- It was noted that the City of Spring Hill is currently attempting to construct the Miles Johnson Parkway as a bypass for the City. They may be interested in handing this project over to TDOT to be utilized as a bypass for SR-6. The Miles Johnson Parkway site was driven to investigate its application. The location is adequate from Kedron Road north, but

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Consulting Engineers
reconnecting it south to SR-6 will be a challenge due to the Battle of Spring Hill location and the extended project limits south of Kedron Road this will require. It would negate a large segment of 5-lane highway along SR-6. It was agreed that this option should be studied further in the TPR.

- It was discussed to include an option that would proceed to the west along the railroad tracks as a bypass for the City of Spring Hill.
- An option to utilize School Road in Spring Hill as a part of a one-way pair with SR-6 was discussed and will be included as an option. This option was more appealing on aerial mapping than in the field. A children's home/school would be impacted, and School Road is narrow and located in a primarily residential/well developed area. Significant impacts would be expected.
- The City of Franklin would like the extension of the right turn lane along SR-6 approaching Mack Hatcher Parkway included as a spot improvement in the study.
- The City of Franklin would like the realignment of the intersection between Coleman and Henpeck Lanes included as a spot improvement. These intersections are currently offset.
- It was discussed that the intersection of Duplex Road and Beechcroft Road should also be included as a spot improvement. These intersections are currently offset. A farmer's COOP would likely be impacted by this improvement.


## Action Items

- Coordination between F\&H and the City of Spring Hill to gain their input.
- Coordination between F\&H and Eric Gardner for multilane roundabout plans.
- Coordination between F\&H and the MPO to determine if the southern limit of study can be extended to allow connection of a bypass option around Spring Hill to SR-6.

As always, if you have any questions or comments, please do not hesitate to call or e-mail.
Thank you,
Florence \& Hutcheson, Inc.

Jon Storey, P.E.

September 10, 2008
Mr. Chris Armstrong
Tennessee Department of Transportation
Project Planning Division
1000 JKP Building
505 Deaderick Street
Nashville, TN 37243-0334

## RE: SR-6 TPR Spring Hill Meeting Minutes

Dear Mr. Armstrong,
A meeting was held with the City of Spring Hill Wednesday 9/10/08 at 11:00, meeting at Spring Hill's City Hall, concerning the SR-6 TPR in Maury and Williamson Counties. This meeting was held to gain input from the City concerning the study. The study bisects Williamson County, a portion of Maury County, the City of Spring Hill, the Town of Thompson's Station, and the City of Franklin. The project limits are from Kedron Road to the South to Mack Hatcher Parkway to the north. A handout was provided to those in attendance that listed environmental constraints (including historic properties) along the route and mapped them on USGS Quad Maps.
Crashes were also mapped along the route. Aerial photography of the route was included in the handout. The meeting began with a short discussion of what occurred at the Site Visit on $8 / 29 / 08$. The meeting was then opened up for questions, comments, recommendations for improvements (including spot improvements). Those in attendance are listed on below:

## Attendance List:

| Name |  | Agency | Title/Department | E-mail | Phone |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Chris | Armstrong | TDOT | Project Planning | christopher.armstrong@state.tn.us | $615-741-3216$ |
| Danny | Leverette | Spring Hill | Mayor |  | $931-384-0500$ |
| John | McCord | Spring Hill | Director of Public <br> Works |  | $931-486-1265$ |
| Jamie | Pierce | Spring Hill | Deputy Director of <br> Public Works |  | $931-384-0640$ |
| Jon | Storey | Florence \& Hutcheson |  | istorey@flohut.com | $615-399-9090$ |
| Ken | Weaver | Spring Hill | City Administrator |  | $931-797-4447$ |

Florence \& Hutcheson, Inc.
Consulting Engineers

## Items discussed concerning the SR-6 TPR included:

- The city is concerned with widening SR-6/US-31 through the city and does not support this option. They wish to revitalize downtown and maintain a walkable community. It appears a streetscape is desired in the future. A 5-lane section is not desirable. The existing 3-lane section is what they envision through the city.
- The City would like the LRTP amended to show a 3-lane segment through the city.
- They are currently studying a signal coordination system through the city and were curious how this study could affect this. It was not believed the TPR would affect their current signal coordination efforts.
- The historic locations along the route and crashes were discussed.
- Improvements to the existing route, an east, and a west bypass option were discussed.
- If a bypass is chosen, ownership of existing SR-6 was discussed. It was noted it would likely become their responsibility. This did not appear to be a major hurdle as it may allow them more flexibility with improving the aesthetics of the route and more freedom in the design. It was noted that prior to turning over the route TDOT would perform needed maintenance, likely including an overlay. The financial aspect of SR-6 becoming local responsibility in the future was discussed as a minus, but was not a deal breaker.
- The city has some issues with the Saturn Parkway Interchange with SR-6. It appears the flyover ramp from Saturn Pkwy to SR-6 Southbound has a history of crashes and should possibly be improved to 2 lanes.
- If no bypass is built, the city would like to see 5 lanes extended along the south terminus of the project to Spring Hill Elementary, then tapered to 3 lanes. The three lanes would continue north until just south of a bridge that is located south of Miles Johnson Parkway. The 5 lanes would continue throughout the city's growth boundary.
- The City is receptive to studying a bypass along Miles Johnson Parkway (an east bypass). The bypass would follow existing Saturn Parkway, exit at what is currently Kedron Parkway and continue north along Miles Johnson Parkway until tying back into existing SR-6. With this option, the city would like to see improvements to the Saturn Parkway Interchange with SR-6.
- A 53 bed hospital will be located along the Proposed Miles Johnson Parkway. The City is concerned by the time the SR-6 Bypass would be constructed, the development in the area will no longer be receptive to a wider Bypass footprint, but still would like this option studied.
- There are wetlands around the proposed Miles Johnson Parkway route.
- The City did not appear interested in continuing the Bypass south of Saturn Parkway. It was noted SR-6 already has 5 lanes south of Saturn Parkway and the limits would extend well south of Spring Hill.
- The City was also interested in studying a west bypass as an option. They currently have a line in their Major Thoroughfare Plan for a similar facility. The route would extend from Town Center Parkway (Yockovich Pkwy.) north to Thompson's Ridge Road. This option would extend west of the railroad tracks to avoid existing development. It was discussed

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Consulting Engineers
that two issues with this route may be the at-grade intersection and turning movements at Town Center Parkway and the long length of existing SR-6 that would become the responsibility of the City. It was also discussed that Thompson's Station may not like the route tying in close to schools within their growth limits. Another issue not discussed could be the railroad crossings. Grade separation would likely be required.

- Spot improvements were discussed. Spot improvements desired by the City included:
o Turning lanes are desired at the Buckner Road and Campbell Station Road Intersections.
o Realignment is desired at the Duplex Road/Beechcroft Road Intersection to align these two routes.
o Signalization is desired at Miles Johnson Parkway and Bellshire Way.
- F\&H was provided a copy of the City's Major Thoroughfare Plan.
- A one-way pair option with existing SR-6 and School Road was briefly discussed. The City does not support this option.
- The City is interested in an interchange between Buckner Road and I-65. The LRTP currently has an interchange at Thompson's Station Road. The City would like this interchange moved to Buckner Road due to a large tract of land that is proposed to be developed into a large retail area.

As always, if you have any questions or comments, please do not hesitate to call or e-mail.
Thank you,
Florence \& Hutcheson, Inc.

Jon Storey, P.E.

Florence \& Hutcheson, Inc.
Consulting Engineers

January 28, 2009
Mr. Chris Armstrong
Tennessee Department of Transportation
Project Planning Division
1000 JKP Building
505 Deaderick Street
Nashville, TN 37243-0334

## RE: SR-6 TPR Meeting

Mr. Armstrong,
A meeting was held on Tuesday 1/27/09, at 2:00 PM in TDOT's $10^{\text {th }}$ floor conference room. Those in attendance included:

| Name | Organization/Title | Phone |
| :--- | :--- | :--- |
| Steve Allen | TDOT Project Planning <br> Division | $741-2208$ |
| Chris Armstrong | TDOT Project Planning <br> Division | $741-3216$ |
| Bill Hart | TDOT Project Planning <br> Division | $741-3688$ |
| Jeanne Stevens | TDOT Long Range <br> Planning Division | $741-3421$ |
| Matt Meservy (via <br> teleconference) | Nashville Metropolitan <br> Planning Organization | $862-6887$ |
| Jon Storey | Florence \& Hutcheson, Inc. | $399-9090$ |

The primary focus of the meeting was to determine if the options included in the SR-6 TPR should be strictly limited by what is listed in the MPO's LRTP, or if additional options requested by local agencies should also be included. The Draft SR-6 TPR currently includes bypass options in addition to an option to widen along the existing alignment. The LRTP only lists widening along the existing alignment. The discussion proceeded to include the interrelation between the MPO planning process and the TPR process for all projects, not just the SR-6 project. The items listed below were discussed.

- The northern terminus was discussed. The Draft SR-6 TPR utilizes Mack Hatcher Parkway as the northern terminus, but notes that the LRTP projects along SR-6 end at Henpeck Lane. After the meeting, it was confirmed that the LRTP has a discrepancy concerning the 1.25 -mile segment between Henpeck Lane and Mack Hatcher Parkway. The LRTP map notes Project Number 6032 along this segment of SR-6. However, the LRTP project list notes that Project Number 6032 is for SR-106 (Lewisburg Pike) from Henpeck Lane to Mack Hatcher Parkway, and includes widening from two lanes to four lanes with bike lanes. Confirmation from the MPO is needed if Project Number 6032 is actually for SR-6 and not SR-106. How to document this in the TPR will be discussed with TDOT.
- The LRTP notes the southern terminus of the project at Old Kedron Road. However, Old Kedron Road does not intersect SR-6. Kedron Road intersects SR-6. The LRTP may need to be revised for this. Also, the bypass options intersect SR-6 south of Kedron Road. The LRTP may need to be revised to account for the extended project limits.
- The LRTP notes the improvements will include 4 lanes along SR-6 in Williamson County and 5 lanes along SR-6 within Spring Hill's City Limits. Stakeholders outside of Spring Hill appear to be in favor of a 5-lane cross section. Stakeholder's in Spring Hill do not want the existing 3-lane cross section widened within their CBD and desire a bypass. Thompson's Station noted they would favor a 3-lane cross section through their city limits instead of 4 lanes. The Draft TPR provides the cross section options noted by the local stakeholders.
- It was understood by F\&H that it is acceptable to provide bypass and cross section options in the SR-6 TPR, as currently shown. F\&H submitted the Draft TPR on 12/12/08. If the bypass options are acceptable, F\&H will await TDOT's review of the draft. F\&H will coordinate with the Project Planning Division to determine what revisions are desired. Revisions concerning issues discussed in these meeting minutes are anticipated.

As always, if you have any questions or comments, please do not hesitate to call or e-mail.
Thank you,
Florence \& Hutcheson, Inc.

Jon Storey, P.E.

October 21, 2009

Mr. Chris Armstrong<br>Tennessee Department of Transportation<br>Project Planning Division<br>1000 JKP Building<br>505 Deaderick Street<br>Nashville, TN 37243

## RE: SR-6/US-31 TPR Meeting Minutes

Dear Mr. Armstrong,
A meeting was held today in TDOT's $10^{\text {th }}$ Floor Conference Room. Those in attendance included:

| Chris Armstrong | TDOT |
| :--- | :--- |
| Steve Allen | TDOT |
| Gary Webber | TDOT |
| Eric Gardner | City of Franklin |
| Ben Worley | City of Franklin |
| Gillian Fischbach | Fischbach Transportation Company |
| Jon Storey | Florence \& Hutcheson |

Items discussed included:

An MPO retreat was held a week ago concerning the role of a TPR in relation to the LRTP. It was agreed at the MPO retreat that the improvement recommendations in the LRTP are an initial guide, and not set in stone. The TPR can therefore still look at other reasonable options. It must be noted in the executive summary of the TPR where these options deviate from the LRTP. This is especially critical when the termini in the TPR deviate from the termini in the LRTP.

Concerning the SR-6 TPR, TDOT is open to looking at bypass options, but wants to ensure they are options that meet TDOT's needs. It is likely if bypass options are to be included, they will not be the same options currently in the TPR. If no feasible bypass option is available, we may simply discuss widening along the existing centerline and note all the constraints. Chris Armstrong and Jon Storey will coordinate with Bill Hart concerning bypass options and how to proceed with the SR-6 TPR.

Florence $\mathcal{\&}$ Hutcheson, Inc.
Consulting Engineers

If there are any questions, comments, or omissions concerning these minutes, please do not hesitate to call.

Thank you,
Florence \& Hutcheson, Inc.

Jon Storey, P.E.

November 3, 2009
Mr. Chris Armstrong
Tennessee Department of Transportation
Project Planning Division
1000 JKP Building
505 Deaderick Street
Nashville, TN 37243

## RE: SR-6/US-31 TPR Meeting Minutes

Dear Mr. Armstrong,
A meeting was held yesterday in TDOT's $10^{\text {th }}$ Floor Conference Room on 11/2/09. Those in attendance included:

Chris Armstrong TDOT
Steve Allen TDOT
Bill Hart TDOT
Jon Storey Florence \& Hutcheson
Items discussed included:
The SR-6 TPR should be revised to include the following options:

1. Widening along the existing corridor as described in the LRTP
2. Reasonable options along the existing corridor taking into account environmental constraints and stakeholder input.
3. The bypass options around Spring Hill will be placed in a section titled "options previously considered but rejected". Why the bypass options are rejected will be discussed, including access control issues, not meeting the State's Purpose and Need or standards for a bypass, the regional significance of a bypass, intersection issues at existing SR-6, and the reallocation of truck traffic through residential areas surrounding the bypass routes.

How the different options affect the project's termini and will be discussed in the TPR and executive summary. Additionally, all options not following the LRTP will be summarized in the executive summary. Also discussed at the meeting was how a city's Major Thoroughfare Plan (MTP) doesn't control the LRTP or options in a TPR. The MTP is to be considered when mutual benefits are achievable, but is not a controlling factor.

Florence $\mathcal{\&}$ Hutcheson, Inc.
Consulting Engineers

If there are any questions, comments, or omissions concerning these minutes, please do not hesitate to call.

Thank you,
Florence \& Hutcheson, Inc.


Jon Storey, P.E.

## Options Requiring Additional Study

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### 1.0 OPTIONS REQUIRING ADDITIONAL STUDY SUMMARY

This Transportation Planning Report (TPR) examines the improvements listed in the Nashville Area Metropolitan Planning Organization's (MPO) Long Range Transportation Plan (LRTP) for 10.87 miles of State Route 6 (SR 6) from the City of Spring Hill to the southern limits of the City of Franklin in south-central Tennessee. The improvements widen the existing route from two (2) travel lanes to four (4) travel lanes.

The Widen Along the Existing Alignment Option does not have the support of the City of Spring Hill due to the impact to their central business district (CBD). The existing R.O.W. within the CBD of Spring Hill is approximately sixty (60) feet wide. Improvements to SR 6 within the CBD will require impacts to numerous religious, historic, and commercial properties. Furthermore, the City wishes to revitalize their downtown and maintain their walkable community. They envision a future streetscape project that will maintain the existing three (3) lane cross section.

Additionally, the Widen Along the Existing Alignment Option does not have the support of the Town of Thompson's Station. A representative of the Town notes that future improvements along Interstate 65 may alleviate congestion along SR 6 within their town limits, and therefore eliminate the need to widen SR 6 to four (4) travel lanes within the town limits. Concern was expressed that widening SR 6 without improvements to Interstate 65 would increase traffic volumes through the town.

As a result of the lack of support from local officials for the improvements listed in the LRTP, spot improvements, travel lane options, and bypass options were studied to limit impacts to sensitive areas along the route. The MPO noted that new options are welcome. It was noted that changes in the project, especially changes to the project termini, will need to be coordinated with the MPO and incorporated into the LRTP. The additional options studied are summarized as follows:

- Spot Improvements
- Three (3) Lane Spring Hill Option
- Three (3) Lane Thompson's Station Option
- Three (3) Lane Spring Hill and Thompson's Station Option
- Spring Hill One-Way Pair Option
- Eastern Spring Hill Bypass Option
- Western Spring Hill Bypass Option

Each of these options require additional study due to one or more of the following reasons: not meeting the purpose and need of the project, not having the support of local officials; not currently being included in the MPO's planning process; not providing acceptable access control measures; not meeting the regional significance or providing for a continuous route for a bypass; the existence of an existing parallel route (Interstate 65) which, if improved, negates the need for a bypass; impacts to environmentally sensitive locations; and due to the reallocation of truck traffic to residential areas along the option's routes. Therefore, these options were not included in the TPR's main body. It should be noted that further refinement of any of these options through the MPO's planning process may create a viable option to meet the Purpose and Need of the project.

### 2.0 SPOT IMPROVEMENTS

Due to limited funding and in consideration that corridor wide improvements would have considerable costs, locations for spot improvements were solicited from local officials. These improvements should be considered interim improvements to address the most urgent needs along the route, but they do not meet the Purpose and Need of the project due to their limited ability to improve capacity through the corridor. These improvements can be incorporated into future corridor wide improvement options, including the Widen Along the Existing Alignment Option. The Spot Improvements identified are listed below, from south to north along SR 6.

- Realign the offset intersection of Duplex Road (SR 247) and Beechcroft Road (SR 247) at SR 6 to create a standard four (4) legged intersection. Estimated cost is \$740,000.
- Signalization of the intersection of SR 6 with Miles Johnson Parkway. Estimated cost in year 2014 dollars is $\$ 150,000$.
- Signalization of the intersection of SR 6 with Bellshire Way. Estimated cost in year 2014 dollars is $\$ 150,000$.
- Turn lane improvements at the intersection of SR 6 with Campbell Station Parkway. Estimated cost in year 2014 dollars is $\$ 300,000$.
- Turn lane improvements at the intersection of SR 6 with Buckner Road. Estimated cost in year 2014 dollars is $\$ 300,000$.
- Turn lane improvements at the intersection of SR 6 with Thompson's Station Road. Estimated cost in year 2014 dollars is $\$ 300,000$.
- Realign Critz Lane at the intersection with SR 6. Estimated cost in year 2014 dollars is \$950,000.
- Improve the bridge over a branch of the Harpeth River located just south of the Goose Creek Bypass Intersection with SR 6. Estimated cost in year 2014 dollars is $\$ 1,512,000$.
- Construct a roundabout at the Goose Creek Bypass intersection with SR 6. Estimated cost in year 2014 dollars is $\$ 5,680,000$.
- Realign the offset intersection of Coleman Road and Henpeck Lane at SR 6 to create a standard four (4) legged intersection. Estimated cost in year 2014 dollars is \$1,950,000.
- Extend the right turn lane of SR 6 Northbound at the intersection with Mack Hatcher Parkway. Estimated cost in year 2014 dollars is $\$ 200,000$.

No HCS LOS analysis or order of priority analysis was developed for the Spot Improvement Options. These options were developed as a result of feedback received during stakeholder meetings that occurred during the development of this TPR. Analysis of the Spot Improvement Options requires detailed traffic and geometric information that is currently not available. The cost of all eleven (11) Spot Improvement Options is estimated to be $\$ 12.2$ million in year 2014 dollars. The Spot Improvement Options are mapped in Figure 2 Spot Improvement Options.


Figure 2 Spot Improvement Options

### 2.1 SR 6 AT SR 247

This spot improvement will realign the offset intersection of Duplex Road (SR 247) and Beechcroft Road (SR 247) at SR 6 to create a standard four (4) legged intersection. These intersections are signalized and operate with one (1) traffic controller. This option will improve traffic operations along SR 6 by eliminating one (1) signal along the route and simplifying the signal timing within the City of Spring Hill's congested CBD. This improvement will likely require one (1) commercial property to be relocated. This spot improvement is programmed in the MPO's Transportation Improvement Program (TIP) for the year 2011. The estimated cost of the improvements is provided in the TIP to be $\$ 740,000$.

### 2.2 SR 6 AT MILES JOHNSON PARKWAY

This spot improvement will provide signalization at the intersection of SR 6 with Miles Johnson Parkway. Although an additional signal along SR 6 will create additional mainline delays, it will improve the operation of the intersection, and possibly improve the safety of the intersection. A signal may also promote use of Miles Johnson Parkway as an alternate route for SR 6 by enabling improved access to the route. A signal warrant analysis will need to be performed prior to signalization of the intersection. The cost of signalizing this intersection is estimated to a planning level to be $\$ 150,000$.

### 2.3 SR 6 AT BELLSHIRE WAY

This spot improvement will provide signalization at the intersection of SR 6 with Bellshire Way. Although an additional signal along SR 6 will create additional mainline delays, it will improve the operation of the intersection, and possibly improve the safety of the intersection. A new Lowe's Home Improvement Center, medical office building, and several outparcel commercial developments have recently been constructed at this intersection. A signal warrant analysis will need to be performed prior to signalization of the intersection. The cost of signalizing this intersection is estimated to a planning level to be $\$ 150,000$.

### 2.4 SR 6 AT CAMPBELL STATION PARKWAY

This spot improvement will provide turn lane improvements at the intersection of SR 6 with Campbell Station Parkway. This intersection is signalized and each approach includes left turn bays. The surrounding land use is commercial, with a Publix Grocery Store anchoring the development. Local officials noted the need for turn lane improvements. A detailed traffic analysis is needed to determine the need for right turn bays and/or to determine the length to increase the left turn bays. Turn lane improvements could improve operations and safety along SR 6 by reducing or eliminating queued vehicles from the mainline, therefore improving traffic flow. With the limited available information, the cost of providing turn lane improvements is estimated at $\$ 300,000$.

### 2.5 SR 6 AT BUCKNER ROAD

This spot improvement will provide turn lane improvements at the intersection of SR 6 with Buckner Road. This intersection is signalized and each approach includes left turn bays. The surrounding land use is commercial, educational, and residential. Local officials noted the need
for turn lane improvements. A detailed traffic analysis is needed to determine the need for right turn bays and/or to determine the length to increase the left turn bays. Turn lane improvements could improve operations and safety along SR 6 by reducing or eliminating queued vehicles from the mainline, therefore improving traffic flow. With the limited available information, the cost of providing turn lane improvements is estimated to be $\$ 300,000$.

### 2.6 SR 6 AT THOMPSON'S STATION ROAD

This spot improvement will provide turn lane improvements at the intersection of SR 6 with Thompson's Station Road. This intersection is signalized and each approach includes left turn bays. The surrounding land use is commercial, religious, and residential/vacant. Local officials noted the need for turn lane improvements. A detailed traffic analysis is needed to determine the need for right turn bays and/or to determine the length to increase the left turn bays. Turn lane improvements could improve operations and safety along SR 6 by reducing or eliminating queued vehicles from the mainline, therefore improving traffic flow. With the limited available information, the cost of providing turn lane improvements is estimated to be $\$ 300,000$.

### 2.7 SR 6 AT CRITZ LANE

This spot improvement will realign Critz Lane at the intersection with SR 6. Critz Lane currently makes a 90 -degree bend 0.2 miles before intersecting SR 6 . The intersection of Critz Lane with SR 6 is skewed. Realigning Critz Lane will improve safety by eliminating the 90 -degree bend in Critz Lane and improving the skew of the intersection with SR 6 . Realigning Critz Lane will also extend the distance from this intersection to the SR 840 Interchange, potentially improving sight distance, safety and traffic operations. R.O.W. has been dedicated for this improvement, but design and construction funds are currently not available. The cost of the improvements is estimated to a planning level to be $\$ 950,000$.

### 2.8 HARPETH RIVER BRIDGE

This spot improvement will improve the SR 6 Bridge over The West Harpeth River located just south of the Goose Creek Bypass Intersection with SR 6. The bridge has no shoulders and substandard bridge rails. Sight distance is restricted due to these factors. Replacing the bridge will improve safety by providing shoulders and improving sight distance. The cost to replace the bridge is estimated to a planning level to be $\$ 1,512,000$. This estimate assumes the bridge would be constructed to accommodate a future five (5) lane cross section with shoulders.

### 2.9 SR 6 AT THE GOOSE CREEK BYPASS

This spot improvement recommends constructing a roundabout at the Goose Creek Bypass intersection with SR 6. Numerous local officials noted the need for improvements at this intersection due to operational and safety deficiencies. The proposed improvements are within the City of Franklin's Urban Growth Boundary. Officials from the City of Franklin noted their support for a roundabout at this location. The roundabout should be constructed to accommodate two (2) travel lanes within the roundabout. This will enable the roundabout to be modified when SR 6 and/or the Goose Creek Bypass are widened. A roundabout could serve as a "gateway" into Franklin from the south and Thompson's Station from the north. Roundabouts eliminate all left turn movements within the intersection. All approaches will operate under yield condition. Because of this, safety benefits can be anticipated compared to the existing configuration. It is estimated that no additional R.O.W. will be needed to construct a
roundabout. The cost to construct a roundabout is estimated to a planning level to be \$5,680,000.


Figure 2.9 Goose Creek Bypass Concept

### 2.10 SR 6 AT COLEMAN ROAD/HENPECK LANE

This spot improvement will realign the offset intersection of Coleman Road and Henpeck Lane at SR 6 to create a standard four (4) legged intersection. These intersections are unsignalized. This option will improve traffic operations along SR 6 by eliminating one (1) intersection along the route. A church, cemetery, and CSX Railroad are in the study area of the improvements. The cost of the improvements is estimated to a planning level to be $\$ 1,950,000$. This cost estimate assumes the church and CSX Railroad can be accommodated, and the cemetery can be avoided. The cost will increase significantly if any of these properties cannot be accommodated.

### 2.11 SR 6 AT MACK HATCHER PARKWAY

This option will extend the length of the right turn lane of SR 6 Northbound at the intersection with Mack Hatcher Parkway. This intersection is signalized. The adjacent land use is vacant. Local officials noted the need for the turn lane improvement. A detailed traffic analysis is needed to determine the length to increase the right turn bay. Extending the right turn lane will improve operations and safety along SR 6 by allowing right turning vehicles to bypass queued vehicles along SR 6 northbound. With the limited available information, the cost of extending the turn lane is estimated to be $\$ 200,000$.

### 3.0 THREE (3) LANE SPRING HILL OPTION

This option presents a three (3) lane cross section through the City of Spring Hill's CBD. Numerous historic structures, places of worship, and businesses are located adjacent to the narrow right-of-way in the CBD. SR 6 is currently three (3) lanes in the CBD. Widening the route to add additional lanes does not have the support of the City due to the negative impact to adjacent developments and pedestrians.

Miles Johnson Parkway was recently constructed to the east of SR 6. Miles Johnson Parkway provides an alternate route for SR 6 between Kedron Road, Duplex Road, and SR 6. Additionally, a new interchange at Interstate 65 and Thompson's Station Road is currently in the MPO's LRTP, along with widening Interstate 65 from SR 840 to SR 96. These improvements may alleviate congestion along SR 6, and therefore eliminate the need to widen SR 6 to four (4) travel lanes through Spring Hill's CBD. This option could be implemented as an interim option to investigate the congestion improvements created by the planned and current roadway improvements in the area.

With this option, SR 6 would be improved to four (4) travel lanes outside the CBD. Within the CBD (between Kedron Road and Miles Johnson Parkway), it is desirable to provide pavement and sidewalk rehabilitation along the existing and proposed three (3) lane section. Approximately $1 / 3$ of a mile of roadway between Witt Hill Drive and Miles Johnson Parkway will be improved from a two (2) lane cross section to a three (3) lane cross section. It is also desired by the city to construct 700 feet of roadway improvements from the study corridor's southern terminus at Kedron Road to Spring Hill Elementary School. Providing additional capacity between Spring Hill Elementary School and the existing four (4) travel lane cross section south of the study corridor will mitigate congestion associated with this large traffic generator. It is also desired by the city for this project to rehabilitate the McCutcheon Creek Culvert. This culvert is located approximately 600 feet south of Miles Johnson Parkway. The four (4) travel lane cross section is anticipated to begin at Miles Johnson Parkway due to this being a logical terminus and proceed north. A detail of the area is provided in Figure 3.1.

Because the MPO's LRTP currently includes improvements to I-65 and a new interchange along Interstate 65 at Thompson's Station Road, the effects of these improvements are included in the MPO's travel demand model. As directed by TDOT, and as standard practice within MPO areas, the travel demand model was utilized in the traffic projections included in this report. Therefore the LOS calculations performed in this report include the effects of these improvements. However, The traffic projections utilized in this report were performed prior to the opening of Miles Johnson Parkway and the idling of the GM plant, neither of which would have been predicted or included in the MPO's travel demand model.

The Three (3) Lane Spring Hill Option is estimated to cost between $\$ 74$ and $\$ 84$ million in year 2014 dollars. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate summary is provided in Table 3.1. The cost estimate calculations are provided in the Appendix.

For the Three (3) Lane Spring Hill Option, the HCS's analysis calculates LOS ranging from A to D along SR 6 through the year 2014 and A to F through the year 2034. The LOS calculations do not take into effect the diminished traffic operations caused by the four (4) school zones along the route. A summary of the LOS calculations for the Three (3) Lane Spring Hill Option is provided in schematic form in Figures 3.2 and 3.3 and in table form in Table 3.2. The LOS are reported for the years 2014 and 2034. The LOS calculations demonstrate that four (4) travel lanes are necessary along the route. Two travel lanes within Spring Hill's CBD are not adequate to meet the projected design year traffic demand.

For the Three (3) Lane Spring Hill Option in the year 2014, the volume to capacity ratio (v/c) of SR 6 is calculated to range from 0.23 to 1.02 , with a weighted average of 0.44 . In 2034, the v/c ranges from 0.28 to 1.50 with a weighted average of 0.58 . The average was weighted based upon the length of each segment analyzed. A v/c ratio near or above " 1 " indicates a roadway experiences congestion. The v/c calculations demonstrate that four (4) travel lanes are necessary along the route to meet the projected traffic demand. A summary of the v/c calculations for the Three (3) Lane Spring Hill Option is provided in table form in Table 3.2. The $\mathrm{v} / \mathrm{c}$ are reported for the years 2014 and 2034.

The posted speed limit ranges from 35 to 55 mph along SR 6 within the study area. For the Three (3) Lane Spring Hill Option in the year 2014, travel speeds along the route are calculated by the HCS to range from 14.6 mph to 54.5 mph , with a weighted average of 37.3 mph . In 2034, the travel speed ranges from 3.5 mph to 54.4 mph with a weighted average of 22.4 mph . The average was weighted based upon the length of each segment analyzed. The weighted average of the speed limit along the route is 45.5 mph . The calculated average route speed is $82 \%$ and $49 \%$ of the posted speed limit in the years 2014 and 2034, respectively. The Nashville Area MPO's Congestion Management Process notes that if the average route speed is 70\% or less of the free flow speed, the roadway is congested. Therefore, for the Three (3) Lane Spring Hill Option, SR 6 is congested in 2034. A summary of the travel speed calculations for the Three (3) Lane Thompson's Station Option is provided in table form in Table 3.2. The travel speeds are reported for the years 2014 and 2034.

The existing SR 6 Corridor (Three (3) Lane Spring Hill Option) between Kedron Road to the south and Mack Hatcher Parkway to the north is 10.9 miles in length. For the Three (3) Lane Spring Hill Option in the year 2014, the travel time along SR 6 is calculated to be 17.5 minutes. In 2034, the travel time is calculated to be 29.1 minutes.

The traffic analysis demonstrates that four (4) travel lanes are necessary along the route. Two travel lanes within Spring Hill's CBD are not adequate to meet the projected design year traffic demand. Therefore, this option does not adequately address the purpose and need of the project.


Figure 3.1 Three (3) Lane Spring Hill Option Detail
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Table 3.1 Three (3) Lane Spring Hill Option Cost Estimate

| ID | Location/ Description | $\begin{array}{\|c\|} \text { Existing } \\ \text { Roadway } \\ \text { Classification } \end{array}$ | Proposed Roadway Classification | $\begin{array}{\|c\|} \hline \text { Length } \\ \text { of } \\ \text { Const. } \\ \text { (Miles) } \\ \hline \end{array}$ | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Maury/ Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.84 | \$ 722,400 | \$ 722,400 |
| 2A | SR-6 from Maury/ Williamson County Line to Miles Johnson Pkwy. | 2-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.20 | \$ | \$ |
| 2 | SR-6 from Miles Johnson Pkwy. to Thompson's Station City Limits (L.M. 0.20 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.29 | \$ 16,281,900 | \$ 19,991,700 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 73,980,000 | \$ 84,030,000 |

Notes: Existing and proposed 3 -lane sections are urban x-section. Multilane sections as described below. Low estimate utilizes 4 -lane rural x -section, high estimate utilizes 5 -lane urban x -section. Total route length is equal to the construction length of 10.9 miles.


Figure 3.2 Three (3) Lane Spring Hill Option 2014 LOS


Figure 3.3 Three (3) Lane Spring Hill Option 2034 LOS

## Table 3.2 Three (3) Lane Spring Hill Option los Table



### 4.0 THREE (3) LANE THOMPSON'S STATION OPTION

This option presents a three (3) lane cross section through the Town of Thompson's Station. A new interchange at Interstate 65 and Thompson's Station Road is currently in the MPO's LRTP, along with widening Interstate 65 from SR 840 to SR 96. The Town notes that these improvements may alleviate congestion along SR 6 , and therefore eliminate the need to widen SR 6 to four (4) travel lanes through Thompson's Station. It is possible the proposed interchange will be relocated to a proposed Buckner Road extension at Interstate 65. It is desired by the town to improve SR 6 to three (3) lanes between either Buckner Road or Thompson's Station Road and Critz Lane. The remainder of SR 6 will be improved to four (4) travel lanes. This option could be implemented as an interim option to investigate the congestion improvements created by the planned improvements along I-65. However, the traffic calculations discussed below demonstrate that a three (3) lane cross section through the Town of Thompson's Station will not meet the projected traffic demand.

Because the MPO's LRTP currently includes improvements to Interstate 65 and a new interchange along Interstate 65 at Thompson's Station Road, the effects of these improvements are included in the MPO's travel demand model. As directed by TDOT, and as standard practice within MPO areas, the travel demand model was utilized in the traffic projections included in this report. Therefore the LOS calculations performed in this report include the effects of these improvements.

The Three (3) Lane Thompson's Station Option is estimated to cost between \$88 and \$100 million in year 2014 dollars. The cost estimate includes widening along SR 6. The estimate does not include a new interchange or widening along Interstate 65. The costs of these improvements are not included in the estimate since these improvements are currently programmed in separate projects in the LRTP. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate summary is provided in Table 4.1. The cost estimate calculations are provided in the Appendix.

For the Three (3) Lane Thompson's Station Option, the HCS's analysis calculates LOS ranging from A to F along SR 6 through the year 2034. The LOS calculations do not take into effect the diminished traffic operations caused by the four (4) school zones along the route. A summary of the LOS calculations for the Three (3) Lane Thompson's Station Option is provided in schematic form in Figures 4.1 and 4.2 and in table form in Table 4.2. The LOS are reported for the years 2014 and 2034. The LOS calculations demonstrate that four (4) travel lanes are necessary along the route, and that two (2) travel lanes within Thompson's Station's Town Limits are not adequate to meet the traffic demand.

For the Three (3) Lane Thompson's Station Option in the year 2014, the volume to capacity ratio ( $\mathrm{v} / \mathrm{c}$ ) of SR 6 is calculated to range from 0.23 to 0.92 , with a weighted average of 0.49 . In 2034, the v/c ranges from 0.28 to 0.99 with a weighted average of 0.61 . The average was weighted based upon the length of each segment analyzed. A v/c ratio near or above "1" indicates a roadway experiences congestion. The v/c calculations demonstrate that four (4) travel lanes are necessary along the route, and that two (2) travel lanes within Thompson's Station's Town Limits are not adequate to meet the traffic demand. A summary of the v/c calculations for the Three (3) Lane Thompson's Station Option is provided in table form in Table 4.2. The v/c are reported for the years 2014 and 2034.

The posted speed limit ranges from 35 to 55 mph along SR 6 within the study area. For the Three (3) Lane Thompson's Station Option in the year 2014, travel speeds along the route are calculated by the HCS to range from 17.7 mph to 54.5 mph , with a weighted average of 32.2 mph. In 2034, the travel speed ranges from 16.3 mph to 54.4 mph with a weighted average of 30.8 mph . The average was weighted based upon the length of each segment analyzed. The weighted average of the speed limit along the route is 45.5 mph . The calculated average route speed is $71 \%$ and $68 \%$ of the posted speed limit in the years 2014 and 2034, respectively. The Nashville Area MPO's Congestion Management Process notes that if the average route speed is $70 \%$ or less of the free flow speed, the roadway is congested. Therefore, for the Three (3) Lane Thompson's Station Option, SR 6 is congested in 2034. A summary of the travel speed calculations for the Three (3) Lane Thompson's Station Option is provided in table form in Table 5.4.2. The travel speeds are reported for the years 2014 and 2034.

The existing SR 6 Corridor (Three (3) Lane Thompson's Station Option) between Kedron Road to the south and Mack Hatcher Parkway to the north is 10.9 miles in length. For the Three (3) Lane Thompson's Station Option in the year 2014, the travel time along SR 6 is calculated to be 20.2 minutes. In 2034 , the travel time is calculated to be 21.2 minutes.

The traffic analysis demonstrates that four (4) travel lanes are necessary along the route. Two travel lanes within Thompson's Station Town Limits are not adequate to meet the projected design year traffic demand. Therefore, this option does not adequately address the purpose and need of the project.

Table 4.1 Three (3) Lane Thompson’s Station Cost Estimate

| ID | Location/ Description | Existing Roadway Classification | Proposed <br> Roadway <br> Classification | Length of Const. (Miles) | Estimated Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Maury/ Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 4-Travel Lane Urban Other Principal Arterial | 0.84 | \$ 13,003,200 | \$ 16,405,200 |
| 2 | SR-6 from Maury/ Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.49 | \$ 19,571,400 | \$ 23,630,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 3-Lane Rural Minor Arterial | 2.26 | \$ 13,605,200 | \$ 14,373,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principa Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 87,740,000 | \$100,300,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to the construction length of 10.9 miles.


Figure 4.1 Three (3) Lane Thompson's Station 2014 LOS


Figure 4.2 Three (3) Lane Thompson's Station 2034 LOS

## Table 4.2 Three (3) Lane Thompson’s Station LOS Table



### 5.0 THREE (3) LANE SPRING HILL \& THOMPSON'S STATION OPTION

This option combines the Three (3) Lane Spring Hill and Three (3) Lane Thompson's Station Options to present a three (3) lane cross section through the City of Spring Hill's CBD and through the Town of Thompson's Station. The remainder of SR 6 will be improved to four (4) travel lanes. This option could be implemented as an interim option to investigate the congestion improvements created by the planned improvements adjacent to the study corridor.

The Three (3) Lane Spring Hill and Thompson's Station Option is estimated to cost between $\$ 77$ and $\$ 81$ million in year 2014 dollars. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate summary is provided in Table 5.1. The cost estimate calculations are provided in the Appendix.

For the Three (3) Lane Spring Hill and Thompson's Station Option, the HCS's analysis calculates LOS ranging from A to F along SR 6 through the year 2034. The LOS calculations do not take into effect the diminished traffic operations caused by the four (4) school zones along the route. A summary of the LOS calculations for the Three (3) Lane Spring Hill and Thompson's Station Option is provided in schematic form in Figures 5.1 and 5.2 and in table form in Table 5.2. The LOS are reported for the years 2014 and 2034. The LOS calculations demonstrate that four (4) travel lanes are necessary along the route. Two (2) travel lanes within Spring Hill's CBD and the Town of Thompson's Station are not adequate to meet the projected design year traffic demand.

For the Three (3) Lane Spring Hill and Thompson's Station Option in the year 2014, the volume to capacity ratio $(\mathrm{v} / \mathrm{c})$ of SR 6 is calculated to range from 0.23 to 1.02 , with a weighted average of 0.53 . In 2034, the v/c ranges from 0.28 to 1.50 with a weighted average of 0.68 . The average was weighted based upon the length of each segment analyzed. A v/c ratio near or above " 1 " indicates a roadway experiences congestion. The v/c calculations demonstrate that four (4) travel lanes are necessary along the route to meet the projected traffic demand. A summary of the v/c calculations for the Three Lane Spring Hill and Thompson's Station Option is provided in table form in Table 5.2. The v/c are reported for the years 2014 and 2034.

The posted speed limit ranges from 35 to 55 mph along SR 6 within the study area. For the Three (3) Lane Spring Hill and Thompson's Station Option in the year 2014, travel speeds along the route are calculated by the HCS to range from 14.6 mph to 54.5 mph , with a weighted average of 31.2 mph . In 2034, the travel speed ranges from 3.5 mph to 54.4 mph with a weighted average of 33.1 mph . The average was weighted based upon the length of each segment analyzed. The weighted average of the speed limit along the route is 45.5 mph . The calculated average route speed is $69 \%$ and $43 \%$ of the posted speed limit in the years 2014 and 2034, respectively. The Nashville Area MPO's Congestion Management Process notes that if the average route speed is $70 \%$ or less of the free flow speed, the roadway is congested. Therefore, for the Three (3) Lane Spring Hill and Thompson's Station Option, SR 6 is congested in 2034. A summary of the travel speed calculations for this option is provided in table form in Table 5.2. The travel speeds are reported for the years 2014 and 2034.

The existing SR 6 Corridor (Three (3) Lane Spring Hill and Thompson's Station Option) between Kedron Road to the south and Mack Hatcher Parkway to the north is 10.9 miles in length. For the Three (3) Lane Spring Hill and Thompson's Station Option in the year 2014, the travel time
along SR 6 is calculated to be 20.9 minutes. In 2034, the travel time is calculated to be 33.1 minutes.

The traffic analysis demonstrates that four (4) travel lanes are necessary along the route. Two travel lanes within Spring Hill's City Limits and Thompson's Station Town Limits are not adequate to meet the projected design year traffic demand. Therefore, this option does not adequately address the purpose and need of the project.

Table 5.1 Three (3) Lane Spring Hill and Thompson’s Station Option Cost Estimate

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Maury/ Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.84 | \$ 722,400 | \$ 722,400 |
| 2A | SR-6 from Maury/ Williamson County Line to Miles Johnson Pkwy. | 2-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.20 | \$ | \$ |
| 2 | SR-6 from Miles Johnson Pkwy. to Thompson's Station City Limits (L.M. 0.20 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.29 | \$ 16,281,900 | \$ 19,991,700 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 13,605,200 | \$ 14,373,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 72,170,000 | \$ 80,980,000 |

Notes: Existing and proposed 3-lane sections are urban x-section. Multilane sections as described below.
Low estimate utilizes 4 -lane rural x -section, high estimate utilizes 5 -lane urban x -section.
Total route length is equal to the construction length of 10.9 miles.


Figure 5.1 Three (3) Lane Spring Hill and Thompson’s Station Option 2014 LOS


Figure 5.2 Three (3) Lane Spring Hill and Thompson’s Station Option 2034 LOS

Table 5.2 Three (3) Lane Spring Hill and Thompson’s Station Option LOS Table


### 6.0 SPRING HILL ONE-WAY PAIR OPTION

It was briefly investigated to utilize School Street in a one-way pair configuration with SR 6 through Spring Hill's CBD. Existing SR 6 would provide southern travel lanes and School Street would provide northern travel lanes. This option did not have the support of the City of Spring Hill. Furthermore, upon site inspection, School Street is a poor candidate for improvements. The vertical geometry and the cross-section of the route are poor, being the functional equivalent of an alley. Numerous historic and religious properties are adjacent to the route. A connector to provide access to Existing SR 6 would intersect The Tennessee Orphans home Historic Site. Therefore, this option was not investigated further.


Figure 6 One-Way Pair Detail

### 7.0 SPRING HILL BYPASS OPTIONS

An Eastern and a Western Spring Hill Bypass Option were studied. The Bypass Options were developed through a relatively informal stakeholder's meeting without public participation or input. Significant changes to the project, including a proposed Bypass Option, should originate through the local, county, or MPO planning process. Additionally, there are several functional issues that led to the Bypass options not being included in the TPR, including lack of route continuity and access control issues. It should be noted that further refinement of the Bypass Options through the MPO's planning process may create a viable option to meet the Purpose and Need of the project.


Figure 7 Bypass Options Map

### 7.1 EASTERN SPRING HILL BYPASS OPTION

This option would provide an eastern bypass around the CBD of the City of Spring Hill. The option would originate along SR 6 at the existing five (5) lane cross section south of Saturn Parkway (SR 396). The option would utilize existing Saturn Parkway (SR 396), Kedron Road, Old Kedron Road, and proposed Miles Johnson Parkway to route SR 6 around the CBD. The rejected Eastern Spring Hill Bypass is 3.3 miles long. The distance along existing SR 6 that would be bypassed is 2.1 miles long. This option was developed due to the City of Spring Hill not being in support of widening SR 6 within the CBD. Widening SR 6 within the CBD will require impacts to numerous religious, historic, and commercial properties and harm the City's efforts to create a more walkable downtown community.

The Eastern Spring Hill Bypass Option would improve Kedron Road, Old Kedron Road, and Miles Johnson Parkway to form a continuous route. Saturn Parkway (SR 396) would be utilized without improvements. The existing three (3) lane cross section of SR 6 in the CBD would not be impacted. The remainder of SR 6 would be improved to four (4) travel lanes. This option was developed in coordination with representatives from the City of Spring Hill to eliminate impacts to their CBD.

The option would utilize 1.4 miles of Saturn Parkway (SR 396) from SR 6 to the Kedron Road Interchange. The option would then provide improvements along 1.9 miles of Kedron Road, Old Kedron Road, and proposed Miles Johnson Parkway. The improvements along these three (3) routes would create a continuous route and are estimated to require widening these routes from their existing two (2) travel lanes to four (4) travel lanes. The southern terminus of the bypass would utilize the existing interchange between SR 6 and Saturn Parkway (SR 396). Local officials noted a need to improve the existing flyover ramp from Saturn Parkway (SR 396) Westbound to SR 6 Southbound for safety reasons. Improvements to this ramp are not included in this option and could be investigated in a separate project. The northern terminus of the bypass could be adjusted to create a continuous route between SR 6 to the north and the proposed bypass to the south, or maintain the existing configuration where existing SR 6 is the continuous route. Constructing a roundabout at this intersection is another option. It is anticipated that the existing interchange between Saturn Parkway (SR 396) and Kedron Road could be utilized with only minor improvements at the ramp termini.

Several environmentally sensitive locations and community resources are located, or are planned to be located, within the Eastern Spring Hill Bypass Corridor. A segment of The Spring Hill Civil War Battlefield is located within the Corridor. The battlefield is located north of Saturn Parkway (SR 396) and west and south of Kedron Road. It was also noted by local officials that wetlands are present in the area. A new fifty-three (53) bed hospital will be located within the Eastern Spring Hill Bypass Corridor adjacent to the proposed Miles Johnson Parkway. Additional development in the area is anticipated with construction of the Miles Johnson Parkway.

The Eastern Spring Hill Bypass Option is estimated to cost between $\$ 91$ and $\$ 105$ million in year 2014 dollars. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate includes between $\$ 17$ and $\$ 20$ million to improve Kedron

Road, Old Kedron Road, and Miles Johnson Parkway to create the bypass; and between $\$ 74$ and $\$ 85$ million to widen SR 6 from Miles Johnson Parkway to Mack Hatcher Parkway to four (4) travel lanes. The cost calculations for the improvements to Kedron Road, Old Kedron Road, and proposed Miles Johnson Parkway include widening these roads from their existing two (2) travel lanes to four (4) travel lanes. The cost estimate does not include improvements along Saturn Parkway (SR 396) or improvements at the interchanges between Saturn Parkway (SR 396) and SR 6 and Kedron Road, because improvements are not anticipated to be needed at these locations for adequate traffic operations. The cost estimate summary is provided in Table 7.1. The cost estimate calculations are provided in the Appendix.

No HCS LOS analysis was performed for the Eastern Spring Hill Bypass Option. This option was developed as a result of feedback received during stakeholder meetings that occurred during the development of this TPR. As discussed in the TPR, representatives of the City of Spring Hill are not in favor of widening SR 6 through the City's CBD. An Eastern Spring Hill Bypass Option is not in the Nashville MPO's LRTP. Therefore, this improvement is not in their Travel Demand Model and traffic projections are not currently available for this option.

This option was not included in the main body of the TPR in part because it is not currently included in the MPO's planning process. The bypass option was developed through a relatively informal stakeholder's meeting without public participation or input. Significant changes to the project, including a proposed bypass option, should originate through the local, county, or MPO planning process. Additionally, there are several functional issues that led to this option being rejected, including route continuity and access control issues. Even with improvements along the bypass corridor, the Eastern Spring Hill Bypass Option would not be continuous, routing SR 6 traffic along Saturn Parkway. It would be infeasible to provide access control measures along the bypass due to the existing development along the proposed route. Additionally, rerouting SR 6 along the existing Kedron Road, Old Kedron Road, and Miles Johnson Parkway corridors would reallocate truck traffic to these primarily residential areas. Several environmentally sensitive locations are also located adjacent to the route. Finally, the existence of I-65 less than 3.5 miles to the east of SR 6 creates questions concerning the regional significance of a bypass at this location. Due to these concerns, the Spring Hill community may be better served by maintaining Kedron Road, Old Kedron Road, and Miles Johnson Parkway as local routes, and not utilizing them as a bypass.

Table 7.1 Eastern Spring Hill Bypass Cost Estimate

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated <br> High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. | Eastern Bypass from Saturn Parkway (SR-396) to SR-6 at Miles Johnson Parkway | n/a | 4-Travel Lane Urban Other Principal Arterial | 1.90 | \$ 16,511,000 | \$ 19,836,000 |
| 1 | SR-6 from Kedron Rd. to Maury/ Williamson County Line (L.M. 32.47 to 33.31 ) | 3-Lane Urban Other Principal Arterial | 2-Travel Lane Urban Other Principal Arterial | 0.00 | \$ | \$ |
| 2 | SR-6 from Maury/ Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 2/4 TravelLane Urban Other Principal Arterial | 2.29 | \$ 17,999,400 | \$ 21,732,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tolligate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 11.73 | \$ 91,490,000 | \$104,880,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section. Total route length is equal to 13.1 miles. 1.4 miles of existing Saturn Parkway is utilized.

### 7.2 WESTERN SPRING HILL BYPASS OPTION

This option would provide a western bypass around the CBD of the City of Spring Hill. The option would originate at the intersection of SR 6 with Stephen P. Yokich Parkway/Town Center Parkway. The option would utilize the 0.7 mile long Stephen P. Yokich/Town Center Parkway to its existing terminus with Beechcroft Road (SR 247), then proceed north on new alignment for 2.53 miles, and terminate near the intersection of SR 6 with Cemetery Road. The section of existing SR 6 between Stephen P. Yokich

Parkway/Town Center Parkway and the northern terminus of the bypass would not be improved. The remainder of SR 6 would be improved to four (4) travel lanes.

The proposed Western Spring Hill Bypass is 3.2 miles long. The 3.2-mile long bypass route includes 0.7 miles along existing Stephen P. Yokich Parkway/Town Center Parkway and 2.5 miles along new location. The distance along existing SR 6 that is being bypassed is 2.4 miles long.

As discussed in Section 5.2, the City of Spring Hill does not support widening SR 6 within the CBD. Widening SR 6 within the CBD will require impacts to numerous religious, historic, and commercial properties and harm the City's efforts to create a more walkable downtown community.

Stephen P. Yokich Parkway/Town Center Parkway would be utilized as a segment of the proposed Western Spring Hill Bypass. Stephen P. Yokich Parkway/Town Center Parkway is four (4) travel lanes wide with a raised median, curb and gutter, and no shoulders. The existing speed limit of Stephen P. Yokich Parkway/Town Center Parkway is 35 miles per hour.

This option is similar to improvements noted in the City of Spring Hill's Major Thoroughfare Plan (MTP). The route listed in the MTP extends from Stephen P. Yokich Parkway/Town Center Parkway to Buckner Road. This route would bisect the Campbell Farm. The Campbell Farm is listed in the National Register of Historic Places and is a Tennessee Century Farm. The MTP Route would also bisect the historic James Giddens House property and be located adjacent to Heritage Elementary and Middle Schools. Due to these environmentally sensitive locations, the northern terminus of the route in the MTP was modified as shown in this TPR to connect to SR 6 near Cemetery Road, instead of Buckner Road.

The Western Spring Hill Bypass is estimated to require four (4) travel lanes. The intersections created by the termini of the Western Spring Hill Bypass with SR 6 would require improvements to ensure that the anticipated heavy turning movement volumes are addressed. Potential intersection improvements at these locations could include multilane roundabouts.

A couple of environmentally sensitive locations and community resources are located within or adjacent to the Western Spring Hill Bypass Corridor. The Spring Hill Water Treatment Plant is located within the corridor, but will not be impacted due to its location adjacent to the existing segment of Town Center Parkway. The Campbell farm is located adjacent to the corridor. The Belshire Village commercial development, which is anchored by a Lowe's Home Improvement Store, is located within the study corridor at the northern terminus. It is believed the existing commercial developments can be avoided by the proposed bypass. The Western Spring Hill Bypass Corridor crosses the CSX Corporation Railroad in two (2) locations. These locations will need to be grade separated.

The Western Spring Hill Bypass Option is estimated to cost between \$114 and \$127 million in year 2014 dollars. The estimated cost was calculated to a planning level utilizing cost per mile data. The cost estimate includes between $\$ 47$ and $\$ 52$ million to construct the four travel lane Western Spring Hill Bypass on new location and between $\$ 67$ and $\$ 75$ million to widen SR 6 from Cemetery Road to Mack Hatcher Parkway to four (4) travel lanes. The cost estimate includes structures to bridge the two (2) CSX Corporation
railroad crossings located along the Western Bypass route. The cost estimate does not include improvements along the existing segment of Stephen P. Yokich/Town Center Parkway that is utilized by the Western Bypass. The cost estimate summary is provided in Table 7.2. The cost estimate calculations are provided in the Appendix.

No HCS LOS analysis was performed for the Western Spring Hill Bypass Option. This option was developed as a result of feedback received during stakeholder meetings that occurred during the development of this TPR. As discussed in the TPR, representatives of the City of Spring Hill are not in favor of widening SR 6 through the City's CBD. A Western Spring Hill Bypass Option is not in the Nashville MPO's LRTP. Therefore, this improvement is not in their Travel Demand Model and traffic projections are not currently available for this option.

This option was not included in the main body of the TPR in part because it is not currently included in the MPO's planning process. The bypass option was developed through a relatively informal stakeholder's meeting without public participation or input. Significant changes to the project, including a proposed bypass option, should originate through the local, county, or MPO planning process. Additionally, there are several functional issues that led to this option being rejected. SR 6 would not be a continuous route if redirected to this bypass and the pavement design and geometrics of Stephen P. Yokich Parkway/Town Center Parkway to be utilized in the bypass may not be sufficient for a State Route. The intersections formed between the bypass and SR 6 would likely not operate well due to the large turning volumes. A couple of environmentally sensitive locations are also located along the route. Finally, the existence of I-65 less than 3.5 miles to the east of SR 6 creates questions concerning the regional significance of a bypass at this location. Due to these concerns, this option was not included in the main body of the TPR.

Table 7.2 Western Spring Hill Bypass Cost Estimate

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated <br> High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. | Western Bypass from Stephen P. Yokich Parkway/Town Center Parkway to Cemetery Road | n/a | 4-Travel Lane Urban Other Principal Arterial | 2.53 | \$ 47,488,100 | \$ 51,915,600 |
| 1 | SR-6 from Kedron Rd. to Maury/ Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 2-Travel Lane Urban Other Principal Arterial | 0.00 | \$ | \$ |
| 2 | SR-6 from Maury/ Williamson County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 2/4 TravelLane Urban Other Principal Arterial | 1.19 | \$ 9,353,400 | \$ 11,293,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ - |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 11.26 | \$113,820,000 | \$126,520,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section. Total route length is equal to 11.9 miles. 0.65 miles of existing Town Center Parkway is utilized.

## COST ESTIMATES

Cost calculation summaries
Cost per mile calculations
Construction and engineering cost per mile calculations
Construction item cost per mile calculations
Structures calculations
Utility calculations
R.O.W. calculations

Paving cost calculations

## Widen Along the Existing Alignment

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 4-Travel Lane Urban Other Principal Arterial | 0.84 | \$ 13,003,200 | \$ 16,405,200 |
| 2 | SR-6 from Maury County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.49 | \$ 19,571,400 | \$ 23,630,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 89,550,000 | \$103,350,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to the construction length of 10.9 miles.

## Three Lane Spring Hill Option

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31 ) | 3-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.84 | \$ 722,400 | \$ 722,400 |
| 2A | SR-6 from Maury County Line to Miles Johnson Pkwy. | 2-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.20 | \$ | \$ |
| 2 | SR-6 from Miles Johnson Pkwy. to Thompson's Station City Limits (L.M. 0.20 to 2.49 ) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.29 | \$ 16,281,900 | \$ 19,991,700 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 73,980,000 | \$ 84,030,000 |

Notes: Existing and proposed 3-lane sections are urban x-section. Multilane sections as described below.
Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5 -lane urban $x$-section.
Total route length is equal to the construction length of 10.9 miles.

## Three Lane Thompson's Station Option

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31 ) | 3-Lane Urban Other Principal Arterial | 4-Travel Lane Urban Other Principal Arterial | 0.84 | \$ 13,003,200 | \$ 16,405,200 |
| 2 | SR-6 from Maury County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.49 | \$ 19,571,400 | \$ 23,630,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 3-Lane Rural Minor Arterial | 2.26 | \$ 13,605,200 | \$ 14,373,600 |
| 4 | SR-6 from south of SR-.......................... to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8. 8.67 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 87,740,000 | \$100,300,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to the construction length of 10.9 miles.

## Three Lane Spring Hill \& Thompson's Station Option

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31 ) | 3-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.84 | \$ 722,400 | \$ 722,400 |
| 2A | SR-6 from Maury County Line to Miles Johnson Pkwy. | 2-Lane Urban Other Principal Arterial | 3-Lane Urban Other Principal Arterial | 0.20 | \$ | \$ |
| 2 | SR-6 from Miles Johnson Pkwy. to Thompson's Station City Limits (L.M. 0.20 to 2.49 ) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 2.29 | \$ 16,281,900 | \$ 19,991,700 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 13,605,200 | \$ 14,373,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 10.87 | \$ 72,170,000 | \$ 80,980,000 |

Notes: Existing and proposed 3-lane sections are urban x-section. Multilane sections as described below.
Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5 -lane urban $x$-section.
Total route length is equal to the construction length of 10.9 miles.

## Eastern Spring Hill Bypass

| ID | Location/ Description | Existing Roadway Classification | Proposed <br> Roadway <br> Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. | Eastern Bypass from Saturn Parkway (SR-396) to SR-6 at Miles Johnson Parkway | n/a | 4-Travel Lane Urban Other Principal Arterial | 1.90 | \$ 16,511,000 | \$ 19,836,000 |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 2-Travel Lane Urban Other Principal Arterial | 0.00 | \$ | \$ |
| 2 | SR-6 from Maury County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 2/4 Travel- <br> Lane Urban Other Principal Arterial | 2.29 | \$ 17,999,400 | \$ 21,732,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75 ) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-840 to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 11.73 | \$ 91,490,000 | \$104,880,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to 13.1 miles. 1.4 miles of existing Saturn Parkway is utilized.

## Western Spring Hill Bypass

| ID | Location/ Description | Existing Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) | Estimated <br> Low Cost | Estimated High Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. | Western Bypass from Stephen P. Yokich Parkway/Town Center Parkway to Cemetery Road | n/a | 4-Travel Lane Urban Other Principal Arterial | 2.53 | \$ 47,488,100 | \$ 51,915,600 |
| 1 | SR-6 from Kedron Rd. to Williamson County Line (L.M. 32.47 to 33.31) | 3-Lane Urban Other Principal Arterial | 2-Travel Lane Urban Other Principal Arterial | 0.00 | \$ | \$ |
| 2 | SR-6 from Maury County Line to Thompson's Station City Limits (L.M. 0 to 2.49) | 2-Lane Urban Other Principal Arterial | 2/4 TravelLane Urban Other Principal Arterial | 1.19 | \$ 9,353,400 | \$ 11,293,100 |
| 3 | SR-6 from Thompson's Station City Limits to south of SR-840 (L.M. 2.5 to 4.75) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 2.26 | \$ 15,413,200 | \$ 17,424,600 |
| 4 | SR-6 from south of SR-.................................. to Tollgate Rd. (L.M. 4.76 to 5.55) | 5-Lane Rural Minor Arterial | 5-Lane Rural Minor Arterial | 0.80 | \$ | \$ |
| 5 | SR-6 from Tollgate Rd. to Nashville Urban Boundary (L.M. 5.56 to 8.67) | 2-Lane Rural Minor Arterial | 4 Travel-Lane Rural Minor Arterial | 3.12 | \$ 25,989,600 | \$ 28,766,400 |
| 6 | SR-6 from Nashville Urban Boundary to SR-397 (Mack Hatcher Pkwy.) (L.M. 8.68 to 10.03) | 2-Lane Urban Other Principal Arterial | 4 Travel-Lane Urban Other Principal Arterial | 1.36 | \$ 15,572,000 | \$ 17,122,400 |
| Total |  |  |  | 11.26 | \$113,820,000 | \$126,520,000 |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to 11.9 miles. 0.65 miles of existing Town Center Parkway is utilized.

Spot Improvements

| ID | Location/ Description | Existing <br> Roadway Classification | Proposed Roadway Classification | Length of Const. (Miles) |  | stimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| n/a | Signalization | n/a | n/a | 0.00 | \$ | 150,000 |
| n/a | Signalization | n/a | n/a | 0.00 | \$ | 150,000 |
| n/a | Turn Lane Improvements | n/a | n/a | 0.00 | \$ | 300,000 |
| n/a | Turn Lane Improvements | n/a | n/a | 0.00 | \$ | 300,000 |
| n/a | Turn Lane Improvements | n/a | n/a | 0.00 | \$ | 300,000 |
| n/a | Bridge over Harpeth | n/a | n/a | 0.00 | \$ | 1,512,000 |
| n/a | SR-6 at SR-247 | n/a | n/a | 0.00 | \$ | 740,000 |
| n/a | SR-6 at Critz Lane | n/a | n/a | 0.19 | \$ | 950,000 |
| n/a | SR-6 at Goose Creek Bypass | n/a | n/a | 0.57 | \$ | 5,680,000 |
| n/a | SR-6 at Coleman Road/Henpeck Lane | n/a | n/a | 0.28 | \$ | 1,950,000 |
| n/a | SR-6 at Mack Hatcher Parkway | n/a | n/a | 3.12 | \$ | 200,000 |
| Total |  |  |  | 4.16 | \$ 12,232,000 |  |

Notes: Low estimate utilizes 4-lane rural x-section, high estimate utilizes 5-lane urban x-section.
Total route length is equal to 11.9 miles. 0.7 miles of existing Town Center Parkway is utilized.

| ID | Low Roadway Construction \& Engineering Cost |  | High Roadway Construction \& Engineering Cost |  |  | ow ROW Cost |  | High ROW Cost |  | Structures Cost | Low Total Estimated Cost | High Total Estimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | \$ | 5,594,000 |  | 6,165,000 | \$ | 9,881,000 |  | 13,369,000 | \$ | - | \$ 15,480,000 | \$ 19,530,000 |
| 2 | \$ | 5,594,000 |  | 6,165,000 | \$ | 1,511,000 | \$ | 2,569,000 | \$ | 756,000 | \$ 7,860,000 | \$ 9,490,000 |
| 3 | \$ | 5,594,000 |  | 6,165,000 | \$ | 465,000 | \$ | 791,000 | \$ | 756,000 | \$ 6,820,000 | \$ 7,710,000 |
| 4 | \$ | - | \$ | - | \$ | - | \$ | - | \$ | - | \$ | \$ |
| 5 | \$ | 5,594,000 |  | 6,165,000 | \$ | 465,000 |  | 791,000 |  | 2,268,000 | \$ 8,330,000 | \$ 9,220,000 |
| 6 |  | 5,594,000 |  | 6,165,000 |  | 814,000 |  | 1,383,000 | \$ | 5,040,000 | \$ 11,450,000 | \$ 12,590,000 |


| ID | Low Roadway Construction \& Engineering Cost |  | High Roadway Construction \& Engineering Cost |  | Low ROW Cost |  | High ROW Cost |  | Structures Cost |  | Low Total Estimated Cost |  | High Total Estimated Cost |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | \$ | 857,000 |  | 857,000 | \$ | - | \$ | - | \$ | - | \$ | 860,000 | \$ | 860,000 |
| 2A | \$ | 4,797,000 |  | 4,812,000 | \$ | 465,000 | \$ | 791,000 | \$ | 756,000 | \$ | 6,020,000 | \$ | 6,360,000 |
| 2 |  | 5,594,000 |  | 6,165,000 | \$ | 1,511,000 | \$ | 2,569,000 | \$ | - | \$ | 7,110,000 | \$ | 8,730,000 |
| 3 | \$ | 5,594,000 |  | 6,165,000 | \$ | 465,000 | \$ | 791,000 | \$ | 756,000 | \$ | 6,820,000 | \$ | 7,710,000 |
| 4 | \$ | - | \$ | - | \$ | - | \$ | - | \$ | - | \$ | - | \$ | - |
| 5 | \$ | 5,594,000 |  | 6,165,000 | \$ | 465,000 | \$ | 791,000 | \$ | 2,268,000 | \$ | 8,330,000 | \$ | 9,220,000 |
| 6 | \$ | 5,594,000 |  | 6,165,000 | \$ | 814,000 | \$ | 1,383,000 | \$ | 5,040,000 |  | 11,450,000 |  | 12,590,000 |


| ID | Low Roadway Construction \& Engineering Cost | High Roadway Construction \& Engineering Cost |  | ow ROW Cost | High ROW Cost | Structures Cost | Low Total Estimated Cost | High Total Estimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | \$ 5,594,000 | \$ 6,165,000 | \$ | 9,881,000 | \$ 13,369,000 | \$ | \$ 15,480,000 | \$ 19,530,000 |
| 2 | \$ 5,594,000 | \$ 6,165,000 | \$ | 1,511,000 | \$ 2,569,000 | \$ 756,000 | \$ 7,860,000 | \$ 9,490,000 |
| 3 | \$ 4,797,000 | \$ 4,812,000 | \$ | 465,000 | \$ 791,000 | \$ 756,000 | \$ 6,020,000 | \$ 6,360,000 |
| 4 | \$ | \$ | \$ | - | \$ | \$ | \$ | \$ |
| 5 | \$ 5,594,000 | \$ 6,165,000 | \$ | 465,000 | \$ 791,000 | \$ 2,268,000 | \$ 8,330,000 | \$ 9,220,000 |
| 6 | \$ 5,594,000 | \$ 6,165,000 | \$ | 814,000 | \$ 1,383,000 | \$ 5,040,000 | \$ 11,450,000 | \$ 12,590,000 |

Three Lane Spring Hill \& Thompson's Station Option
Cost per mile Calculations

| ID | Low Roadway Construction \& Engineering Cost |  | h Roadway struction \& ngineering Cost |  | ow ROW Cost |  | igh ROW Cost |  | Structures Cost | Low Total Estimated Cost | High Total Estimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | \$ 857,000 |  | 857,000 | \$ | - | \$ | - | \$ | - | \$ 860,000 | \$ 860,000 |
| 2A | \$ 4,797,000 |  | 4,812,000 | \$ | 465,000 | \$ | 791,000 | \$ | 756,000 | \$ 6,020,000 | \$ 6,360,000 |
| 2 | \$ 5,594,000 |  | 6,165,000 | \$ | 1,511,000 | \$ | 2,569,000 | \$ | - | \$ 7,110,000 | \$ 8,730,000 |
| 3 | \$ 4,797,000 |  | 4,812,000 | \$ | 465,000 | \$ | 791,000 | \$ | 756,000 | \$ 6,020,000 | \$ 6,360,000 |
| 4 | \$ | \$ | - | \$ | - | \$ | - | \$ | - | \$ | \$ |
| 5 | \$ 5,594,000 |  | 6,165,000 | \$ | 465,000 | \$ | 791,000 | \$ | 2,268,000 | \$ 8,330,000 | \$ 9,220,000 |
| 6 | \$ 5,594,000 |  | 6,165,000 | \$ | 814,000 | \$ | 1,383,000 | \$ | 5,040,000 | \$ 11,450,000 | \$ 12,590,000 |


| ID | Low Roadway Construction \& Engineering Cost |  | High Roadway Construction \& Engineering Cost |  |  | Low ROW Cost |  | High ROW Cost |  | Structures Cost | Low Total Estimated Cost | High Total Estimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. |  | 7,894,000 |  | 9,368,000 | \$ | 791,000 |  | 1,070,000 | \$ | - | \$ 8,690,000 | \$ 10,440,000 |
| 1 |  | 5,594,000 |  | 6,165,000 | \$ | 9,881,000 |  | 13,369,000 | \$ | - | \$ 15,480,000 | \$ 19,530,000 |
| 2 | \$ | 5,594,000 |  | 6,165,000 | \$ | 1,511,000 |  | 2,569,000 | \$ | 756,000 | \$ 7,860,000 | \$ 9,490,000 |
| 3 |  | 5,594,000 |  | 6,165,000 | \$ | 465,000 |  | 791,000 | \$ | 756,000 | \$ 6,820,000 | \$ 7,710,000 |
| 4 | \$ | - | \$ | - | \$ | - | \$ | - | \$ | - | \$ | \$ |
| 5 |  | 5,594,000 |  | 6,165,000 | \$ | 465,000 |  | 791,000 | \$ | 2,268,000 | \$ 8,330,000 | \$ 9,220,000 |
| 6 |  | 5,594,000 |  | 6,165,000 |  | 814,000 |  | 1,383,000 | \$ | 5,040,000 | \$ 11,450,000 | \$ 12,590,000 |


| ID | Low Roadway Construction \& Engineering Cost | High Roadway Construction \& Engineering Cost |  | ow ROW Cost | High ROW Cost | Structures Cost | Low Total Estimated Cost | High Total Estimated Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Byp. | \$ 7,894,000 | \$ 9,368,000 | \$ | 791,000 | \$ 1,070,000 | \$ 10,080,000 | \$ 18,770,000 | \$ 20,520,000 |
| 1 | \$ 5,594,000 | \$ 6,165,000 | \$ | 9,881,000 | \$ 13,369,000 | \$ | \$ 15,480,000 | \$ 19,530,000 |
| 2 | \$ 5,594,000 | \$ 6,165,000 | \$ | 1,511,000 | \$ 2,569,000 | \$ 756,000 | \$ 7,860,000 | \$ 9,490,000 |
| 3 | \$ 5,594,000 | \$ 6,165,000 | \$ | 465,000 | \$ 791,000 | \$ 756,000 | \$ 6,820,000 | \$ 7,710,000 |
| 4 | \$ | \$ | \$ | - | \$ | \$ | \$ | \$ |
| 5 | \$ 5,594,000 | \$ 6,165,000 | \$ | 465,000 | \$ 791,000 | \$ 2,268,000 | \$ 8,330,000 | \$ 9,220,000 |
| 6 | \$ 5,594,000 | \$ 6,165,000 |  | 814,000 | \$ 1,383,000 | \$ 5,040,000 | \$ 11,450,000 | \$ 12,590,000 |



## Route Construction \& Engineering Cost Data Sheet

Option: 2 lane existing to 3 lane rural cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | 478,500 |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 60,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | 957,000 |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$ 1,958,500$ |  |
| Construction Item Subtotal | $\$$ | 6,000 |
| Other Construction Items (15\%) | $\$$ | 294,000 |
| Erosion Control (3.5\%) | $\$$ | 69,000 |
| Mobilization (5\%) | $\$$ | 98,000 |


| Construction Cost | $\$$ | $2,419,500$ |
| :--- | :--- | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 242,000 |

Total Construction Cost

| $\$$ | $2,661,500$ |
| :--- | ---: |
| $\$$ | 266,000 |

Total Cost (Rounded) Present Year AUP (2007)
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years

| $\$$ | $3,190,000$ |
| :---: | ---: |
| $\$$ | $4,797,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: 2 lane existing to 3 lane urban cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | 214,000 |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 435,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | 856,000 |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$ 1,968,000$ |  |
| Construction Item Subtotal | $\$$ | 6,000 |
| Other Construction Items (15\%) | $\$$ | 295,000 |
| Erosion Control (3.5\%) | $\$$ | 69,000 |
| Mobilization (5\%) | $\$$ | 98,000 |


| Construction Cost | $\$$ | $2,430,000$ |
| :--- | ---: | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 243,000 |

Total Construction Cost

| $\$$ | $2,673,000$ |
| :--- | ---: |
| $\$$ | 267,000 |

Total Cost (Rounded) Present Year AUP (2007)

| $\$$ | $3,200,000$ |
| :--- | :--- |
| $\$$ | $4,812,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: 2 lane existing to 4 lane rural cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :---: | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing |  | \$ | 20,000 | Structures estimated separately |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Earthwork |  | \$ | 598,500 |  |  |  |  |
| Pavement Removal |  | \$ | 5,000 |  |  |  |  |
| Drainage |  | \$ | 60,000 |  |  |  |  |
| Structures |  | \$ | - |  |  |  |  |
| Railroad Crossing or Separation |  | \$ | - |  |  |  |  |
| Paving |  | \$ | 1,197,000 |  |  |  |  |
| Retaining Walls |  | \$ | 71,000 |  |  |  |  |
| Maintenance of Traffic |  | \$ | 100,000 |  |  |  |  |
| Topsoil |  | \$ | 20,000 |  |  |  |  |
| Seeding |  | \$ | 7,000 |  |  |  |  |
| Sodding |  | \$ | - |  |  |  |  |
| Signing |  | \$ | 5,000 |  |  |  |  |
| Lighting |  | \$ | 25,000 |  |  |  |  |
| Signalization |  | \$ | 150,000 |  |  |  |  |
| Fence |  | \$ | - |  |  |  |  |
| Guardrail |  | \$ | 54,000 |  |  |  |  |
| Rip Rap or Slope Protection |  | \$ | 6,000 |  |  |  |  |
| Construction Item Subtotal | \$ 2,318,500 |  |  |  |  |  |  |
| Other Construction Items (15\%) |  | \$ | 348,000 |  |  |  |  |
| Erosion Control (3.5\%) |  | \$ | 81,000 |  |  |  |  |
| Mobilization (5\%) |  | \$ | 116,000 |  |  |  |  |
| Construction Cost |  |  |  | \$ | 2,863,500 |  |  |
| 10\% Engineering \& Cont. |  |  |  | \$ | 286,000 |  |  |
| Total Construction Cost |  |  |  |  |  | \$ | 3,149,500 |
| Preliminary Engineering (10\%) |  |  |  |  |  | \$ | 315,000 |

Total Cost (Rounded) Present Year AUP (2007)
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years

| $\$$ | $3,720,000$ |
| :---: | ---: |
| $\$$ | $5,594,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: 2 lane existing to 5 lane urban cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | 333,750 |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 435,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | $1,335,000$ |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$, 566,750$ | $\$$ |
| Construction Item Subtotal |  | 6,000 |
| Other Construction Items (15\%) | $\$$ | 385,000 |
| Erosion Control (3.5\%) | $\$$ | 90,000 |
| Mobilization (5\%) | $\$$ | 128,000 |


| Construction Cost | $\$$ | $3,169,750$ |
| :--- | :--- | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 317,000 |

Total Construction Cost

| $\$$ | $3,486,750$ |
| :--- | ---: |
| $\$$ | 349,000 |

Total Cost (Rounded) Present Year AUP (2007)

| $\$$ | $4,100,000$ |
| :---: | :---: |
| $\$$ | $6,165,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: 3 lane existing to 4 lane rural cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing |  | \$ | 20,000 | Structures estimated separately |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Earthwork |  | \$ | 598,500 |  |  |  |  |
| Pavement Removal |  | \$ | 5,000 |  |  |  |  |
| Drainage |  | \$ | 60,000 |  |  |  |  |
| Structures |  | \$ | - |  |  |  |  |
| Railroad Crossing or Separation |  | \$ | - |  |  |  |  |
| Paving |  | \$ | 1,197,000 |  |  |  |  |
| Retaining Walls |  | \$ | 71,000 |  |  |  |  |
| Maintenance of Traffic |  | \$ | 100,000 |  |  |  |  |
| Topsoil |  | \$ | 20,000 |  |  |  |  |
| Seeding |  | \$ | 7,000 |  |  |  |  |
| Sodding |  | \$ | - |  |  |  |  |
| Signing |  | \$ | 5,000 |  |  |  |  |
| Lighting |  | \$ | 25,000 |  |  |  |  |
| Signalization |  | \$ | 150,000 |  |  |  |  |
| Fence |  | \$ | - |  |  |  |  |
| Guardrail |  | \$ | 54,000 |  |  |  |  |
| Rip Rap or Slope Protection |  | \$ | 6,000 |  |  |  |  |
| Construction Item Subtotal | \$ 2,318,500 |  |  |  |  |  |  |
| Other Construction Items (15\%) |  | \$ | 348,000 |  |  |  |  |
| Erosion Control (3.5\%) |  | \$ | 81,000 |  |  |  |  |
| Mobilization (5\%) |  | \$ | 116,000 |  |  |  |  |
| Construction Cost |  |  |  | \$ | 2,863,500 |  |  |
| 10\% Engineering \& Cont. |  |  |  | \$ | 286,000 |  |  |
| Total Construction Cost |  |  |  |  |  | \$ | 3,149,500 |
| Preliminary Engineering (10\%) |  |  |  |  |  | \$ | 315,000 |

Total Cost (Rounded) Present Year AUP (2007)
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years

| $\$$ | $3,720,000$ |
| :---: | :---: |
| $\$$ | $5,594,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: 3 lane existing to 5 lane urban cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | 333,750 |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 435,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | $1,335,000$ |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$, 566,750$ | $\$$ |
| Construction Item Subtotal |  | 6,000 |
| Other Construction Items (15\%) | $\$$ | 385,000 |
| Erosion Control (3.5\%) | $\$$ | 90,000 |
| Mobilization (5\%) | $\$$ | 128,000 |


| Construction Cost | $\$$ | $3,169,750$ |
| :--- | :--- | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 317,000 |

Total Construction Cost

| $\$$ | $3,486,750$ |
| :--- | ---: |
| $\$$ | 349,000 |

Total Cost (Rounded) Present Year AUP (2007)

| $\$$ | $4,100,000$ |
| :---: | :---: |
| $\$$ | $6,165,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: New 4 lane rural cross section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | $1,408,000$ |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 60,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | $1,408,000$ |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$ 3,339,000$ |  |
| Construction Item Subtotal | $\$$ | 6,000 |
| Other Construction Items (15\%) | $\$$ | 501,000 |
| Erosion Control (3.5\%) | $\$$ | 117,000 |
| Mobilization (5\%) | $\$$ | 167,000 |


| Construction Cost | $\$$ | $4,124,000$ |
| :--- | :--- | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 412,000 |
|  |  |  |
| Total Construction Cost |  |  |
| Preliminary Engineering (10\%) | $\$$ | $\mathbf{4 , 5 3 6 , 0 0 0}$ |

Total Cost (Rounded) Present Year AUP (2007)
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years

| $\$$ | $5,250,000$ |
| :---: | ---: |
| $\$$ | $7,894,000$ |

## Route Construction \& Engineering Cost Data Sheet

Option: New 5 lane urban cross Section
Length: 1 Mile

## Right-of-Way

TDOT cost per mile data utilzed \& varies by adjacent land use. Please see ROW cost calculations

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :---: |
| Non-reimbursable | $\$$ | 260,000 |

Total Utility Cost $\quad$| 260,000 |
| :--- |

## Construction

| Clear and Grubbing | $\$$ | 20,000 |
| :--- | ---: | ---: |
| Earthwork | $\$$ | $1,547,000$ |
| Pavement Removal | $\$$ | 5,000 |
| Drainage | $\$$ | 435,000 |
| Structures | $\$$ | - |
| Railroad Crossing or Separation | $\$$ | - |
| Paving | $\$$ | $1,547,000$ |
| Retaining Walls | $\$$ | 71,000 |
| Maintenance of Traffic | $\$$ | 100,000 |
| Topsoil | $\$$ | 20,000 |
| Seeding | $\$$ | 7,000 |
| Sodding | $\$$ | - |
| Signing | $\$$ | 5,000 |
| Lighting | $\$$ | 25,000 |
| Signalization | $\$$ | 150,000 |
| Fence | $\$$ | - |
| Guardrail | $\$$ | 54,000 |
| Rip Rap or Slope Protection | $\$, 992,000$ |  |
| Construction Item Subtotal | $\$$ | 6,000 |
| Other Construction Items (15\%) | $\$$ | 599,000 |
| Erosion Control (3.5\%) | $\$$ | 140,000 |
| Mobilization (5\%) | $\$$ | 200,000 |


| Construction Cost | $\$$ | $4,931,000$ |
| :--- | :---: | ---: |
| $10 \%$ Engineering \& Cont. | $\$$ | 493,000 |


| Total Construction Cost |
| :--- |
| Preliminary Engineering (10\%) |

Total Cost (Rounded) Present Year AUP (2007)
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years

## Route Construction \& Engineering Cost Data Sheet

Option: Rehabilitate 3 lane urban cross Section
Length: 1 Mile

## Right-of-Way

No R.O.W. Cost for Rehabilitated Section

Utility Relocation

| Reimbursable | $\$$ | - |
| :--- | :--- | :--- |
| Non-reimbursable |  |  |
| Total Utility Cost |  | $\$$ |

## Construction



Total Cost (Rounded) Present Year AUP (2007)
\$ 570,000
Total Cost (Rounded) Construction Year (2014) at 6\% Inflation for 7 Years



Total:
1,959,000


Total:
2,110,000


Total:
2,319,000


Total:
2,709,000


Total:
2,319,000


Total:
2,709,000



Total:
4,134,000


Total:

## SR-6 TPR Structures Cost Data Sheet

| Route | Log <br> Mile | Section <br> ID | Description | Estimated <br> Length <br> (ft.) | Estimated <br> Width (ft.) | Cost per <br> S.F. | Estimated <br> Structure <br> Cost |
| :--- | :---: | :---: | :--- | :---: | :---: | :---: | :---: |
| SR-6 | 0.17 | 2 | Crossing McCutcheon <br> Creek | 60 | 84 | $\$$ | 150 |
| SR-6 | 4.66 | 3 | Crossing Branch | 60 | 84 | $\$ 56,000$ |  |
| SR-6 | 5.72 | 5 | Crossing West <br> Harpeth River | 120 | 84 | $\$$ | 150 |
| SR-6 | 8.11 | 5 | Crossing Branch | 60 | $84,512,000$ |  |  |
| SR-6 | 8.89 | 6 | Exist. Crossing CSX | 200 | 84 | $\$$ | 150 |
| W. Bypass | n/a | n/a | Prop. Crossing CSX | 200 | 846,000 |  |  |
| W. Bypass | n/a | n/a | Prop. Crossing CSX | 200 | 84 | $\$$ | 300 |

## Route Utility Cost Estimate Calculations



## 2008 COST DATA SHEET

Right Of Way (ROW) Factor

| Area | Factor | $\begin{gathered} \frac{\text { TDOT }}{\text { Estimated }} \\ \text { Cost per Mile } \\ \hline \end{gathered}$ | For Widening |  | For New Const. |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Low <br> Estimate (50\%) | High <br> Estimate (85\%) | Low <br> Estimate (85\%) | $\frac{$$\frac{\text { High }}{\text { Estimate }}$ <br> $(1.15 \%)$}{ (1.15 } |
| CBD | 3.25 | \$3,022,500 | \$1,511,000 | \$2,569,000 | \$2,569,000 | \$3,476,000 |
| CBD Urbanized | 12.50 | \$11,625,000 | \$5,813,000 | \$9,881,000 | \$9,881,000 | \$13,369,000 |
| Heavy Commercial (High Rise, Large Building) | 3.25 | \$3,022,500 | \$1,511,000 | \$2,569,000 | \$2,569,000 | \$3,476,000 |
| Strip Commercial | 3.25 | \$3,022,500 | \$1,511,000 | \$2,569,000 | \$2,569,000 | \$3,476,000 |
| Fringe (Mixed, Residential/Commercial) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Industries (Factories, Warehouse) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Light Residential (1/4- Acres) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Medium Residential (Acres+) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Heavy Residential (Apartments) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Public Use (Parks, School) | 1.75 | \$1,627,500 | \$814,000 | \$1,383,000 | \$1,383,000 | \$1,872,000 |
| Rural | 1.00 | \$930,000 | \$465,000 | \$791,000 | \$791,000 | \$1,070,000 |

Note: TDOT provided estimated cost per mile for new construction. Percentages of the cost per mile taken into account for New Construction (Bypass) and widening (SR-6) Sections.

For SR-6 widening, Section 1 utilizes "CBD Urbanized" (for new construction due to compact adjacent development), Section 2 utilizes "Strip Commercial", Section 3-5 utilizes "Rural".

For the Bypass Options, "Rural" for new construction is utilized.

## Paving Cost by Cross Section

JHS

| City Street Mainline: |  |  |  |  |
| :--- | :--- | :--- | ---: | ---: | ---: | ---: | :--- |
| Item No. | Description |  |  |  |

Arterial (Asphalt) Mainline:

| Item No. | Description | Units | $\begin{gathered} 2007 \text { Unit } \\ \text { Cost } \\ \hline \end{gathered}$ | Thickness (Inches) |  | $\begin{aligned} & \text { st per } \\ & \text { S.F. } \end{aligned}$ | Based On |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 411-02.10 | Bituminous Surface | Tons | \$ 62.68 | 1.25 | \$ | 0.46 | Design Guidelines 4-411.00 |
| 403-01 | Tack Coat | Tons | \$ 339.50 | 0.00 | \$ | 0.00 | Design Guidelines 4-403.00 |
| 307-02.08 | Bituminous Binder | Tons | \$ 57.47 | 2.00 | \$ | 0.72 | Design Guidelines 4-307.00 |
| 307-02.01 | Bituminious Base | Tons | \$ 55.79 | 6.50 | \$ | 2.32 | Design Guidelines 4-307.00 |
| 402-01 | Prime Coat | Tons | \$ 382.39 | 0.00 | \$ | 0.06 | Design Guidelines 4-402.00 |
| 402-02 |  | Tons | \$ 18.19 |  | \$ | 0.01 | Design Guidelines 4-402.00 |
| 303-01 | Mineral Aggregate Base | Tons | \$ 14.18 | 10.00 | \$ | 0.89 | Design Guidleines 4-303.00 |
| Total: |  |  |  | 19.75 | \$ | 4.45 |  |

City Street Overlay (Assume Double Layer of Surface AC)

| City Street Overlay (Assume Double Layer of Surface AC) |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Ramp (Concrete) Mainline:


City Street Shoulder:

| Item No. | Description | Units | 2007 Unit Cost |  | Thickness (Inches) |  | st per F. | Based On |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 411-01.07 | Surface AC (PG64-22) GR "E" | Ton | \$ | 56.92 | 1.5 | \$ | 0.49 | Design Guidelines 4-411.00 |
| 303-01 | Mineral Agg Base GRA "D" | Ton | \$ | 14.18 | 12.75 | \$ | 1.13 | Design Guidelines 4-303.00 |
| Total: |  |  |  |  | 14.25 | \$ | 1.62 |  |

Arterial and Ramp (Asphalt) Shoulder:

| Item No. | Description | Units | $\begin{gathered} 2007 \text { Unit } \\ \text { Cost } \\ \hline \end{gathered}$ | Thickness (Inches) |  | $\begin{aligned} & \text { t per } \\ & \text { F. } \end{aligned}$ | Based On |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 411-01.07 | Bituminous Surface | Tons | \$ 56.92 | 1.25 | \$ | 0.41 | Design Guidelines 4-411.00 |
| 403-01 | Tack Coat | Tons | \$ 339.50 | 0.00 | \$ | 0.00 | Design Guidelines 4-403.00 |
| 307-01.08 | Bituminous Binder | Tons | \$ 56.40 | 2.00 | \$ | 0.71 | Design Guidelines 4-307.00 |
| 402-01 | Prime Coat | Tons | \$ 382.39 | 0.00 | \$ | 0.06 | Design Guidelines 4-402.00 |
| 402-02 |  | Tons | \$ 18.19 |  | \$ | 0.01 | Design Guidelines 4-402.00 |
| 303-01 | Mineral Aggregate Base | Tons | \$ 14.18 | 16.50 | \$ | 1.47 | Design Guidleines 4-303.00 |
| Total: |  |  |  | 19.75 | \$ | 2.65 |  |

[^0]
## Appendix

# SR-6 (US-31) TRAFFIC STUDY 

# MAURY AND WILLIAMSON COUNTIES, TENNESSEE L.M. 32.47(MAURY COUNTY) TO 10.03 (WILLIAMSON COUNTY) 

PIN \# 111040.00

PREPARED BY:


Florence \& Hutcheson, Inc.
Consulting Engineers

FOR:
$\mathrm{TD} \mathrm{T}_{6}^{\mathrm{T}}$
TENNESSEE DEPARTMENT OF TRANSPORTATION PROJECT PLANNING DIVISION

Updated 8/08

## TRAFFIC PROJECTIONS

County: Maury and Williamson
Location: $\quad$ SR-6 (US-31) from Log Mile (L.M.) 32.47 (Kedron Road) to L.M. 10.03 (Mack Hatcher Parkway)
Date of Counts: June, 2008

### 1.0 DESCRIPTION OF PROJECT AND BACKGROUND

The traffic projections provided in this Appendix are utilized in the SR-6 (US-31) Transportation Planning Report (TPR) in Maury and Williamson Counties, Tennessee. The Annual Average Daily Traffic (AADT) are projected for the years 2014 and 2034. The AADT turning volumes are included in these projections.

A map of the study area is provided in Exhibit 1.1. Vicinity Map. The results of the traffic projections are provided in the form of a line sketch following the Vicinity Map. A discussion of the traffic projection calculations is provided.

Exhibit 1.1 Vicinity Map


## SR-6 TPR AADT TRAFFIC PROJECTIONS MAURY \& WILLIAMSON COUNTIES



## HENPECK LANE

COLEMAN ROAD

| SR-6 TRAFFIC | DATA |
| :--- | :--- |
| AADT (2014): | 23,300 |
| AADT (2034): | 30,700 |
| "K": | $8 \%$ |
| D: | $70: 30$ |
| PRF: | 0.95 |
| TRUCK \%: | $3 \%$ |
| SIDE ROAD DATA |  |
| AADT (2014): |  |
| SEE DIAGRAM |  |
| AADT (2034): | SEE DIAGRAM |
| "K: | $8 \%$ |
| D: | $60: 40$ |
| PRF: | 0.95 |
| TRUCK \%: | $3 \%$ |

BEECHCROFT ROAD


GOOSE CREEK BYPASS

## LEGEND

| $---\quad$ YEAR 2016 |  |
| :--- | :--- |
| $X, X X X$ | IMPROVEMENT |
| 2014AADT |  |
| $(X, X X X)$ | 2034 AADT |

### 2.0 INFORMATION USED IN THE PROJECTIONS

## 8-HOUR TURNING MOVEMENT TRAFFIC COUNTS

The counts were collected in June of 2008 between the hours of: 6-9 am, $11 \mathrm{am}-1 \mathrm{pm}, 3 \mathrm{pm}$ to 6 pm at the following locations along SR-6:

1. L.M. 32.47 Kedron Road
2. L.M. 32.99 Beechcroft/Duplex (collected $11 / 28 / 2006$ )
3. L.M. 3.07 Thompson's Station Road
4. L.M. 6.00 Goose Creek Bypass
5. L.M. 8.71 Coleman Road
6. L.M. 8.78 Henpeck Lane
7. L.M. 10.03 Hillview Lane/Mack Hatcher Parkway

The count data collected is provided in the Calculations and Data Collection portion of this Appendix.

TDOT 24-HOUR AADT HISTORICAL DATA
Count data was attained from TDOT's Tennessee Roadway Information Management System (TRIMS) database. Historical growth rate data was also attained from TDOT's Traffic History GIS system available online at the following address:
http://ww3.tdot.state.tn.us/traffichistory/
The following count locations within the study area were utilized:

1. SR-6 Maury County Station 007
2. SR-6 Maury County Station 009
3. SR-6 Williamson County Station 67
4. SR-6 Williamson County Station 94
5. SR-6 Williamson County Station 197
6. SR-6 Williamson County Station 35
7. Kedron Road Maury County Station 170
8. Beechcroft Road Maury County Station 169
9. Duplex Road Maury County Station 010
10. Thompson Station Williamson County Stations 068 and 066
11. SR-840 Williamson County Station 201
12. Goose Creek Bypass Williamson County Station 095
13. Henpeck Lane Williamson County Station 036
14. Mack Hatcher Parkway Williamson County Station 142

The TDOT count data is provided in the Calculations and Data Collection portion of this Appendix.

## STAKEHOLDER INTERVIEWS

Interviews were conducted to determine known proposed traffic generators of significant size, zoning information, and to attain traffic data from the Metropolitan Planning Organization (MPO) Traffic Model. The following people were contacted:

Nashville Area MPO
Michael Skipper, Director (615) 862-7204
Matt Meservy (615) 862-6887
Williamson County
Mike Matteson, Director of Planning (615) 790-5725
Joe Horne, Community Development Director (615) 790-5725

## City of Spring Hill

Ferrell White, Director of Planning (931) 486-2242x212

## City of Franklin

Eric Gardner, Engineering Director (615) 791-3218
Town of Thompson's Station
Greg Langeliers, Town Administrator (615) 794-4333

## MPO TRAFFIC MODEL

The output from the MPO's Traffic Model was provided for the years 2006, 2016, and 2030. The MPO noted that the travel demand model information is regional in nature and not necessarily intended for corridors. It was recommended to utilize the model as a tool, but not to take the numbers directly as traffic projections. The model should primarily be utilized to observe trends related to roadway improvements and create factors or orders of magnitude to adjust field counts. It was also noted that the model includes future improvements as programmed in the Long Range Transportation Plan (LRTP). The MPO Traffic Model output is provided in the Calculations and Data Collection portion of this Appendix.

### 3.0 METHODOLOGY AND PROJECTIONS

## DATA COLLECTION

The following data was collected to create the traffic projections along the route. The traffic projections were created for the years 2014 and 2034. The year 2014 was determined as the "existing" year due to it being 5 years from the year the TPR is scheduled to be approved. The year 2034 was determined as the "design" year due to it being 20 years from the "existing" date.

| Base AADT | The 2006 base Annual Average Daily Traffic (AADT) was determined from <br> TRIMS, TDOT AADT Traffic Maps, and the MPO Traffic Model. Year 2006 <br> traffic was utilized due to that being a year in the MPO model (there is no <br> 2008 MPO traffic model). |
| :--- | :--- |
| Base Turning <br> Movement <br> Volumes | The base turning movement volumes were determined from field-collected <br> data along the route. |
| Growth Rate | Growth rates were attained from TDOT's website and the MPO Traffic Model. |
| Directional <br> Split | The directional split, or "D", was attained from field collected turning <br> movement counts taken along the route and from TRIMS traffic reports. A <br> "D" of 70:30 was observed. |
| Truck <br> Percentages | The truck percentages were determined from field collected turning <br> movement counts and from TDOT's TRIMS traffic report. A truck percentage <br> of 3\% was observed. |
| "K" Factor | The percentage of traffic in the peak hour compared to the AADT was <br> determined from TRIMS traffic reports and by comparing the observed peak <br> hours collected in the turning movement counts with the 2006 AADT data in <br> TDOT's TRIMS database. A "K" factor of 0.08 was observed. |
| PHF | The Peak Hour Factor (PHF) was determined from field collected turning <br> movement counts. The PHF observed was 0.95. |
| Future Traffic <br> Generators | Interviews determined that although considerable development is occurring <br> in the vicinity of the corridor, no planned generators are known that would <br> warrant separate consideration for traffic projections. In other words, <br> historical and MPO Traffic Model growth rates are estimated to account for <br> the future growth in traffic along the corridor. Williamson County noted that <br> the area along the corridor in the county's jurisdiction is zoned 1 dwelling unit <br> per acre (max density), with no commercial development. It was also noted <br> that the primary development was occurring within the cities and towns along <br> the route. These developments are primarily residential. |

## METHODOLOGY

## AADT Calculations

The AADT calculations were performed in two iterations. In the first iteration, the 2006 TDOT AADT traffic data was compared to the 2006 MPO Traffic Model AADT data. A factor comparing the TDOT to the MPO data was attained. This factor was applied to the 2016 and 2030 MPO AADT model information. The $1^{\text {st }}$ iteration 2014 and 2034 AADT was interpolated/extrapolated directly from this data, with no engineering judgment being applied.

The $1^{\text {st }}$ iteration AADT volumes were examined and compared to the MPO models, and to expected volumes utilizing historic growth rates. Engineering judgment was utilized to smooth the AADT volumes along the route and to eliminate unrealistic projections. The $2^{\text {nd }}$ iteration AADT volumes are what appear on the provided Traffic Volume Line Sketch. The procedures described here were utilized for both SR-6 and the side roads. A chart summarizing the various AADT data points utilized in the calculations is provided in Exhibit 3.1 AADT Traffic Volumes. The traffic calculations are provided in the Calculations and Data Collection portion of this Appendix.

## Exhibit 3.1 AADT Traffic Volumes

| SR-6 AADT Projections |  | $\begin{aligned} & \text { MPO } \\ & 2006 \\ & \text { AADT } \end{aligned}$ | $\begin{aligned} & \text { TDOT } \\ & 2006 \\ & \text { AADT } \end{aligned}$ | Factor |  | MPO2030 AADT | $2^{\text {nd }}$ Iter. AADT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From | To |  |  |  |  |  | 2014 | 2034 |
| Beginning | Kedron | 7,482 | 14,621 | 1.95 | 8,179 | 12,305 | 15,750 | 26,500 |
| Kedron | Duplex/Beechcroft | 13,252 | 18,007 | 1.36 | 17,289 | 28,315 | 22,400 | 36,700 |
| Duplex/Beechcroft | Thompson's Sta. | 18,195 | 18,007 | 0.99 | 27,114 | 34,843 | 25,000 | 36,700 |
| Thompson's Sta. | SR-840 | 17,602 | 21,645 | 1.23 | 30,173 | 35,346 | 34,000 | 36,700 |
| SR-840 | Goose Creek | 13,341 | 21,645 | 1.62 | 14,912 | 21,558 | 24,000 | 27,600 |
| Goose Creek | Coleman/Henpec | 13,622 | 12,646 | 0.93 | 14,816 | 18,643 | 15,000 | 18,300 |
| Coleman/Henpeck | Mack Hatcher | 12,027 | 16,682 | 1.39 | 15,715 | 21,532 | 20,800 | 32,200 |
| MackHatcher | End | 8,620 | 21,835 | 2.53 | 14,620 | 18,435 | 27,100 | 34,200 |


| Side Road | Side of SR-6 |  | $\begin{aligned} & \text { MPO } \\ & 2006 \\ & \text { AADT } \\ & \hline \end{aligned}$ |  | Factor | $\begin{array}{\|c} \hline \text { MPO } \\ 2016 \\ \text { AADT } \end{array}$ | $\begin{aligned} & \hline \text { MPO } \\ & 2030 \\ & \text { AADT } \end{aligned}$ | $2^{\text {nd }}$ Iter. AADT |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | West | East |  |  |  |  |  | 2014 | 2034 |
| Kedron Parkway | X |  |  |  |  |  |  | 3,000 | 5,000 |
| Kedron Road |  | X | 2254 | 5,239 | 2.32 | 4,613 | 8,611 | 5,950 | 10,000 |
| Beechcroft Road | X |  | 3922 | 4,353 | 1.11 | 5,979 | 8,361 | 6,200 | 10,100 |
| Duplex Road |  | X | 3992 | 6,126 | 1.53 | 4,485 | 6,552 | 6,700 | 11,000 |
| Thompson's Sta. Rd. | X |  | 6543 | 2,858 | 0.44 | 4,806 | 13,637 | 4,800 | 14,000 |
| Thompson's Sta. Rd. |  | X | 3719 | 2,571 | 0.69 | 4,910 | 5,083 | 6,000 | 14,000 |
| SR-840 | X |  | n/a | n/a | n/a | 14,055 | 20,639 | 0 | 33,800 |
| SR-840 |  | X | 6222 | 10,060 | 1.62 | 18,836 | 24,583 | 26,400 | 42,300 |
| Goose Creek Byp. |  | X | 1096 | 8,447 | 7.71 | 1,570 | 4,709 | 10,800 | 21,600 |
| Coleman Road | x |  | 6626 |  |  | 1,254 | 3,318 | 6,000 | 8,000 |
| Henpeck Lane |  | X | 1770 | 3,335 | 1.88 | 1,541 | 1,788 | 4,000 | 6,000 |
| Mack Hatcher Pkwy. | X |  | n/a | n/a | n/a | 12,355 | 16,196 | 6,000 | 25,500 |
| Mack Hatcher Pkwy. |  | X | 5994 | 19,207 | 3.20 | 13,791 | 18,485 | 23,000 | 30,000 |

## AADT Turning Movements

After the AADT mainline volumes were projected, the AADT turning movements were calculated. The turning volumes were estimated by using software that performed the calculations outlined in Chapter 8, Turning Movement Procedures, of NCHRP 255 Highway Traffic Data for Urbanized Area Project Planning and Design. These calculations utilize the field-collected counts as the basis of the turning volumes. The results of the AADT turning movement calculations are provided in the Traffic Schematics. The traffic calculations are provided in the Calculations and Data Collection portion of this Appendix.

## CALCULATIONS AND DATA COLLECTIONS

AADT Calculations
AADT Turning Movement Calculations "D" \& "K" Calculations
Field Collected Traffic Counts
MPO Traffic Model Data
TRIMS Traffic Data

A-1
A-7
A-26
A-31
A-66
A-70

## TD $\uparrow$ T

## Tennessee Department of Transportation EARLY ENVIRONMENTAL SCREENING PROCESS (EES) PROJECT SCORING

## Project Score Factors

|  | Total Impacts <br> Evaluated | Total Impacts <br> to Evaluate | EES Evaluation |
| :--- | :--- | ---: | :--- |
| Project Impact Areas: | $\mathbf{1 5}$ | $\mathbf{1 5}$ | Complete |
| Date of Evaluation: | December 30, 2008 |  |  |
| Evaluation done by: | Chris Armstrong |  |  |
|  | Transportation Planner 4 |  |  |
| County: | Maury-Williamson |  |  |
| Route: | State Route 6 (US-31) |  |  |
| PIN: | 111040.00 | From Old Kedron Road in Maury Co. to Mack Hatcher Pkwy. in Williamson Co. |  |
| Termini: |  |  |  |

Impact Ranking of Features Evaluated:
Features with No Impact Total by Rank

## Bat

Terrestrial Species
TDEC Conservation Sites \& TDEC Scenic Waterways
Superfund Sites
Caves
Pyritic Rock
Tennessee Natural Areas Program
Wildlife Management Areas
TWRA Lakes \& Other Public Lands
Features with Low Impact 0

Features with Moderate Impact
3

[^1]National Register Sites
Large Wetland Impacts

## Community Impacts Present:

## Institutions:

School
Church

## Populations:

No population present
Linguistically isolated populations
EES Project Impact:

## Community Impact Group

## INSTITUTIONS \& SENSITIVE COMMUNITY POPULATIONS

Sensitive Populations Project Impact:
Present
Not Present

## Institutions:

| Hospital | $\Gamma$ | $\nabla$ |
| :--- | :---: | :---: |
| School | $\Gamma$ | $\Gamma$ |
| Church | $\Gamma$ | $\Gamma$ |
| Public Building | $\Gamma$ | $\Gamma$ |

## Populations:

| No population present | $\Gamma$ | $\Gamma$ |
| :--- | :--- | :---: |
| 65 and older populations | $\Gamma$ | $\Gamma$ |
| Disability populations | $\Gamma$ | $\Gamma$ |
| Households without a vehicle | $\Gamma$ | $\Gamma$ |
| Minority populations $24 \%$ | $\Gamma$ | $\Gamma$ |
| Linguistically isolated populations | $\Gamma$ | $\Gamma$ |
| Populations below poverty - State average $-13 \%$ | $\Gamma$ | $\Gamma$ |
| Populations below poverty - State average $-27 \%$ | $\Gamma$ | $\checkmark$ |

## Historic Architecture/Archaeology Group

NATIONAL REGISTER SITES
Impact

| Project Impact | VnverSubstantial - A substantial impact on the project is anticipated as there is a National <br> Register historic property within the project study area or corridor. It is not possible to <br> (Environmental, Time, <br> avoid a taking or impact of the surveyed site and/or the historic property. Other concerns <br> are the visual or audible effects upon the survey site and/or historic property that need to be <br> considered and minimized. An environmental impact will likely result and necessitate <br> coordination with State Historic Preservation Office as part of NEPA. Additional project <br> calternatives may be needed to minimiza the amount of takings. Indirect effects (visual and <br> audible) may occur, and may require one of more of the following: vegetative plantings, a <br> noise wall and/or beautification of the roadway/bridges. Design efforts that may be <br> required to include coordination, negotiation and planning mitigation measures for any <br> taking. |
| :--- | :--- |

## CEMETERY SITES \& CEMETERY PROPERTIES

## Impact

| Project Impact | $\nabla$ Moderate - Medium impact on environment is anticipated as there is a cemetery within the |
| :--- | :--- |
| (Environmental, Time, | Mroject study area or corridor. It is possible to avoid impacts to the cemetery. Although the <br> prome <br> cemetery site is present in the study area or corridor, it is possible to avoid impacts to the <br> cemetery. An environmental impact may still result and necessitate an archaeological <br> Cost, Design, and <br> review as part of NEPA. A moderate level of environmental documentation and time will <br> be required to proceed with development of the project, including steps reach 'no adverse <br> effect' and/or de minimus impact determination on the impacts to the cemetery. |

## Ecology Group

## RARE AND PROTECTED SPECIES

## BAT

Impact

| Project Impact <br> (Environment, Time, <br> Cost, Design, and <br> Maintenance) | V <br> None - No project impact is anticipated. There is no occurrence of Indiana or gray bats <br> within 4 miles of the proposed project study area or corridor. |
| :--- | :--- |

## TERRESTRIAL SPECIES

## Impact

| Project Impact | $\nabla$None - No impact to the project is anticipated. There is no known occurrence of a rare, <br> state, or federally-protected terrestrial species within the proposed transportation study area <br> (Environment, Time, |
| :--- | :--- |
| Cor corridor. |  |

## AQUATIC SPECIES

## Impact

Project Impact<br>(Environment, Time, Cost, Design, and Maintenance)

[^2]
## TDEC CONSERVATION SITES \& TDEC SCENIC WATERWAYS

## Impact

| Project Impact | $\sqrt{\text { None - No project impact is expected as there are no scenic waterways or TDEC }}$(Environment, Time, <br> Conservation Sites within project study area or corridor. <br> Cosign, |
| :--- | :--- |

## LARGE WETLAND IMPACTS

## Impact

| Project Impact |
| :--- |
| (Environment, Time, |
| Cost, Design, |
| Maintenance) |

$\sqrt{ } \sqrt{ }$ Substantial - Regions 1, 2, and 3: A substantial impact to the project is probable as there is greater than 2 acres of wetlands within the project study area or corridor. Compensatory mitigation will be required. Design effort will be needed to avoid and minimize impacts to wetlands to the maximum extent practicable. If a floodplain is crossed by the project, floodplain culverts may be necessary.

## Hazardous Substances/Geology Group

## SUPERFUND SITES

## Impact

Project Impact<br>(Environment, Time, Cost, Design, and Maintenance)

None - No project impact is anticipated as there are no known contaminated land tracts abutting or within the project study area or corridor.

## CAVES

Impact

| Project Impact <br> (Environment, Time, <br> Cost, Design, and <br> Maintenance) | V |
| :--- | :--- |

## PYRITIC ROCK

## Impact

> Project Impact (Environment, Time, Cost, Design, and Maintenance)
V None - No project impact is anticipated. Pyritic rock is not known to occur in the study area/corridor or project does not involve excavation. Limestone (symbolized as dark green) and dolomite (symbolized as light green) are present.

## Railroad \& Public Lands Group

## RAILROADS

## Impact

Project Impact (Environment, Time, Cost, Design, and Maintenance)

$\checkmark$ Moderate - Medium impact on the project is anticipated as a railroad lies within the project study area or corridor. An impact on the railroad cannot be avoided through more detailed planning or the railroad will be within 200 feet of the proposed transportation project. The initial idea is that there will be an existing at-grade crossing, and coordination with the Tennessee DOT Safety Planning and Travel Data Office and the Tennessee DOT Right-OfWay Division - Utilities Section should be initiated. An impact on the project is likely due to the need to resolve major drainage issues, grade crossing mitigation, grade separations, railroad property acquisition, and railroad relocations. Coordination with the railroad on right of way issues is anticipated which may require much time, including a maintenance agreement. Additional design effort to avoid/minimize impacts may be needed and to create additional alternatives. Maintenance agreements with the railroad may need to be resolved, and any maintenance will be dependent upon these agreements. Typical maintenance includes mowing and clearing of the right of way and/or repairs of signalized at-grade intersection.

## TENNESSEE NATURAL AREAS PROGRAM

## Impact

| Project Impact <br> (Environment, Time, <br> Cost, Design, and <br> Maintenance) | V |
| :--- | :--- |
| None - No impact on the project is anticipated as the project study area or corridor does not <br> include a Natural Area. |  |

## WILDLIFE MANAGEMENT AREAS

## Impact

| Project Impact <br> (Environment, Time, <br> Cost, Design, and | $\checkmark$None - No project impact is anticipated as a WMA does not abut nor is located within the <br> project study area or corridor. |
| :--- | :--- |

## TWRA LAKES \& OTHER PUBLIC LANDS

## Impact

Project Impact<br>(Environment, Time, Cost, Design, and Maintenance)

$\checkmark$ None - No impact on the project is anticipated as there area no parks located within or abutting the project study area or corridor.

## EES Repor $\dagger$

PIN 111040.00
10,000 Foot Corridor
December 30, 2008

## Ecology

Rare \& Protected Species

Aquatic Species

Etheostoma luteovinctum
Etheostoma luteovinctum
Etheostoma luteovinctum
Etheostoma luteovinctum
Etheostoma luteovinctum
Total $=5$
USESA
SPROT
D
D
D
D
D

Hazardous Substances \& Geology
Geology
Caves

## EES Report

PIN 111040.00
4,000 Foot Corridor
December 30, 2008

## Ecology

Rare \& Protected Species

Terrestrial Species
TDEC Conservation Sites
TDEC Scenic Waterways
Large Wetland Impacts
POWHh
POWHx
PFO1A
POWHx
POWHx
POWHx
POWHh
POWHx
POWHh
POWHx
POWHh
POWHh
POWHh
POWHh
POWHh
POWHh
POWHh
POWHx
PEM1Cx
POWHh
POWHh
POWHx
POWHh
PEM1C
POWHx
POWHh
POWHh
POWHh
POWHx
POWHh

There are none.
There are none.
There are none.
Total $=134$

PIN 111040.00
4,000 Foot Corridor
December 30, 2008
POWHx
POWHh
POWHh
POWHx
POWHh
POWHh
POWH
POWHh
POWHh
POWHh
POWHx
POWHh
POWHh
POWHx
POWHh
POWHx
POWHh
POWHx
PUSCh
POWHh
POWHh
POWHh
POWHh
PEM1Ax
POWHx
POWHh
POWHh
POWHh
POWHh
POWHh
POWHh
POWHx
POWHh
PEM1F
POWHx
POWHx
POWHh
POWHx
POWHx
POWHh

PIN 111040.00
4,000 Foot Corridor December 30, 2008

PEM1C
POWHh
POWHx
POWHh
POWHx
POWHh
POWHx
POWHh
PUSCh
POWHh
PEM1C
POWHh
POWHx
POWHh
POWHh
POWHx
POWHh
POWHh
POWHh
POWHx
POWHh
POWHx
POWHx
PEM1C
POWHx
POWHh
POWHh
POWHh
POWHx
POWHh
POWHx
POWHh
POWHx
POWHh
POWHx
POWHx
POWHx
POWHh
POWHh
POWHh

PIN 111040.00
4,000 Foot Corridor
December 30, 2008
POWHx
POWHx
POWHx
POWHx
POWHh
POWHx
POWHx
POWHx
POWHx
POWHh
PEM1C
POWHx
POWHh
POWHx
POWHh
POWHh
POWHh
POWHh
POWHh
POWHh
POWFX
POWHx
POWHh
POWHx

## Railroads \& Public Lands

## Public Lands

Tennessee Natural Areas Program
Wildlife Management Areas

There are none.
There are none.

## EES Report

PIN 111040.00
2,000 Foot Corridor December 30, 2008

## Historic Architecture \& Archaeology

Historic Architecture
National Register Sites
Pointer, Henry, House
Homestead Manor
Cheairs, Martin, House
Spring Hill Presbyterian Church
Ritter-Morton House
Harrison House
Winstead Hill
Grace Episcopal Church
White Hall
St. Mark United Primitive Baptist Church
Johnson, James P., House
Buford, Spencer, House

## Hazardous Substances \& Geology

## Superfund Sites

Geology
Pyritic Rock
Carters Limestone
Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Fort Payne Formation, Chattanooga Shale Carters Limestone

Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Fort Payne Formation, Chattanooga Shale Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Carters Limestone

Bigby-Cannon Limestone, Hermitage Formation

Total $=12$

There are none.

Total $=16$
Classification
Limestone
Limestone

Limestone
Limestone
Limestone

Limestone

Limestone

Limestone

Limestone

Limestone
imestone

Limestone
Limestone

PIN 111040.00
2,000 Foot Corridor December 30, 2008

Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation Sequatchie Formation, Liepers Formation, Inman Formation, Catheys Formation

## Railroads \& Public Lands

Public Lands

TWRA Lakes
Other Public Lands

There are none.
There are none.

## EES Report

PIN 111040.00
1,000 Foot Corridor December 29, 2008

## Community Impact

## Cemetery Sites

Cemetery

Total $=3$

Total $=1$

Total= 4
School
School
Church
School

Present
Not Present
Not Present
Not Present
Not Present
Present
Not Present
Not Present

Ecology
Rare \& Protected Species Bats

## Railroads \& Public Lands

Railroads

There are none

Present


## Legend

| ${ }^{+}+$ | Cemetery | （6） | Disability |
| :---: | :---: | :---: | :---: |
|  | Cemetery Property | 娄 國 | Households without a vehicle |
| H | Hospital | $\because$ | Minority Population－24\％ |
| ： | School | － | Linguistically Isolated |
| 4 | Church | $P / 7$ | Below Poverty－13．5\％ |
| 血 | Public Building | N1\％ | Below Poverty－27\％ |
|  | No Population Present |  | Bat |
| $5>65$ | Population 65 and Over | ぃ | Railroads |



## Legend

| 0 | 0.5 | 1 | 2 Miles |
| :--- | :---: | :--- | :--- | :--- |
| $\vdash, 1,1,1,1$ |  |  |  |

Terrestrial Species
TDEC Conservation Sites
TDEC Scenic Waterways
$\square$ Large Wetland Impacts
Tennessee Natural Areas Program
Wildlife Management Areas
Rivers and Waterways
State Route 6 (US-31) From Old Kedron Road to Mack Hatcher Pkwy. Maury \& Williamson Counties

| 0 | 0.5 | 1 | 2 Miles |
| :--- | :--- | :--- | :--- |

## Legend

* National Register Sites
- Superfund SitesFormation that contains acid producing rock

|  | Formation that may contain potentially acid producing rock |
| :--- | :--- |
| Includes formations that may contain acid producing rock |  |


$\begin{array}{lllll}0 & 0.5 & 1 & 2 \text { Miles }\end{array}$
1 111111

## Legend

# HCS+ CALCULATIONS 

No Build Option
Widen Along the Existing Alignment Option
Three Lane Thompson's Station Option

## Maury and Williamson Counties

## NO BUILD OPIION

## HCS+ ANALYSIS

Segment 1 L.M. 32.47 to 33.31 Year 2014
Segment 1 L.M. 32.47 to 33.31 Year 2034
Segment 2A L.M. 0.00 to 1.20 Year 2014
Segment 2A L.M. 0.00 to 1.20 Year 2034
Segment 2B L.M. 1.30 to 2.17 Year 2014
Segment 2B L.M. 1.30 to 2.17 Year 2034
Segment 2C L.M. 2.17 to 2.49 Year 2014
Segment 2C L.M. 2.17 to 2.49 Year 2034
Segment 3 L.M. 2.49 to 4.75 Year 2014
Segment 3 L.M. 2.49 to 4.75 Year 2034
Segment 4 L.M. 4.15 to 5.55 Year 2014
Segment 4 L.M. 4.15 to 5.55 Year 2034
Segment 5A L.M. 5.55 to 6.60 Year 2014
Segment 5A L.M. 5.55 to 6.60 Year 2034
Segment 5B L.M. 6.60 to 8.67 Year 2014
Segment 5B L.M. 6.60 to 8.67 Year 2034
Segment 6 L.M. 8.67 to 10.03 Year 2014
Segment 6 L.M. 8.67 to 10.03 Year 2034

HCS+: Urban Streets Release 5.3
Jon Storey
Florence \& Hutcheson 1325 Murfreesboro Rd.
Nashville, TN 37217
Phone: 615-399-9090 Fax:
E-Mail:
PLANNING ANALYSIS

| Analyst: |  |
| :--- | :--- |
| Agency/Co.: |  |
| Date Performed: | $11 / 6 / 2008$ |
| Analysis Time Period: | 2014 |
| Urban Street: | SR-6 |
| Direction of Travel: |  |
| Jurisdiction: | Segment 1 |
| Analysis Year: | 2014 |
| Project ID: SR-6 TPR |  |

Traffic Characteristics


Signal Characteristics $\qquad$

| Signalized intersections | 3 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/c | 0.400 |  |

Results

| Annual average daily traffic, AADT | 22400 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 1791 | vph |
| Hourly directional volume | 1253 | vph |
| Through-volume 15-min. flow rate | 659 | v |
| Running time | 96.8 | sec |
| v/c ratio | 0.92 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 34.1 | sec |
| Filtering/metering factor, I | 0.282 |  |
| Incremental delay | 6.5 | sec |
| Control delay | 37.0 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 14.6 | mph |
| Total urban street LOS | D |  |

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Phone: 615-399-9090 Fax:
E-Mail:
PLANNING ANALYSIS

| Analyst: |  |
| :--- | :--- |
| Agency/Co.: |  |
| Date Performed: | $11 / 6 / 2008$ |
| Analysis Time Period: | 2014 |
| Urban Street: | SR-6 |
| Direction of Travel: |  |
| Jurisdiction: | Segment 1 |
| Analysis Year: | 2014 |
| Project ID: SR-6 TPR |  |

Traffic Characteristics

| Annual average daily traffic, AADT | 367 |  | vpd |
| :---: | :---: | :---: | :---: |
| Planning analysis hour factor, K 0.080 |  |  |  |
| Directional distribution factor, D 0.700 |  |  |  |
| Peak-hour factor, PHF 0.950 |  |  |  |
| Adjusted saturation flow rate | 180 |  | pcphgpl |
| Percent turns from exclusive lanes 50 \% |  |  |  |
| Roadway Characteristics |  |  |  |
| Number of through lanes one direction, N 1 |  |  |  |
| Free flow speed, FFS |  | 35 | mph |
| Urban class |  | 3 |  |
| Section length |  | 0.84 | miles |
| Median |  | Yes |  |
| Left-turn bays |  | Yes |  |

Signal Characteristics

| Signalized intersections | 3 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 1081 | v |
| Running time | 96.8 | sec |
| v/c ratio | 1.50 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 36.0 | sec |
| Filtering/metering factor, I | 0.090 |  |
| Incremental delay | 226.3 | sec |
| Control delay | 258.5 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 3.5 | mph |
| Total urban street LOS | F |  |

HCS+: Urban Streets Release 5.3
Jon Storey
Florence \& Hutcheson
1321 Murfreesboro Rd.
Nashville, TN 37217
Phone: 615-399-9090
Fax:
E-Mail:
PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2A
Analysis Year: 2014
Project ID: SR-6 TPR LM 0.00 to 1.30
Traffic Characteristics


Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |
|  |  |  |
|  | Results |  |
| Annual average daily traffic, AADT | 25000 | vpd |
| Two-way hourly volume | 1999 | vph |
| Hourly directional volume | 1399 | vph |
| Through-volume 15-min. flow rate | 736 | v |
| Running time | 199.2 | sec |
| v/c ratio | 1.08 |  |
| Through capacity | 683 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 36.0 | sec |
| Filtering/metering factor, I | 0.090 |  |
| Incremental delay | 38.0 | sec |
| Control delay | 70.2 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 26.4 | mph |
| Total urban street LOS | C |  |

HCS+: Urban Streets Release 5.3
Jon Storey
Florence \& Hutcheson
1321 Murfreesboro Rd.
Nashville, TN 37217
Phone: 615-399-9090
Fax:
E-Mail:
PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2A
Analysis Year: 2034
Project ID: SR-6 TPR LM 0.00 to 1.30
Traffic Characteristics

| Annual average daily traffic, AADT 36700 |  | vpd |
| :---: | :---: | :---: |
| Planning analysis hour factor, K 0.080 |  |  |
| Directional distribution factor, D 0.700 |  |  |
| Peak-hour factor, PHF 0.950 |  |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes 50 \% |  |  |
| Roadway Characteristics |  |  |
| Number of through lanes one direction, N 1 |  |  |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | No |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | sec |  |
| Effective green ratio, g/C | 0.400 |  |
|  | Results_- |  |
|  |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 1081 | v |
| Running time | 199.2 | sec |
| v/c ratio | 1.58 |  |
| Through capacity | 683 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 36.0 | sec |
| Filtering/metering factor, I | 0.090 |  |
| Incremental delay | 262.9 | sec |
| Control delay | 295.1 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 11.4 | mph |
| Total urban street LOS | F |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2B
Analysis Year: 2014
Project ID: SR-6 TPR LM 1.30 to 2.17
Traffic Characteristics

| Annual average daily traffic, AADT | 25000 | vpd |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, $D$ | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
| Roadway Characteristics |  |  |


| Number of through lanes one direction, $N$ | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, c | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results
Annual average daily traffic, AADT 25000 vpd
Two-way hourly volume 1999 vph
Hourly directional volume 1399 vph
Through-volume 15-min. flow rate 368 v
Running time
v/c ratio
199.2 sec

Through capacity
Progression factor, PF
Uniform delay 27.2
Filtering/metering factor, I
Incremental delay
Control delay
Total travel speed, Sa
Total urban street LOS
0.51

720 vph
0.895
27.2 sec
0.849
2.2 sec
26.5 sec/v
35.5 mph

A

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2034
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2B
Analysis Year: 2034
Project ID: SR-6 TPR LM 1.30 to 2.17
Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, $D$ | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
| Roadway Characteristics |  |  |


| Number of through lanes one direction, $N$ | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections Arrival type AT | 2 | sec |
| :---: | :---: | :---: |
|  | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 |  |
| Effective green ratio, g/C | 0.400 |  |
| Results |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 540 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.75 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 30.9 | sec |
| Filtering/metering factor, I | 0.579 |  |
| Incremental delay | 4.2 | sec |
| Control delay | 31.8 | sec/v |
| Total travel speed, Sa | 34.1 | mph |
| Total urban street LOS | B |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2014
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 25000 | vpd |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
|  | Roadway Characteristics_ |  |


| Number of through lanes one direction, N | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | No |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | sec |  |
| Effective green ratio, g/C | 0.400 |  |
|  | Results_- |  |
|  |  |  |
| Annual average daily traffic, AADT | 25000 | vpd |
| Two-way hourly volume | 1999 | vph |
| Hourly directional volume | 1399 | vph |
| Through-volume 15-min. flow rate | 368 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.54 |  |
| Through capacity | 683 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 27.5 | sec |
| Filtering/metering factor, I | 0.827 |  |
| Incremental delay | 2.5 | sec |
| Control delay | 27.1 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 35.4 | mph |
| Total urban street LOS | A |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2034
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2034
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
|  | Roadway Characteristics_ |  |


| Number of through lanes one direction, $N$ | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | No |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | sec |  |
| Effective green ratio, g/C | 0.400 |  |
|  | Results |  |
|  |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 540 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.79 |  |
| Through capacity | 683 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 31.6 | sec |
| Filtering/metering factor, I | 0.515 |  |
| Incremental delay | 4.9 | sec |
| Control delay | 33.1 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 33.8 | mph |
| Total urban street LoS | B |  |

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2014
Highway
From/To LM 2.49 to LM 4.75
Jurisdiction TDOT
Analysis Year 2014
Description SR-6 TPR
Input Data

| Highway class | Class | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Shoulder width | 5.0 | ft | Peak-hour factor, PHF | 0.95 |  |
| Lane width | 12.0 | ft | \% Trucks and buses | 3 | $\%$ |
| Segment length | 2.3 | mi | \% Recreational vehicles | 0 | $\%$ |
| Terrain type | Rolling |  | \% No-passing zones | 100 | $\%$ |
| Grade: Length |  | mi | Access points/mi | 16 | $/ \mathrm{mi}$ |
|  |  |  |  |  |  |


| Two-way hourly volume, V | 2720 |  | veh/h |
| :--- | :--- | :--- | :--- | :--- |
| Directional split | 70 | $/ \quad 30$ | $\%$ |

Average Travel Speed

| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 2935 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 2055 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: |  |  |
| Base free-flow speed, BFFS | 50.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 4.0 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS | 44.7 | $\mathrm{mi} / \mathrm{h}$ |
| Adjustment for no-passing zones, fnp |  | 0.8 |
| Average travel speed, ATS | $\mathrm{mi} / \mathrm{h}$ |  |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 2863 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 2004 |  |
| Base percent time-spent-following, BPTSF | 91.9 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 4.2 |  |
| Percent time-spent-following, PTSF | 96.1 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | F |  |
| :--- | :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.92 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 1618 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 6147 | veh-mi |
| Peak 15-min total travel time, TT15 | 76.7 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2034
Highway
From/To LM
Jurisdiction TDOT
Analysis Year 2034
Description SR-6 TPR
Input Data

| Highway class | Class | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Shoulder width | 5.0 | ft | Peak-hour factor, PHF | 0.95 |  |
| Lane width | 12.0 | ft | \% Trucks and buses | 3 | $\%$ |
| Segment length | 2.3 | mi | \% Recreational vehicles | 0 | $\%$ |
| Terrain type | Rolling |  | \% No-passing zones | 100 | $\%$ |
| Grade: Length |  | mi | Access points/mi | 16 | $/ \mathrm{mi}$ |
|  |  |  |  |  |  |


| Two-way hourly volume, V | 2936 |  | veh/h |
| :--- | :--- | :--- | :--- | :--- |
| Directional split | 70 | $/ \quad 30$ | $\%$ |

$\qquad$

| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 3169 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 2218 | $\mathrm{pc} / \mathrm{h}$ |
|  |  |  |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: | 50.0 | $\mathrm{mi} / \mathrm{h}$ |
| Base free-flow speed, BFFS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 4.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA |  | 44.7 |
| Free-flow speed, FFS | $\mathrm{mi} / \mathrm{h}$ |  |
|  |  | 0.7 |
| Adjustment for no-passing zones, fnp | $\mathrm{mi} / \mathrm{h}$ |  |
| Average travel speed, ATS |  | 19.4 |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 3091 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 2164 |  |
| Base percent time-spent-following, BPTSF | 93.4 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 4.2 |  |
| Percent time-spent-following, PTSF | 97.6 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | F |  |
| :--- | :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.99 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 1746 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 6635 | veh-mi |
| Peak 15-min total travel time, TT15 | 90.0 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 4 LM 4.75 to LM 5.55
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

Direction
Lane width Direction
Lateral clearance:
Right edge
Left edge
Total lateral clearance

Access points per mile
Median type
Free-flow speed:
FFS or BFFS
Lane width adjustment, FLW
Lateral clearance adjustment, FLC
Median type adjustment, FM
Access points adjustment, FA
Free-flow speed

|  |  | 2 |  |
| :--- | :--- | :--- | :--- |
| 12.0 | ft | 12.0 | ft |
|  |  |  |  |
| 6.0 | ft | 6.0 | ft |
| 6.0 | ft | 6.0 | ft |
| 12.0 | ft | 12.0 | ft |
| 16 |  | 0 |  |
| Divided |  |  |  |
| Base |  | Measured |  |
| 50.0 | mph | 60.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 4.0 | mph | 0.0 | mph |
| 46.0 | mph | 60.0 | mph |

$\qquad$ VOLUME

| Direction | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Volume, V | 1904 | vph | 816 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 501 |  | 215 |  |
| Trucks and buses | 3 | $\%$ | 3 | $\%$ |
| Recreational vehicles | 0 | $\%$ | 0 | $\%$ |
| Terrain type | Rolling |  | Rolling |  |
| Grade | 0.00 | $\%$ | 0.00 | $\%$ |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1047 | pcphpl | 448 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :--- | :--- | :--- | :--- |
| Flow rate, vp | 1047 | pcphpl | 448 | pcphpl |
| Free-flow speed, FFS | 46.0 | mph | 60.0 | mph |
| Avg. passenger-car travel speed, S | 46.0 | mph | 60.0 | mph |
| Level of service, LOS | C |  | A |  |
| Density, D | 22.8 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 7.5 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 4 LM 4.75 to LM 5.55
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED
Direction
Lane width Direction
Lateral clearance:
Right edge
Left edge
Total lateral clearance

Access points per mile
Median type
Free-flow speed:
FFS or BFFS
Lane width adjustment, FLW
Lateral clearance adjustment, FLC
Median type adjustment, FM
Access points adjustment, FA
Free-flow speed

|  |  | 2 |  |
| :--- | :--- | :--- | :--- |
| 12.0 | ft | 12.0 | ft |
|  |  |  |  |
| 6.0 | ft | 6.0 | ft |
| 6.0 | ft | 6.0 | ft |
| 12.0 | ft | 12.0 | ft |
| 16 |  | 0 |  |
| Divided |  |  |  |
| Base |  | Measured |  |
| 50.0 | mph | 60.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 4.0 | mph | 0.0 | mph |
| 46.0 | mph | 60.0 | mph |

$\qquad$ VOLUME

| Direction | 1 |  | 2 |  |
| :--- | :--- | :--- | :--- | :--- |
| Volume, V | 2055 | vph | 881 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 541 |  | 232 |  |
| Trucks and buses | 3 | $\%$ | 3 | $\%$ |
| Recreational vehicles | 0 | $\%$ | 0 | $\%$ |
| Terrain type | Rolling |  | Rolling |  |
| Grade | 0.00 | $\%$ | 0.00 | $\%$ |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1130 | pcphpl | 484 | pcphpl |


| Direction |  | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Flow rate, vp |  | 1130 | pcphpl | 484 | pcphpl |
| Free-flow speed, FFS |  | 46.0 | mph | 60.0 | mph |
| Avg. passenger-car travel speed, | S | 46.0 | mph | 60.0 | mph |
| Level of service, LOS |  | C |  | A |  |
| Density, D |  | 24.6 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 8.1 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2014
Highway
From/To
SR-6
LM 5.55 to LM 6.60
Jurisdiction TDOT
Analysis Year 2014
Description SR-6 TPR
Input Data

| Highway class | Class | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Shoulder width | 5.0 | ft | Peak-hour factor, PHF | 0.95 |  |
| Lane width | 12.0 | ft | \% Trucks and buses | 3 | $\%$ |
| Segment length | 1.0 | mi | \% Recreational vehicles | 0 | $\%$ |
| Terrain type | Rolling |  | \% No-passing zones | 100 | $\%$ |
| Grade: Length |  | mi | Access points/mi | 8 | $/ \mathrm{mi}$ |
|  |  |  |  |  |  |


| Two-way hourly volume, V | 1920 | veh/h |  |
| :--- | :--- | :--- | :--- | :--- |
| Directional split | 70 | $/ \quad 30$ | $\%$ |

Average Travel Speed

| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 2072 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1450 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: |  |  |
| Base free-flow speed, BFFS | 50.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 2.0 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS | 46.7 | $\mathrm{mi} / \mathrm{h}$ |
|  |  |  |
| Adjustment for no-passing zones, fnp | 1.1 | $\mathrm{mi} / \mathrm{h}$ |
| Average travel speed, ATS |  |  |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 2021 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1415 |  |
| Base percent time-spent-following, BPTSF | 83.1 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 4.2 |  |
| Percent time-spent-following, PTSF | 87.3 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | E |  |
| :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.65 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 531 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 2016 | veh-mi |
| Peak 15-min total travel time, TT15 | 18.0 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2034
Highway
From/To
SR-6
LM 5.55 to LM 6.60
Jurisdiction TDOT
Analysis Year 2034
Description SR-6 TPR
Input Data

| Highway class | Class | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Shoulder width | 5.0 | ft | Peak-hour factor, PHF | 0.95 |  |
| Lane width | 12.0 | ft | \% Trucks and buses | 3 | $\%$ |
| Segment length | 1.0 | mi | \% Recreational vehicles | 0 | $\%$ |
| Terrain type | Rolling |  | \% No-passing zones | 100 | $\%$ |
| Grade: Length |  | mi | Access points/mi | 8 | $/ \mathrm{mi}$ |
|  |  |  |  |  |  |


| Two-way hourly volume, V | 2208 |  | veh/h |
| :--- | :--- | :--- | :--- | :--- |
| Directional split | 70 | $/ \quad 30$ | $\%$ |

Average Travel Speed

| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 2383 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1668 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: |  |  |
| Base free-flow speed, BFFS | 50.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 2.0 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS | 46.7 | $\mathrm{mi} / \mathrm{h}$ |
| Adjustment for no-passing zones, fnp |  | 1.1 |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 2324 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1627 |  |
| Base percent time-spent-following, BPTSF | 87.0 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 4.2 |  |
| Percent time-spent-following, PTSF | 91.2 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | $E$ |  |
| :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.74 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 610 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 2318 | veh-mi |
| Peak 15-min total travel time, TT15 | 22.5 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2014
Highway
From/To
SR-6
LM 6.60 to 8.67
Jurisdiction TDOT
Analysis Year 2014
Description SR-6 TPR
Input Data

| Input Data |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Highway class Class 1 |  |  |  |  |
| Shoulder width 5.0 | $f t$ | Peak-hour factor, PHF | 0.95 |  |
| Lane width 12.0 | ft | \% Trucks and buses | 3 | \% |
| Segment length 1.0 | mi | \% Recreational vehicles | 0 | \% |
| Terrain type Rolling |  | \% No-passing zones | 100 | \% |
| Grade: Length Up/down | $\begin{aligned} & \mathrm{mi} \\ & \% \end{aligned}$ | Access points/mi | 8 | /mi |
| Two-way hourly volume, V | 1200 | veh/h |  |  |
| Directional split 70 | / 30 | \% |  |  |


| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 1295 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 907 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: |  |  |
| Base free-flow speed, BFFS | 60.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 2.0 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS | 56.7 | $\mathrm{mi} / \mathrm{h}$ |
| Adjustment for no-passing zones, fnp |  | 1.9 |
| Average travel speed, ATS | $\mathrm{mi} / \mathrm{h}$ |  |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 1263 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 884 |  |
| Base percent time-spent-following, BPTSF | 67.0 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 9.7 |  |
| Percent time-spent-following, PTSF | 76.8 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | D |  |
| :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.40 |  |
| Peak 15 -min vehicle-miles of travel, VMT15 | 332 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 1260 | veh-mi |
| Peak 15-min total travel time, TT15 | 7.4 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2034
Highway
From/To
SR-6
LM 6.60 to 8.67
Jurisdiction TDOT
Analysis Year 2034
Description SR-6 TPR
Input Data

| Input Data |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Highway class Class 1 |  |  |  |  |
| Shoulder width 5.0 | $f t$ | Peak-hour factor, PHF | 0.95 |  |
| Lane width 12.0 | ft | \% Trucks and buses | 3 | \% |
| Segment length 1.0 | mi | \% Recreational vehicles | 0 | \% |
| Terrain type Rolling |  | \% No-passing zones | 100 | \% |
| Grade: Length Up/down | $\begin{aligned} & \mathrm{mi} \\ & \% \end{aligned}$ | Access points/mi | 8 | /mi |
| Two-way hourly volume, V | 1464 | veh/h |  |  |
| Directional split 70 | / 30 | \% |  |  |


| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 1580 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1106 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: |  |  |
| Base free-flow speed, BFFS | 60.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 2.0 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS | 56.7 | $\mathrm{mi} / \mathrm{h}$ |
| Adjustment for no-passing zones, fnp |  | 1.5 |
| Average travel speed, ATS | $\mathrm{mi} / \mathrm{h}$ |  |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 1541 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1079 |  |
| Base percent time-spent-following, BPTSF | 74.2 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 7.3 |  |
| Percent time-spent-following, PTSF | 81.5 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | E |  |
| :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.49 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 405 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 1537 | veh-mi |
| Peak 15-min total travel time, TT15 | 9.4 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2014
Highway
From/To
SR-6
LM 6.60 to 8.67
Jurisdiction TDOT
Analysis Year 2014
Description SR-6 TPR
Input Data


| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 1796 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1257 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: | 45.0 | $\mathrm{mi} / \mathrm{h}$ |
| Base free-flow speed, BFFS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 4.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 39.7 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS |  | 1.3 |
| Adjustment for no-passing zones, fnp | $\mathrm{mi} / \mathrm{h}$ |  |

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 1752 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1226 |  |
| Base percent time-spent-following, BPTSF | 78.6 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 5.9 |  |
| Percent time-spent-following, PTSF | 84.5 | $\%$ | Level of Service and Other Performance Measures


| Level of service, LOS | $E$ |  |
| :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.56 |  |
| Peak 15 -min vehicle-miles of travel, VMT15 | 460 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 1747 | veh-mi |
| Peak 15-min total travel time, TT15 | 18.8 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

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Two-Way Two-Lane Highway Segment Analysis $\qquad$
Analyst
Agency/Co.
Date Performed 11/6/2008
Analysis Time Period 2034
Highway
From/To
SR-6
LM 6.60 to 8.67
Jurisdiction TDOT
Analysis Year 2034
Description SR-6 TPR
Input Data

| Input Data |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Highway class Class 1 |  |  |  |  |
| Shoulder width 5.0 | $f t$ | Peak-hour factor, PHF | 0.95 |  |
| Lane width 12.0 | ft | \% Trucks and buses | 3 | \% |
| Segment length 1.0 | mi | \% Recreational vehicles | 0 | \% |
| Terrain type Rolling |  | \% No-passing zones | 100 | \% |
| Grade: Length Up/down | $\begin{aligned} & \mathrm{mi} \\ & \% \end{aligned}$ | Access points/mi | 16 | /mi |
| Two-way hourly volume, V | 2576 | veh/h |  |  |
| Directional split 70 | / 30 | \% |  |  |


| Grade adjustment factor, fG | 0.99 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.5 |  |
| PCE for RVs, ER | 1.1 |  |
| Heavy-vehicle adjustment factor, | 0.985 |  |
| Two-way flow rate, (note-1) vp | 2780 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1946 | $\mathrm{pc} / \mathrm{h}$ |
| Free-Flow Speed from Field Measurement: |  |  |
| Field measured speed, SFM | - | $\mathrm{mi} / \mathrm{h}$ |
| Observed volume, Vf | - | $\mathrm{veh} / \mathrm{h}$ |
| Estimated Free-Flow Speed: | 45.0 | $\mathrm{mi} / \mathrm{h}$ |
| Base free-flow speed, BFFS | 1.3 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for lane and shoulder width, fLS | 4.0 | $\mathrm{mi} / \mathrm{h}$ |
| Adj. for access points, fA | 39.7 | $\mathrm{mi} / \mathrm{h}$ |
| Free-flow speed, FFS |  | 0.9 | $\mathrm{mi/h}$

Percent Time-Spent-Following

| Grade adjustment factor, fG | 1.00 |  |
| :--- | :--- | :--- |
| PCE for trucks, ET | 1.0 |  |
| PCE for RVs, ER | 1.0 |  |
| Heavy-vehicle adjustment factor, fHV | 1.000 |  |
| Two-way flow rate, (note-1) vp | 2712 | $\mathrm{pc} / \mathrm{h}$ |
| Highest directional split proportion (note-2) | 1898 |  |
| Base percent time-spent-following, BPTSF | 90.8 | $\%$ |
| Adj.for directional distribution and no-passing zones, fd/np | 4.2 |  |
| Percent time-spent-following, PTSF | 95.0 | $\%$ |

Level of Service and Other Performance Measures

| Level of service, LOS | F |  |
| :--- | :--- | :--- | :--- |
| Volume to capacity ratio, v/c | 0.87 |  |
| Peak 15-min vehicle-miles of travel, VMT15 | 712 | veh-mi |
| Peak-hour vehicle-miles of travel, VMT60 | 2705 | veh-mi |
| Peak 15-min total travel time, TT15 | 41.4 | veh-h |

Notes:

1. If vp >= $3200 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.
2. If highest directional split vp >= $1700 \mathrm{pc} / \mathrm{h}$, terminate analysis-the LOS is F.

## Maury and Williamson Counties

WDEN ALONG THE EXISTING ALGNMENTOPION

## HCS+ ANALYSIS

Segment 1 L.M. 32.47 to 33.31 Year 2014
Segment 1 L.M. 32.47 to 33.31 Year 2034
Segment 2A L.M. 0.00 to 1.20 Year 2014
Segment 2A L.M. 0.00 to 1.20 Year 2034
Segment 2B L.M. 1.30 to 2.17 Year 2014
Segment 2B L.M. 1.30 to 2.17 Year 2034
Segment 2C L.M. 2.17 to 2.49 Year 2014
Segment 2C L.M. 2.17 to 2.49 Year 2034
Segment 3 L.M. 2.49 to 4.75 Year 2014
Segment 3 L.M. 2.49 to 4.75 Year 2034
Segment 4 L.M. 4.15 to 5.55 Year 2014
Segment 4 L.M. 4.15 to 5.55 Year 2034
Segment 5A L.M. 5.55 to 6.60 Year 2014
Segment 5A L.M. 5.55 to 6.60 Year 2034
Segment 5B L.M. 6.60 to 8.67 Year 2014
Segment 5B L.M. 6.60 to 8.67 Year 2034
Segment 6 L.M. 8.67 to 10.03 Year 2014
Segment 6 L.M. 8.67 to 10.03 Year 2034

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PLANNING ANALYSIS

| Analyst: |  |
| :--- | :--- |
| Agency/Co.: |  |
| Date Performed: | $11 / 6 / 2008$ |
| Analysis Time Period: | 2014 |
| Urban Street: | SR-6 |
| Direction of Travel: |  |
| Jurisdiction: | Segment 1 |
| Analysis Year: | 2014 |
| Project ID: SR-6 TPR |  |

Traffic Characteristics

| Annual average daily traffic, AADT | 22400 | vpd |  |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |  |
| Directional distribution factor, D | 0.700 |  |  |
| Peak-hour factor, PHF | 0.950 |  |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |  |
| Percent turns from exclusive lanes | 50 |  |  |
|  | Roadway Characteristics_- |  |  |
|  |  |  |  |
| Number of through lanes one direction, N | 2 |  |  |
| Free flow speed, FFS | 35 | mph |  |
| Urban class | 3 |  |  |
| Section length | 0.84 | miles |  |
| Median | Yes |  |  |
| Left-turn bays | Yes |  |  |

Signal Characteristics $\qquad$

| Signalized intersections | 3 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results

| Annual average daily traffic, AADT | 22400 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 1791 | vph |
| Hourly directional volume | 1253 | vph |
| Through-volume 15-min. flow rate | 659 | v |
| Running time | 96.8 | sec |
| v/c ratio | 0.46 |  |
| Through capacity | 1440 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 26.4 | sec |
| Filtering/metering factor, I | 0.888 |  |
| Incremental delay | 0.9 | sec |
| Control delay | 24.6 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 17.7 | mph |
| Total urban street LOS | D |  |

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PLANNING ANALYSIS

| Analyst: |  |
| :--- | :--- |
| Agency/Co.: |  |
| Date Performed: | $11 / 6 / 2008$ |
| Analysis Time Period: | 2034 |
| Urban Street: | SR-6 |
| Direction of Travel: |  |
| Jurisdiction: | Segment 1 |
| Analysis Year: | 2034 |
| Project ID: SR-6 TPR |  |

Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |  |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |  |
| Directional distribution factor, D | 0.700 |  |  |
| Peak-hour factor, PHF | 0.950 |  |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |  |
| Percent turns from exclusive lanes | 50 | $\%$ |  |
|  | Roadway Characteristics |  |  |


| Number of through lanes one direction, N | 2 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 35 | mph |
| Urban class | 3 |  |
| Section length | 0.84 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 3 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, c | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 1081 | v |
| Running time | 96.8 | sec |
| v/c ratio | 0.75 |  |
| Through capacity | 1440 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 30.9 | sec |
| Filtering/metering factor, I | 0.578 |  |
| Incremental delay | 2.1 | sec |
| Control delay | 29.8 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 16.3 | mph |
| Total urban street LOS | D |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2A
Analysis Year: 2014
Project ID: SR-6 TPR LM 0.00 to 1.30
Traffic Characteristics


Signal Characteristics $\qquad$

| Signalized intersections | 2 | sec |
| :---: | :---: | :---: |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actua |  |
| Cycle length, C | 120.0 |  |
| Effective green ratio, g/C | 0.400 |  |
| Results |  |  |
| Annual average daily traffic, AADT | 25000 | vpd |
| Two-way hourly volume | 1999 | vph |
| Hourly directional volume | 1399 | vph |
| Through-volume 15-min. flow rate | 736 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.54 |  |
| Through capacity | 1367 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 27.5 | sec |
| Filtering/metering factor, I | 0.827 |  |
| Incremental delay | 1.3 | sec |
| Control delay | 25.9 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 35.7 | mph |
| Total urban street LOS | A |  |

HCS+: Urban Streets Release 5.3
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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2A
Analysis Year: 2034
Project ID: SR-6 TPR LM 0.00 to 1.30
Traffic Characteristics


Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | sec |  |
| Effective green ratio, g/C | 0.400 |  |
|  | Results_- |  |
|  |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 1081 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.79 |  |
| Through capacity | 1367 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 31.6 | sec |
| Filtering/metering factor, I | 0.515 |  |
| Incremental delay | 2.5 | sec |
| Control delay | 30.8 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 34.4 | mph |
| Total urban street LOS | B |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2B
Analysis Year: 2014
Project ID: SR-6 TPR LM 1.30 to 2.17
Traffic Characteristics


| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results
Annual average daily traffic, AADT 25000 vpd
Two-way hourly volume 1999 vph
Hourly directional volume 1399 vph
Through-volume 15-min. flow rate 368 v
Running time
v/c ratio
199.2 sec

Through capacity 1440 vph
Progression factor, PF
Uniform delay
Filtering/metering factor, I
Incremental delay
Control delay
Total travel speed, Sa
Total urban street LOS
0.895
24.1 sec
$\begin{array}{ll}0.976 \\ 0.4 & \text { sec }\end{array}$
$\begin{array}{ll}0.4 & \mathrm{sec} \\ 21.9 & \mathrm{sec} / \mathrm{v}\end{array}$
36.9 mph

A

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2034
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2B
Analysis Year: 2034
Project ID: SR-6 TPR LM 1.30 to 2.17
Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
| Roadway Characteristics |  |  |


| Number of through lanes one direction, $N$ | 2 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections Arrival type AT | 2 | sec |
| :---: | :---: | :---: |
|  | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 |  |
| Effective green ratio, g/C | 0.400 |  |
| Results |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume $15-\mathrm{min}$. flow rate | 540 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.38 |  |
| Through capacity | 1440 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 25.4 | sec |
| Filtering/metering factor, I | 0.934 |  |
| Incremental delay | 0.7 | sec |
| Control delay | 23.4 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 36.4 | mph |
| Total urban street LOS | A |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2014
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 25000 | vpd |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
|  | Roadway Characteristics_ |  |


| Number of through lanes one direction, $N$ | 2 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | No |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | sec |  |
| Effective green ratio, g/C | 0.400 |  |
|  | Results_- |  |
|  |  |  |
| Annual average daily traffic, AADT | 25000 | vpd |
| Two-way hourly volume | 1999 | vph |
| Hourly directional volume | 1399 | vph |
| Through-volume 15-min. flow rate | 368 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.27 |  |
| Through capacity | 1367 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 24.2 | sec |
| Filtering/metering factor, I | 0.973 |  |
| Incremental delay | 0.5 | sec |
| Control delay | 22.1 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 36.8 | mph |
| Total urban street LOS | A |  |

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PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2034
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2034
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
|  | Roadway Characteristics_ |  |


| Number of through lanes one direction, $N$ | 2 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | No |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections Arrival type AT | 2 | sec |
| :---: | :---: | :---: |
|  | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 |  |
| Effective green ratio, g/C | 0.400 |  |
| Results |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume $15-\mathrm{min}$. flow rate | 540 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.40 |  |
| Through capacity | 1367 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 25.7 | sec |
| Filtering/metering factor, I | 0.924 |  |
| Incremental delay | 0.8 | sec |
| Control delay | 23.7 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 36.3 | mph |
| Total urban street LOS | A |  |

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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 3 LM 2.49 to LM 4.75
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | $f t$ | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 50.6 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 45.0 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1904 | vph | 816 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 501 |  | 215 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | $\bigcirc$ | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1047 | pcphpl | 448 | pcphpl |


| Direction |  | 1 | 2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Flow rate, vp |  | 1047 | pcphpl | 448 | pcphpl |
| Free-flow speed, FFS |  | 45.0 | mph | 50.0 | mph |
| Avg. passenger-car travel speed, | S | 45.0 | mph | 50.0 | mph |
| Level of service, LOS |  | C |  | A |  |
| Density, D |  | 23.3 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 9.0 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 3 LM 2.49 to LM 4.75
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | $f t$ | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 50.6 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 45.0 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 2055 | vph | 881 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 541 |  | 232 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | $\bigcirc$ | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1130 | pcphpl | 484 | pcphpl |


| Direction |  | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Flow rate, vp |  | 1130 | pcphpl | 484 | pcphpl |
| Free-flow speed, FFS |  | 45.0 | mph | 50.0 | mph |
| Avg. passenger-car travel speed, | S | 45.0 | mph | 50.0 | mph |
| Level of service, LOS |  | C |  | A |  |
| Density, D |  | 25.1 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 9.7 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 4 LM 4.75 to LM 5.55
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

Direction
Lane width Direction
Lateral clearance:
Right edge
Left edge
Total lateral clearance

Access points per mile
Median type
Free-flow speed:
FFS or BFFS
Lane width adjustment, FLW
Lateral clearance adjustment, FLC
Median type adjustment, FM
Access points adjustment, FA
Free-flow speed

|  |  | 2 |  |
| :--- | :--- | :--- | :--- |
| 12.0 | ft | 12.0 | ft |
|  |  |  |  |
| 6.0 | ft | 6.0 | ft |
| 6.0 | ft | 6.0 | ft |
| 12.0 | ft | 12.0 | ft |
| 16 |  | 0 |  |
| Divided |  |  |  |
| Base |  | Measured |  |
| 50.0 | mph | 60.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 4.0 | mph | 0.0 | mph |
| 46.0 | mph | 60.0 | mph |

$\qquad$ VOLUME

| Direction | 1 |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| Volume, V | 1904 | vph | 816 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 501 |  | 215 |  |
| Trucks and buses | 3 | $\%$ | 3 | $\%$ |
| Recreational vehicles | 0 | $\%$ | 0 | $\%$ |
| Terrain type | Rolling |  | Rolling |  |
| Grade | 0.00 | $\%$ | 0.00 | $\%$ |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1047 | pcphpl | 448 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :--- | :--- | :--- | :--- |
| Flow rate, vp | 1047 | pcphpl | 448 | pcphpl |
| Free-flow speed, FFS | 46.0 | mph | 60.0 | mph |
| Avg. passenger-car travel speed, S | 46.0 | mph | 60.0 | mph |
| Level of service, LOS | C |  | A |  |
| Density, D | 22.8 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 7.5 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

HCS+: Multilane Highways Release 5.3

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E-mail:
```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 4 LM 4.75 to LM 5.55
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED
Direction
Lane width Direction
Lateral clearance:
Right edge
Left edge
Total lateral clearance

Access points per mile
Median type
Free-flow speed:
FFS or BFFS
Lane width adjustment, FLW
Lateral clearance adjustment, FLC
Median type adjustment, FM
Access points adjustment, FA
Free-flow speed

|  |  | 2 |  |
| :--- | :--- | :--- | :--- |
| 12.0 | ft | 12.0 | ft |
|  |  |  |  |
| 6.0 | ft | 6.0 | ft |
| 6.0 | ft | 6.0 | ft |
| 12.0 | ft | 12.0 | ft |
| 16 |  | 0 |  |
| Divided |  |  |  |
| Base |  | Measured |  |
| 50.0 | mph | 60.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 0.0 | mph | 0.0 | mph |
| 4.0 | mph | 0.0 | mph |
| 46.0 | mph | 60.0 | mph |

$\qquad$ VOLUME

| Direction | 1 |  | 2 |  |
| :--- | :--- | :--- | :--- | :--- |
| Volume, V | 2055 | vph | 881 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 541 |  | 232 |  |
| Trucks and buses | 3 | $\%$ | 3 | $\%$ |
| Recreational vehicles | 0 | $\%$ | 0 | $\%$ |
| Terrain type | Rolling |  | Rolling |  |
| Grade | 0.00 | $\%$ | 0.00 | $\%$ |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 1130 | pcphpl | 484 | pcphpl |


| Direction |  | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Flow rate, vp |  | 1130 | pcphpl | 484 | pcphpl |
| Free-flow speed, FFS |  | 46.0 | mph | 60.0 | mph |
| Avg. passenger-car travel speed, | S | 46.0 | mph | 60.0 | mph |
| Level of service, LOS |  | C |  | A |  |
| Density, D |  | 24.6 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 8.1 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

HCS+: Multilane Highways Release 5.3

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```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 5A LM 5.55 to LM 6.60
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | ft | 6.0 | $f t$ |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | $f t$ |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 50.0 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 44.4 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1344 | vph | 576 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 354 |  | 152 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | 0 | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 739 | pcphpl | 316 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :--- | :--- | :--- | :--- |
| Flow rate, vp | 739 | pcphpl | 316 | pcphpl |
| Free-flow speed, FFS | 44.4 | mph | 50.0 | mph |
| Avg. passenger-car travel speed, S |  | mph | 50.0 | mph |
| Level of service, LOS |  |  | A |  |
| Density, D |  | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 6.3 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 5A LM 5.55 to LM 6.60
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 | 2 |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | ft | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | ft | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | $f t$ |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | $\bigcirc$ |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measured |  |
| FFS or BFFS | 50.0 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 44.4 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1546 | vph | 662 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 407 |  | 174 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | 0 | \% | 0 | \% |
| Terrain type | Rolling |  | Rolling |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 850 | pcphpl | 364 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :---: | :--- | :--- | :--- |
| Flow rate, vp | 850 | pcphpl | 364 | pcphpl |
| Free-flow speed, FFS | 44.4 | mph | 50.0 | mph |
| Avg. passenger-car travel speed, S |  | mph | 50.0 | mph |
| Level of service, LOS |  |  | A |  |
| Density, D |  | $\mathrm{pc} / \mathrm{mi} / \ln 7.3$ | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |  |

Overall results are not computed when free-flow speed is less than 45 mph .

HCS+: Multilane Highways Release 5.3

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E-mail:
```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 5B LM 6.60 to LM 8.67
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | $f t$ |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | ft | 6.0 | ft |
| Left edge | 6.0 | $f t$ | 6.0 | $f t$ |
| Total lateral clearance | 12.0 | ft | 12.0 | $f t$ |
| Access points per mile | 16 |  | $\bigcirc$ |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 60.0 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 54.4 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 840 | vph | 360 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 221 |  | 95 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | 0 | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 462 | pcphpl | 198 | pcphpl |


| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Flow rate, vp | 462 | pcphpl | 198 | pcphpl |
| Free-flow speed, FFS | 54.4 | mph | 50.0 | mph |
| Avg. passenger-car travel speed, S | 54.4 | mph | 50.0 | mph |
| Level of service, LOS | A |  | A |  |
| Density, D | 8.5 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 4.0 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

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```

OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 5B LM 6.60 to LM 8.67
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | ft | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | ft | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 60.0 | mph | 50.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 54.4 | mph | 50.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1025 | vph | 439 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 270 |  | 116 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | 0 | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 563 | pcphpl | 241 | pcphpl |



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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2014
Highway: SR-6
From/To: Seg. 6 LM 8.67 to 10.03
Jurisdiction: TDOT
Analysis Year: 2014
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | $f t$ | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 45.0 | mph | 45.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 39.4 | mph | 45.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1165 | vph | 499 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 307 |  | 131 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | $\bigcirc$ | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 640 | pcphpl | 274 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :---: | :--- | :--- | :--- |
| Flow rate, vp | 640 | pcphpl | 274 | pcphpl |
| Free-flow speed, FFS | 39.4 | mph | 45.0 | mph |
| Avg. passenger-car travel speed, S |  | mph | 45.0 | mph |
| Level of service, LOS |  |  | A |  |
| Density, D |  | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 6.1 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

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OPERATIONAL ANALYSIS $\qquad$
Analyst:
Agency/Co:
Date: 11/6/2008
Analysis Period: 2034
Highway: SR-6
From/To: Seg. 6 LM 8.67 to 10.03
Jurisdiction: TDOT
Analysis Year: 2034
Project ID: SR-6 TPR
FREE-FLOW SPEED

| Direction | 1 |  | 2 |  |
| :---: | :---: | :---: | :---: | :---: |
| Lane width | 12.0 | $f t$ | 12.0 | ft |
| Lateral clearance: |  |  |  |  |
| Right edge | 6.0 | $f t$ | 6.0 | ft |
| Left edge | 6.0 | ft | 6.0 | ft |
| Total lateral clearance | 12.0 | ft | 12.0 | ft |
| Access points per mile | 16 |  | 0 |  |
| Median type | Undivid |  |  |  |
| Free-flow speed: | Base |  | Measu |  |
| FFS or BFFS | 45.0 | mph | 45.0 | mph |
| Lane width adjustment, FLW | 0.0 | mph | 0.0 | mph |
| Lateral clearance adjustment, FLC | 0.0 | mph | 0.0 | mph |
| Median type adjustment, FM | 1.6 | mph | 0.0 | mph |
| Access points adjustment, FA | 4.0 | mph | 0.0 | mph |
| Free-flow speed | 39.4 | mph | 45.0 | mph |
| VOLUME |  |  |  |  |
| Direction | 1 |  | 2 |  |
| Volume, V | 1803 | vph | 773 | vph |
| Peak-hour factor, PHF | 0.95 |  | 0.95 |  |
| Peak 15-minute volume, v15 | 474 |  | 203 |  |
| Trucks and buses | 3 | \% | 3 | \% |
| Recreational vehicles | $\bigcirc$ | \% | 0 | \% |
| Terrain type | Rolling |  | Rolli |  |
| Grade | 0.00 | \% | 0.00 | \% |
| Segment length | 0.00 | mi | 0.00 | mi |
| Number of lanes | 2 |  | 2 |  |
| Driver population adjustment, fP | 1.00 |  | 1.00 |  |
| Trucks and buses PCE, ET | 2.5 |  | 2.5 |  |
| Recreational vehicles PCE, ER | 2.0 |  | 2.0 |  |
| Heavy vehicle adjustment, fHV | 0.957 |  | 0.957 |  |
| Flow rate, vp | 991 | pcphpl | 425 | pcphpl |


|  | Direction | 1 |  | 2 |
| :--- | :--- | :--- | :--- | :--- |
| Flow rate, vp | 991 | pcphpl | 425 | pcphpl |
| Free-flow speed, FFS | 39.4 | mph | 45.0 | mph |
| Avg. passenger-car travel speed, S |  | mph | 45.0 | mph |
| Level of service, LOS |  |  | A |  |
| Density, D |  | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ | 9.4 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |

Overall results are not computed when free-flow speed is less than 45 mph .

## Maury and Williamson Counties

## THREE LANE SPRING HILL OPTION

HCS+ ANALYSIS

Segment 2A L.M. 0.00 to 0.20 Year 2014
Segment 2A L.M. 0.00 to 0.20 Year 2034

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PLANNING ANALYSIS
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2A
Analysis Year: 2014
Project ID: SR-6 TPR LM 0.00 to 0.20 (SH 3 Lane Option)
Traffic Characteristics

| Annual average daily traffic, AADT | 25000 | vpd |  |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |  |
| Directional distribution factor, D | 0.700 |  |  |
| Peak-hour factor, PHF | 0.950 |  |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |  |
| Percent turns from exclusive lanes | 50 | $\%$ |  |
|  | Roadway Characteristics_- |  |  |
|  |  |  |  |
| Number of through lanes one direction, N | 1 |  |  |
| Free flow speed, FFS | 45 | mph |  |
| Urban class | 2 |  |  |
| Section length | 2.49 | miles |  |
| Median | Yes |  |  |
| Left-turn bays | Yes |  |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results

| Annual average daily traffic, AADT | 25000 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 1999 | vph |
| Hourly directional volume | 1399 | vph |
| Through-volume 15-min. flow rate | 736 | v |
| Running time | 199.2 | sec |
| v/c ratio | 1.02 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 36.0 | sec |
| Filtering/metering factor, I | 0.090 |  |
| Incremental delay | 16.3 | sec |
| Control delay | 48.5 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 30.3 | mph |
| Total urban street LOS | B |  |

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E-Mail:
PLANNING ANALYSIS

| Analyst: |  |
| :--- | :--- |
| Agency/Co.: |  |
| Date Performed: | $11 / 6 / 2008$ |
| Analysis Time Period: | 2014 |
| Urban Street: | SR-6 |
| Direction of Travel: |  |
| Jurisdiction: | Segment 2A |
| Analysis Year: | 2034 |
| Project ID: SR-6 TPR LM 0.00 to 0.20 |  |

Traffic Characteristics

| Annual average daily traffic, AADT 36700 |  |  | vpd |
| :---: | :---: | :---: | :---: |
| Planning analysis hour factor, K 0.080 |  |  |  |
| Directional distribution factor, D 0.700 |  |  |  |
| Peak-hour factor, PHF 0.950 |  |  |  |
| Adjusted saturation flow rate | 180 |  | pcphgpl |
| Percent turns from exclusive lanes 50 |  |  | \% |
| Roadway Characteristics |  |  |  |
| Number of through lanes one direction, N 1 |  |  |  |
| Free flow speed, FFS |  | 45 | mph |
| Urban class |  | 2 |  |
| Section length |  | 2.49 | miles |
| Median |  | Yes |  |
| Left-turn bays |  | Yes |  |


| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = for planning) | Actuated |  |
| Cycle length, C | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 1081 | v |
| Running time | 199.2 | sec |
| v/c ratio | 1.50 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 36.0 | sec |
| Filtering/metering factor, I | 0.090 |  |
| Incremental delay | 226.3 | sec |
| Control delay | 258.5 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 12.5 | mph |
| Total urban street LOS | F |  |

## THREE LANE THOMPSON'S STATION OPTION

HCS+ ANALYSIS

Segment 2C L.M. 2.17 to 2.49 Year 2014
Segment 2C L.M. 2.17 to 2.49 Year 2034

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Nashville, TN 37217
Phone: 615-399-9090
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E-Mail:
PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2014
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2014
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 25000 | vpd |
| :--- | :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
| Roadway Characteristics |  |  |


| Number of through lanes one direction, $N$ | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 |  |
| :--- | :--- | :--- |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actuated |  |
| Cycle length, c | 120.0 | sec |
| Effective green ratio, g/C | 0.400 |  |

Results
Annual average daily traffic, AADT 25000
Two-way hourly volume
Two-way hourly volume 1999 vph
Hourly directional volume 1399 vph
Through-volume 15-min. flow rate 368 v
Running time
199.2 sec
v/c ratio
0.51

Through capacity
Progression factor, PF
Uniform delay 27.2
Filtering/metering factor, I
Incremental delay
Control delay
Total travel speed, Sa
Total urban street LOS
720 vph
0.895
27.2 sec
0.849
2.2 sec
26.5 sec/v
35.5 mph

A

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Nashville, TN 37217
Phone: 615-399-9090
Fax:
E-Mail:
PLANNING ANALYSIS $\qquad$
Analyst:
Agency/Co.:
Date Performed: 11/6/2008
Analysis Time Period: 2034
Urban Street: SR-6
Direction of Travel:
Jurisdiction: Segment 2C
Analysis Year: 2034
Project ID: SR-6 TPR LM 2.17 to 2.49
Traffic Characteristics

| Annual average daily traffic, AADT | 36700 | vpd |
| :--- | :--- | :--- |
| Planning analysis hour factor, K | 0.080 |  |
| Directional distribution factor, D | 0.700 |  |
| Peak-hour factor, PHF | 0.950 |  |
| Adjusted saturation flow rate | 1800 | pcphgpl |
| Percent turns from exclusive lanes | 75 | $\%$ |
|  | Roadway Characteristics_ |  |


| Number of through lanes one direction, $N$ | 1 |  |
| :--- | :--- | :--- |
| Free flow speed, FFS | 45 | mph |
| Urban class | 2 |  |
| Section length | 2.49 | miles |
| Median | Yes |  |
| Left-turn bays | Yes |  |

Signal Characteristics $\qquad$

| Signalized intersections | 2 | sec |
| :---: | :---: | :---: |
| Arrival type, AT | 4 |  |
| Signal type (k = 0.5 for planning) | Actua |  |
| Cycle length, C | 120.0 |  |
| Effective green ratio, g/C | 0.400 |  |
| Results |  |  |
| Annual average daily traffic, AADT | 36700 | vpd |
| Two-way hourly volume | 2935 | vph |
| Hourly directional volume | 2054 | vph |
| Through-volume 15-min. flow rate | 540 | v |
| Running time | 199.2 | sec |
| v/c ratio | 0.75 |  |
| Through capacity | 720 | vph |
| Progression factor, PF | 0.895 |  |
| Uniform delay | 30.9 | sec |
| Filtering/metering factor, I | 0.579 |  |
| Incremental delay | 4.2 | sec |
| Control delay | 31.8 | $\mathrm{sec} / \mathrm{v}$ |
| Total travel speed, Sa | 34.1 | mph |
| Total urban street LOS | B |  |


[^0]:    = referenced from previous input price

[^1]:    Cemetery Sites \& Cemetery Properties
    Aquatic Species

[^2]:    Moderate - Medium impact on the project is expected as there is a known occurrence of federally-protected aquatic species or a state protected species with a status of threatened or endangered located within the project study area or corridor. Additional alternatives could likely reduce species impacts. Consultation with the US Fish and Wildlife Service and/or the Tennessee Wildlife Resources Agency will be required possibly resulting in a survey for the species. Special construction considerations may be required.

