# TRANSPORTATION PLANNING REPORT 

State Route 115 (U.S. 129) at Louisville Road
INTERSECTION IMPROVEMENTS
BLOUNT COUNTY
PIN\# 107899.00

PREPARED BY
HMB PROFESSIONAL ENGINEERS, INC.
FOR
TENNESSEE DEPARTMENT OF TRANSPORTATION
PROJECT PLANNING DIVISION

| Recommended by: | Signature | DATE |
| :--- | :--- | :--- |
| CHIEF OF <br> ENVIRONMENT <br> AND PLANIING |  |  |
| TRANSPORTATION DIRECTOR <br> PROJECT PLANNING DIVISION |  | 8 |
| TRANSPORTATION MANAGER 2 <br> PROJECT PLANNING DIVISION |  | 8 |

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## BACKGROUND

The Tennessee Department of Transportation (TDOT) has initiated this study in response to a request from the Knoxville Regional Transportation Planning Organization and the City of Alcoa to evaluate the intersection of State Route 115 (U.S. 129) at Louisville Road in Blount County, Tennessee. This request was advanced due to the rapid increase of commercial and retail development within this immediate area. In addition to increased traffic, vehicular crashes have also increased significantly. Vicinity and location maps are provided in Figures $\mathbf{1}$ and $\mathbf{2}$ for reference. The study area includes not only this main intersection but three other intersections located immediately to the east and west along Louisville Road.

## EXISTING CONDITIONS

The intersection of SR-115 (US 129) and Louisville Road is located in the City of Alcoa, which lies between the City of Maryville and the City of Knoxville. The study area consists of this major signalized intersection, two intersections immediately to the west, one of which is signalized, and another unsignalized intersection immediately to the east.

SR-115 (US 129) is a major four-lane arterial roadway consisting of $12-\mathrm{ft}$ travel lanes, with a full 12 -ft outside shoulder, and having a 38 -ft divided median. This roadway serves as a major commuter route between Maryville and Knoxville. Base year (2012) average daily traffic (ADT) along this route ranges from 44,560 to 46,460 . This data is based upon 2006 cycle counts and 8 hr turning movement counts obtained on February 20, 2007.

Louisville Road is a two-lane arterial that serves as a east/west connector for this area. This road consists mainly of 12 -ft lanes with no shoulder and is comprised of several left and right auxiliary lanes within the intersection study area. Base year (2012) ADT for this road ranges from 23,160 to 23,590 based upon 2006 cycle counts and 8 -hr turning movement counts obtained on February 20, 2007. This ADT represents data taken at the specific intersection of SR-115 and Louisville Road.

The area south of Louisville Road has been the focus of large commercial development. Located on the southwest quadrant of the intersection are two "big-box" stores (Wal-Mart and Lowes) and several restaurant out-parcels. The southeast quadrant of the intersection currently has several restaurants with two "big-box" retailers (unknown at this time) currently planned for this area and are under construction. At the time of this report, further commercial and retail development is planned for both the east and west quadrants, south of Louisville Road. An aerial photograph showing this development is provided in Illustration 1.

The northwest quadrant of the intersection is predominantly residential, with a local church and the county 911 call center located immediately on the corner. No further development is expected or can be accommodated in this area. On the northeast corner resides a local lumber company who is a major supplier of building materials for the surrounding area. Also located on this corner are a few retail stores and restaurants. Future development is expected in this quadrant, but the area is limited and no development is planned at this time.
SCALE: 1:24,000 $\qquad$

## PROJECT VICINITY MAP

INTERSECTION IMPROVEMENTS
SR-115 (US-129) @ LOUISVILLE ROAD

## BLOUNT COUNTY, TN

USGS MARYVILLE QUANDRANGLE
Figure 1


Figure 2

Accident Summary. From the period January 1, 2003 to December 31, 2005, TRIMS crash data obtained from the Tennessee Department of Transportation (TDOT) indicated a total of 186 crashes were reported at the intersection of SR-115 and Louisville Road. Approximately 33 crashes were considered minor injury and only 1 severe injury accident. Of the total number of crashes, 157 of the reported 186 were rear-end collisions. A crash rate of 3.64 was calculated for this period. The critical rate was determined to be 1.23 and the severity index was 0.022 . The ratio of crashes/critical rate was 2.96 and the actual rate/statewide average was 4.00.

In comparison, data obtained from the City of Alcoa indicates that a majority of accidents have occurred within the right turn lane for traffic traveling eastbound Louisville Road to southbound SR-115 and westbound Louisville Road to northbound SR-115. A copy of this data, shown in Figure 3, reveals 267 accidents, 37 of which were considered injury related, occurred at this intersection from the period of January 2004 to December 2006. The figure also illustrates the location and manner of the collisions within the intersection. A majority of the crashes occurred in the right turn lanes from Louisville Road to SR-115.

## COMMUNITY PROFILE

The City of Alcoa is a small industrial and manufacturing community located approximately 20 miles south of Knoxville, Tennessee. The City is situated along SR-115 (US 129) which is a major route between Maryville and Knoxville and is easily accessible to Interstate 40. The name of the city originates from the aluminum company, Alcoa, Inc., then known as the Aluminum Company of America (ALCOA).

In 1910, Alcoa, Inc., began a long-range program to develop power along the Little Tennessee River near present-day Calderwood. Several dams were constructed to supply huge amounts of low-cost hydroelectric power necessary for the production of aluminum. Large land tracts north of Maryville were purchased for the establishment of reduction plants.

The actual City of Alcoa was planned and designed by Alcoa engineers. By 1919, the city contained houses, stores, schools, and other amenities. The early planners established water, sewer, and electrical; created zoning for industrial, commercial, and residential usage; and provided community facilities such as one(1) acre of land for parks and recreation for each 100 persons. The original site for the City covered five and one-half square miles and was designed for a population in excess of 10,000. Since that time, the City has grown to a size almost 15 square miles.

Since that time, Alcoa, Inc. has downsized, but the City has been able to develop a diverse economy consisting of a strong manufacturing base with retail and commercial services. Continuous growth has taken place in both Alcoa and fringe areas as the entire Alcoa-Maryville area has become a growing part of the Knoxville metropolitan area. Table 1 indicates the current and projected growth for the City of Alcoa and surrounding communities.
$\qquad$
$\qquad$
PERIOD 3 YEARS FROM
JAN. 2004
TO $\qquad$ DEC. 2006

NUMBER OF ACCIDENTS

$\qquad$ TOTAL ACCIDENTS
SHOW

1. TIME, DAY, DATE
2. WEATHER AND ROAD

SURFACE- IF UNUSUAL
CONDITIONS EXISTED
3. NITE- IF BETWEEN

DUSK AND DAWN

Table 1 - Current and Projected Census ${ }^{1}$

| County | Municipality |  | $\mathbf{2 0 0 0}$ | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 1 0}$ | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 2 0}$ | $\mathbf{2 0 2 5}$ |
| :--- | :--- | :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Blount | Alcoa | city | 7,734 | 8,316 | 9,056 | 9,808 | 10,540 | 11,252 |
|  | Friendsville | city | 890 | 920 | 953 | 991 | 1,009 | 1,035 |
|  | Louisville | city | 2,001 | 2,188 | 2,424 | 2,664 | 2,904 | 3,147 |
|  | Maryville | city | 23,120 | 24,655 | 26,639 | 28,588 | 30,462 | 32,277 |
|  | Rockford | city | 798 | 830 | 868 | 901 | 941 | 963 |
|  | Townsend | city | 244 | 247 | 265 | 270 | 273 | 287 |
|  | Unincorporated |  | 71,036 | 75,065 | 80,386 | 85,495 | 90,228 | 94,746 |
|  | Total |  | 105,823 | 112,222 | 120,592 | 128,718 | 136,357 | 143,707 |

${ }^{1}$ Source: A joint publication of the Tennessee Advisory Commission on Intergovernmental Relations and The University of Tennessee Center for Business and Economic Research.

Land Use. In year 2003, it was determined that the City of Alcoa has approximately 22,366 acres in and around the City limits; approximately 9,730 acres lie within the City limits. Approximately 11,024 acres, or roughly $50 \%$, of this area is presently developed. Table 2 indicates the current distribution of land use followed by Figure 4 on the following page which offers a visual illustration of this usage.

Table 2 - Existing Land Use (2003) ${ }^{2}$

| Land Use Category | Acres | Percentage of Total Land Area | Percentage of Developed Land | $\begin{gathered} \text { Acres } \\ \text { Per } \\ \text { Person } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: |
| Residential | 5,058 | 23 | 46 | 1.5 |
| Commercial/Office | 1,260 | 6 | 11 | 6.1 |
| Industrial | 1,237 | 6 | 11 | 6.3 |
| Airport | 1,421 | 6 | 13 | 5.4 |
| Parks/Open Space/Public |  |  |  |  |
| Buildings | 1,630 | 7 | 15 | 4.7 |
| Institutional/Churches/Cemetery | 418 | 2 | 4 | 18.5 |
| Total Developed | 11,024 | 50 | N/A | 7 |
| Vacant(Developable) | 9,717 | 43 | N/A | N/A |
| Right-of-Way | 424 | 2 | N/A | N/A |
| Water | 910 | 4 | N/A | N/A |
| Wetlands | 293 | 1 | N/A | N/A |
| Within City Limits | 9,730 | 4 | N/A | N/A |

${ }^{2}$ Source: TVA Land Use Survey, as provided in 2025 Comprehensive Plan, May 9, 2006, City of Alcoa.



The data provided in Table 3 shown below projects the developable percentage of land use based upon the respective categories. Also, a visual illustration of future land use is shown in Figure 5.

Table 3 - Future Land Use ${ }^{2}$

| Land Use Category | Acres | Percentage of <br> Total Land Area |
| :--- | :---: | :---: |
| Residential | 9,204 | 41 |
| Commercial/Office | 3,932 | 18 |
| Industrial | 4,996 | 22 |
| Parks/Open |  |  |
| Space/Public | 3,033 | 14 |
| Buildings | 910 | 4 |
| Water | 293 | 1 |
| Wetlands |  |  |

## PURPOSE AND NEED

The objective of this report is to analyze existing and projected conditions at the intersection of SR-115 (US 129) and Louisville Road located in Blount County, Tennessee, to determine the purpose and need for improvements. This study was initiated due to the rapid and large commercial/retail development, as well as industrial development, occurring within the area which is surrounded by existing residential neighborhoods. These entities are a contributing factor of additional traffic to an already congested local and commuter transportation system. Also of significant interest is the number of vehicular crashes occurring in this area.

The intersection is affected by three adjacent intersections which are also included as part of this study. This study will review existing conditions, analyze traffic for existing and future conditions and levels of service, prepare recommendations for improvement, and determine preliminary costs relating to the proposed recommendations.

The primary need along SR-115 (US 129) is to provide for improved local and regional mobility and access. Several specific needs are included in this goal.

1. Provide an improved north/south route to serve demand for local and regional access to the interstate and neighboring communities.
2. Improve safety and mobility along SR-115 (US 129).
3. Accommodate the increased traffic demand spurred by commercial development within the adjacent intersection and local community.
4. Create an opportunity for additional economic growth within the City of Alcoa and Blount County by providing an improved transportation system.

## PROPOSED IMPROVEMENTS

Year 2012 traffic data for the main SR-115/Louisville Road intersection indicates this intersection and adjacent intersections have capacity deficiencies. Also, future year volumes suggest that major relief to congestion and driver comfort can only be achieved through developing a gradeseparated interchange at this location; SR-115 being elevated over Louisville Road. However, several issues complicate the viability this option.

1. Cost.
2. Inconvenience to the motoring public during construction.
3. Limited right-of-way.
4. No new parallel corridors or improvements to existing facilities have been considered to alleviate congestion along the SR-115 corridor. Local officials are currently working with the TPO to determine projects for the long-range plan. (Note, TDOT is currently developing right-of-way plans for widening Middlesettlements Road, located immediately south, which is scheduled due by end of year 2007. While this does not affect the SR-115 corridor, it will have an affect upon this study area since Hunters Crossing ties into Middlesettlements Road.)

While a grade-separated interchange does meet the purpose and need for this project, this option was not considered due to the aforementioned issues. Further study is suggested that will encompass a much broader study area for the purpose of reviewing the affects of a gradeseparated interchange and its impacts along the entire SR-115 corridor.

Assessment of the study area by the design team, and input from the stakeholders field review, determined the following options that will best meet the purpose and need goals for this project.

Option 1: $\quad$ Create auxiliary lanes along SR-115 for the movements eastbound (EB) Louisville Road to southbound (SB) SR-115, and westbound (WB) Louisville Road to northbound (NB) SR-115. Also create a deceleration lane for northbound (NB) SR-115 to eastbound (EB) Louisville Road.
Option 2: $\quad$ Signalize the right-turn movements for EB Louisville Road to SB SR-115 and WB Louisville Road to NB SR-115.
Option 3: $\quad$ Remove the left turn entry from Louisville Road onto both Hunters Crossing and Brenda Drive, and define these intersections to be right-in/right-out only.
Option 4: Create a double left-turn movement for WB Louisville Road onto SB SR115.

Suggested Options: Several options are recommended as a suggestion to improving the overall operation and safety of the intersection at SR-115/Louisville Road, based upon a traffic operations standpoint. Because of their significant impact to the overall area and community, further study and stakeholder involvement is suggested to review the affects of these suggestions and consider other possible alternatives.

1. Eliminate the intersection of Hunters Crossing at Louisville Road.
2. Eliminate the intersection of Brenda Drive at Louisville Road.
3. Eliminate the intersection of the Unnamed Business Entrance (adjacent to Cracker Barrel) at Louisville Road.

A copy of the meeting minutes from the stakeholder field review is located in Appendix A.

Option 1:
Existing right-turn lanes at the intersection of SR-115/Louisville Road are such that they limit freeflow movement of traffic when exiting Louisville Road to SR-115. Currently vehicles on Louisville Road must yield to on-coming traffic along SR-115 when making a right turn which accounts for a large number of rear-end collisions occurring at these points. Development of an auxiliary lane for the movements EB Louisville Road to SB SR-115 and WB Louisville Road to NB SR-115 will likely help alleviate congestion by facilitating free-flow operations and reducing traffic queues waiting to enter the roadway, thereby reducing and/or eliminating potential crashes. A deceleration lane is also suggested for NB SR-115 to EB Louisville Road.


The conceptual layout illustrated in Figures 6-1 and 6-2 shows that sufficient length along SR115 exists for the development of a full $12-\mathrm{ft}$ auxiliary lane in the suggested locations. A minimum length of 660 -feet can be obtained for the northbound and southbound acceleration lanes along SR-115 (Exhibit 10-70, AASHTO Greenbook - Chapter 10). Also, a minimum length of 435 -feet can be obtained for the deceleration lane for northbound traffic along SR-115 ((Exhibit 10-73, AASHTO Greenbook - Chapter 10). Addition of the recommended auxiliary lanes can be accommodated within the existing right-of-way without the need for acquiring additional right-ofway.

Creation of an auxiliary lane along SB SR-115 will introduce a weaving situation near the exit ramp for the Wal-Mart shopping center and Middlesettlements Road. No traffic data was available for this exit ramp, therefore $15 \%$ traffic was assumed. Table 4 indicates the results of this weave analysis.



Figure 6-3 - COST DATA SHEET (OPTION 1)

| PROJECT | SR-115 (US 129) @ LOUISVILLE ROAD |
| :--- | :--- |
| LOCATION | BLOUNT COUNTY |
| LENGTH | N/A |
| CROSS SECTION |  |

## RIGHT-OF-WAY

Land, Improvements, \& Damages
Incidentals
Relocation

| (\# Acres $=$ | ) | $\$$ | - |  |
| :--- | :--- | :--- | :--- | :--- |
| (\# Tracts $=$ | ) | $\$$ | - |  |
| (Residences $=$ | ) | $\$$ | - |  |
| (Businesses $=$ | ) | $\$$ | - | $\$$ |
| HT-OF-WAY COST |  |  |  | $\$$ |

## UTILITY RELOCATION

Reimbursable
Non-Reimbursable

|  | $\$$ | - |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  | $\$$ | - |  |  |
| TOTAL UTILITY COST |  |  | $\$$ | - |

## CONSTRUCTION ITEMS

Mobilization
Clearing \& Grubbing
Removal of Pavement
Earthwork
Drainage
Structures
Concrete Curb \& Gutter
Paving
Maintenance of Traffic
Concrete Barrier Rail
Seeding
Signing/Pavement Marking
Signalization
Silt Fence
Rip-rap or Slope Protection
Erosion Control (assume 5\%)
Other Items (assume 7.5\%)
10\% Contingency

45,000
5,000
49,000
43,000
5,000
-

232,000
25,000
50,000
6,000
11,000
12,000
31,000
46,000
60,000

TOTAL CONSTRUCTION COST

| $\mathbf{\$}$ | $\mathbf{6 2 0 , 0 0 0}$ |
| :--- | :--- |
| $\$$ | 62,000 |

Estimated Cost (OPTION 1)
\$ 682,000

TABLE 4 - PEAK HOUR LEVELS OF SERVICE WEAVE WITH FUTURE ACCELERATION LANES

| LOCATION | YEAR | AM PEAK <br> HOUR | PM PEAK <br> HOUR |
| :--- | :---: | :---: | :---: |
|  | 2012 | LOS A | LOS B |
| State Route 115, between <br> Louisville Road and ramp to <br> Hunter's Crossing Drive / <br> Bessemer Street | 2032 | LOS B | LOS C |

## Option 2:

As an alternative to Option 1, additional consideration was given to reconstructing the eastbound and westbound right turn lanes so that they would be controlled by the traffic signal instead of the existing yield conditions. Under this scenario, the eastbound and westbound right turns would be given "green" time with the eastbound and westbound through movements. Also, right turn overlap signal phases would be provided to allow the eastbound and westbound right turns to operate with the northbound and southbound left turns. With this configuration, it is likely that rear-end accidents would decrease for the eastbound and westbound turning movements. However, the projected vehicle queues for these turning movements will be excessive, creating further congestion and delays. Specifically, based on both Year 2012 and Year 2032 traffic volumes, these queues are expected to extend beyond the adjacent intersections. Table 5 shown in the next section, Level of Service, discusses the results of this analysis. Specifically, Table $5 \mathbf{a}$ and $\mathbf{5 b}$ include the results of the capacity analyses conducted for the existing laneage and traffic control. Table 5c and 5d include the results of the analyses conducted for the changes proposed for Option 2. A conceptual layout for this option is provided in Figure 7.

Since single right-turn lanes are not feasible, the formation of double right-turns for this option was considered. Analysis revealed this configuration also creates excessive vehicle queues, adding further congestion and delays. Results are also provided in Table 5e and $\mathbf{5 f}$.


## Option 3:

Currently, vehicles can access Hunters Crossing and Brenda Drive directly via left-turn lanes off of Louisville Road. Each of these intersections reside approximately $350-\mathrm{ft}$ within the main SR115/Louisville Road intersection. The close proximity of these intersections combined with the heavy traffic volumes creates added congestion, backups, and reduced driver comfort levels within the area. This option proposes eliminating the left turns from Louisville Road. In each case, vehicles that desire to turn left onto the side streets can utilize an adjacent signalized intersection which will still provide easy access to these locations. Hunters Crossing and Brenda Drive would then be striped into a right-in/right-out only situation. A conceptual layout of this option is provided in Figure 8-1.


Left-turn onto Brenda Drive to be eliminated.

## Option 4:

Traffic movement from WB Louisville Road to SB SR-115 currently utilizes a single left-turn lane at the intersection. Adequate area exists to create a double left-turn for this movement. As discussed in Option 3, the elimination of the left-turn from Louisville Road onto Brenda Drive would further facilitate this option and produce greater storage for this particular movement. A conceptual striping reconfiguration is also shown in Figure 8-1.


Figure 8-2 - COST DATA SHEET (OPTION 3)

| PROJECT | SR-115 (US 129) @ LOUISVILLE ROAD |
| :--- | :--- |
| LOCATION | BLOUNT COUNTY |
| LENGTH | N/A |
| CROSS SECTION |  |

## RIGHT-OF-WAY

Land, Improvements, \& Damages (\# Acres = \$
Incidentals
Relocation

| (\# Tracts $=$ | ) | $\$$ |
| :--- | :--- | :--- |
| (Residences $=$ | ) | $\$$ |
| (Businesses $=$ | ) | $\$$ |

TOTAL RIGHT-OF-WAY COST
\$

## UTILITY RELOCATION

Reimbursable
Non-Reimbursable

|  | $\$$ | - |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  | $\$$ | - |  |  |
| TOTAL UTILITY COST |  |  | $\$$ | - |

## CONSTRUCTION ITEMS

| Mobilization | $\$$ | 3,000 |
| :--- | :---: | :---: |
| Clearing \& Grubbing | $\$$ | - |
| Removal of Pavement | $\$$ | - |
| Earthwork | $\$$ | - |
| Drainage | $\$$ | - |
| Structures | $\$$ | - |
| Concrete Curb \& Gutter | $\$$ | - |
| Concrete Median Pavement | $\$$ | 10,000 |
| Paving | $\$$ | - |
| Maintenance of Traffic | $\$$ | 5,000 |
| Seeding | $\$$ | - |
| Signing/Pavement Marking | $\$$ | 5,000 |
| Removal of Pavement Marking | $\$$ | 11,000 |
| Signalization | $\$$ | 10,000 |
| Silt Fence | $\$$ | - |
| Rip-rap or Slope Protection | $\$$ | - |
| Erosion Control (assume 5\%) | $\$$ | - |
| Other Items (assume 5\%) | $\$$ | 2,000 |
| 10\% Contingency | $\$$ | 4,000 |


| TOTAL CONSTRUCTION COST |
| :--- |
| Engineering Cost ( $10 \%$ of Constr.) |

Estimated Cost (OPTION 3)

\$ 55,000

Figure 8-3 - COST DATA SHEET (OPTION 4)

| PROJECT | SR-115 (US 129) @ LOUISVILLE ROAD |
| :--- | :--- |
| LOCATION | BLOUNT COUNTY |
| LENGTH | N/A |
| CROSS SECTION |  |

## RIGHT-OF-WAY

Land, Improvements, \& Damages (\# Acres = \$
Incidentals
Relocation

| (\# Tracts $=$ | ) | $\$$ |
| :--- | :--- | :--- |
| (Residences $=$ | ) | $\$$ |
| (Businesses $=$ | ) | $\$$ |

TOTAL RIGHT-OF-WAY COST
\$

## UTILITY RELOCATION

Reimbursable
Non-Reimbursable


## CONSTRUCTION ITEMS

Mobilization
Clearing \& Grubbing
Removal of Pavement
Earthwork
Drainage
Structures
Concrete Curb \& Gutter
Concrete Median Pavement
Paving
Maintenance of Traffic
Seeding
Signing/Pavement Marking
Removal of Pavement Marking
Signalization
Silt Fence
Rip-rap or Slope Protection
Erosion Control
Other Items (assume 5\%)
10\% Contingency

| $\$$ | 3,000 |
| :--- | :---: |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | 3,000 |
| $\$$ | - |
| $\$$ | 14,000 |
| $\$$ | 5,000 |
| $\$$ | - |
| $\$$ | - |
| $\$$ | - |
| $\$$ | 1,500 |
| $\$$ | 2,500 |

TOTAL CONSTRUCTION COST
Engineering Cost ( $10 \%$ of Constr.)
Estimated Cost (OPTION 4)

| $\$$ | 29,000 |
| :---: | :---: |
| $\$$ | 3,000 |

\$ 32,000

## Suggested Options:

The intersections of Hunters Crossing, Brenda Drive, and Unnamed Business Entrance (near the Cracker Barrel restaurant) along Louisville Road impact the overall traffic operation and safety of the main intersection SR-115 and Louisville Road. Their close proximity, approximately 350 -ft., $350-\mathrm{ft}$., and $450-\mathrm{ft}$., respectively, creates added congestion, delays, and limits vehicle storage due to their turning movements along Louisville Road. The operation and performance of the intersection at SR-115 and Louisville Road suggests these intersections be closed due to their close proximity to the main intersection. Figure 9 illustrates conceptually that existing traffic could utilize a nearby signalized intersection; Hunters Crossing rerouted to the signalized Wal-Mart entrance, Brenda Drive and the Unnamed Business Entrance rerouted to the signalized intersection at Marilyn Drive.

Improvements to these intersections do not fall within the scope of this report due to the local jurisdiction surrounding this study area. It is recommended that further study be initiated between local officials and stakeholders such as Wal-Mart, Cracker Barrel, Anderson Lumber, etc., who will be impacted the most to discuss the affects of closure at these intersections and determine acceptable solutions.


## LEVEL OF SERVICE

Operating conditions within a transportation route are distinguished by a "Level of Service" (LOS) analysis. This analysis reflects the ability of the roads, in this case, intersections, to accommodate motor vehicle traffic and subsequent physical and psychological comfort levels of drivers. A LOS analysis considers several factors including traffic volumes, number of travel lanes, terrain, truck traffic, and turning movements. Existing and projected traffic volumes for this study report are included in Appendix B of this report. Project volumes are for the base year (present year +5 years) and design year (present year +25 years). Schematic diagrams of each intersection are also included with their respective traffic volume.

LOS is a qualitative measure that describes the character of traffic conditions related to speed and travel time, freedom to maneuver, congestion, etc. There are six levels of operation ranging from "A" to " $F$ " with " $F$ " being the worst. A description of the operating conditions for each level is provided in the following.
\(\left.$$
\begin{array}{cl}\text { LOS } & \begin{array}{l}\text { Traffic Flow Condition } \\
\text { A } \\
\text { Free flow operations. Vehicles are almost completely unimpeded in their } \\
\text { ability to maneuver within the traffic stream. The general level of physical } \\
\text { and psychological comfort provided to the driver is the highest. }\end{array} \\
\text { C } & \begin{array}{l}\text { Reasonably free flow operation. The ability to maneuver within the traffic } \\
\text { stream is only slightly restricted and the general level of physical and } \\
\text { psychological comfort provided to the driver is still high. }\end{array}
$$ <br>
D Flow speeds at or near free flow. Freedom to maneuver within the traffic <br>
stream is noticeably restricted and lane changes require more vigilance on <br>
the part of the driver. The drive notices an increase in tension because of <br>

the additional vigilance required for safe operation.\end{array}\right\}\)| Speeds decline with increasing traffic. Freedom to maneuver within the |
| :--- |
| traffic stream is more noticeably limited. The driver experiences reduced |
| physical and psychological comfort levels. |

Specifically, these analyses are based on the Year 2012 and Year 2032 projected traffic volumes that have been developed by the Tennessee Department of Transportation (TDOT). In addition, it is important to note that two scenarios were considered:

## Existing Laneage and Traffic Control Proposed Laneage and Traffic Control

The proposed laneage and traffic control includes the following changes to the existing system:

## State Route 115 and Louisville Road (Option 1)

It was assumed that an acceleration lane will be constructed on northbound State Route 115 for vehicles in the westbound right turn lane. Also, it was assumed that an acceleration lane will be constructed on southbound State Route 115 for vehicles in the eastbound right turn lane. Although these improvements will not provide additional capacity at the intersection of State Route 115 and Louisville Road, they are expected to reduce the vehicle crashes at this location.

## Addition of Signalized Right-turn for WB / EB Louisville Road at SR-115 (Option 2)

Single and double right-turn lanes to be controlled by the signal at SR-115 were each considered for eastbound and westbound Louisville Road. Results for these configurations are shown in Table 5.

The intersection at Marilyn Drive was not part of this report's scope of work, and therefore no traffic data was available to analyze the performance of this intersection

A copy of the capacity analysis worksheets for this study can be found in Appendix C of this report.

TABLE 5a - YEAR 2012 PEAK HOUR LEVELS OF SERVICE EXISTING LANEAGE AND TRAFFIC CONTROL

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | $\begin{gathered} \text { LOS E } \\ 73.7 \\ \text { sec/veh } \end{gathered}$ | 17 vehicles | LOS F <br> 262 sec/veh | 20 vehicles |
|  | Westbound Left Turns |  | 3 vehicles |  | 28 vehicles |
|  | Northbound Lefts/Thrus |  | 7 vehicles |  | 18 vehicles |
|  | Southbound Left Turns |  | 9 vehicles |  | 9 vehicles |

TABLE 5b - YEAR 2032 PEAK HOUR LEVELS OF SERVICE EXISTING LANEAGE AND TRAFFIC CONTROL

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | LOS F <br> $135 \mathrm{sec} / \mathrm{veh}$ | 25 vehicles | LOS F <br> 362 sec/veh | 29 vehicles |
|  | Westbound Left Turns |  | 4 vehicles |  | 38 vehicles |
|  | Northbound Lefts/Thrus |  | 9 vehicles |  | 26 vehicles |
|  | Southbound Left Turns |  | 12 vehicles |  | 11 vehicles |

TABLE 5c - YEAR 2012 PEAK HOUR LEVELS OF SERVICE WITH EASTBOUND AND WESTBOUND RIGHT TURNS INCLUDED IN THE TRAFFIC SIGNAL AND RIGHT TURN OVERLAP PHASES PROVIDED

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | LOS F <br> 160 sec/veh | 17 vehicles | LOS F 309 sec/veh | 20 vehicles |
|  | Eastbound Right Turns |  | 18 vehicles |  | 65 vehicles |
|  | Westbound Left Turns |  | 3 vehicles |  | 28 vehicles |
|  | Westbound Right Turns |  | 82 vehicles |  | 49 vehicles |
|  | Northbound Lefts/Thrus |  | 7 vehicles |  | 18 vehicles |
|  | Southbound Left Turns |  | 9 vehicles |  | 9 vehicles |

TABLE 5d - YEAR 2032 PEAK HOUR LEVELS OF SERVICE WITH EASTBOUND AND WESTBOUND RIGHT TURNS INCLUDED IN THE TRAFFIC SIGNAL AND RIGHT TURN OVERLAP PHASES PROVIDED

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | LOS F <br> 241 sec/veh | 25 vehicles | LOS F <br> undefined | 29 vehicles |
|  | Eastbound Right Turns |  | 26 vehicles |  | undefined |
|  | Westbound Left Turns |  | 4 vehicles |  | 38 vehicles |
|  | Westbound Right Turns |  | 102 vehicles |  | undefined |
|  | Northbound Lefts/Thrus |  | 9 vehicles |  | 26 vehicles |
|  | Southbound Left Turns |  | 12 vehicles |  | 11 vehicles |

TABLE 5e - YEAR 2012 PEAK HOUR LEVELS OF SERVICE WITH DOUBLE EASTBOUND AND WESTBOUND RIGHT TURNS INCLUDED IN THE TRAFFIC SIGNAL AND RIGHT TURN OVERLAP PHASES PROVIDED

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | LOS F <br> 104 sec/veh | 17 vehicles | LOS F <br> 262 sec/veh | 20 vehicles |
|  | Eastbound Right Turns |  | 7 vehicles |  | 29 vehicles |
|  | Westbound Left Turns |  | 3 vehicles |  | 28 vehicles |
|  | Westbound Right Turns |  | 39 vehicles |  | 19 vehicles |
|  | Northbound Lefts/Thrus |  | 7 vehicles |  | 18 vehicles |
|  | Southbound Left Turns |  | 9 vehicles |  | 9 vehicles |

TABEL 5 f - YEAR 2032 PEAK HOUR LEVELS OF SERVICE WITH DOUBLE EASTBOUND AND WESTBOUND RIGHT TURNS INCLUDED IN THE TRAFFIC SIGNAL AND RIGHT TURN OVERLAP PHASES PROVIDED

| INTERSECTION | TURNING MOVEMENT | AM PEAK HOUR |  | PM PEAK HOUR |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LEVEL OF SERVICE | $95^{\text {TH }} \quad \%-$ <br> ILE QUEUE | LEVEL OF SERVICE | $95^{\text {TH }} \quad$ \%ILE QUEUE |
| State Route 115 and Louisville Road | Eastbound Left Turns | LOS F <br> 172 sec/veh | 25 vehicles | LOS F <br> undefined | 29 vehicles |
|  | Eastbound Right Turns |  | 9 vehicles |  | undefined |
|  | Westbound Left Turns |  | 4 vehicles |  | 38 vehicles |
|  | Westbound Right Turns |  | 50 vehicles |  | undefined |
|  | Northbound Lefts/Thrus |  | 9 vehicles |  | 26 vehicles |
|  | Southbound Left Turns |  | 12 vehicles |  | 11 vehicles |

## ASSESSMENT OF OPTIONS

The Tennessee Department of Transportation has adopted seven guiding principles against which all transportation projects are to be evaluated. These guiding principles address concerns for system management, mobility, economic growth, safety, community, environmental stewardship, and fiscal responsibility. The guiding principles are discussed in the following paragraphs as they relate to the options for improving the intersection of SR-115 at Louisville Road.

## Guiding Principle 1: Preserve and Manage the Existing Transportation System

The intersection of SR-115 and Louisville Road is at capacity based upon year 2012 and 2032 traffic. Traffic volumes will continue to increase with the continued commercial and retail development. Options 1, 2, 3, and 4 are not intended to reduce traffic volumes but rather increase safety, reduce vehicle delays, and improve operation. Implementation of Option 1 or 2, and Option 3 and 4 will provide the greatest affect. Suggested closure of existing intersections should consider surrounding stakeholders prior to implementation.

## Guiding Principle 2: Move a Growing, Diverse, and Active Population

Commercial development along the southern portion of the SR-115/Louisville Road has increased tremendously. Further commercial and industrial development within the area is anticipated which will also affect existing transportation network. The options for improvements are necessary to address an expanding market while accommodating regional commuter activity. Suggested options should be reviewed along with discussion and input between local officials and stakeholders.

## Guiding Principle 3: Support the State's Economy

SR-115(US129) is a major corridor between Maryville and Knoxville. This highway serves as a major commuter route as well as providing direct and indirect access for industry located within Alcoa. The Knoxville Regional Airport is also located along SR-115, just a few miles to the north. The cities of Alcoa and Maryville have consistently grown since 2000, and continued growth is expected. Alcoa estimates that commercial/office and industrial businesses will account for forty percent ( $40 \%$ ) of usable land and residential accounting for over forty percent (40\%), a combined $80 \%$, within the City's boundaries by year 2025. The improvement of the SR-115/Louisville Road intersection and corridor will create improved and safer access to residential, commercial, and industrial areas in the future.

## Guiding Principle 4: Maximize Safety and Security

A crash rate of 3.64 was calculated for the period January 1, 2003 to December 31, 2005, and the statewide average rate is 0.91 . The critical rate was determined to be 1.23 and the severity index was 0.02 . The ratio of actual/critical rate was 2.96 and the actual rate/statewide average was 4.00. A total of 186 crashes were reported with only one crash considered as severe injury during this period. A majority of crashes occurred in the right turn lanes from Louisville Road to SR-115.

## Guiding Principle 5: Build Partnerships for Livable Communities

Communication and involvement with stakeholders affected by the suggested closure options is crucial to the overall community. In addition, local officials have been coordinating with Knoxville TPO staff as well as TDOT officials to identify objectives for improving the region's transportation system. Long term goals would not only address this particular intersection but the entire SR-115 corridor.

## Guiding Principle 6: Promote Stewardship of the Environment

A detailed environmental study will not be necessary to implement the improvements from described by each option. These options do not pose a significant impact to the environment.

## Guiding Principle 7: Promote Financial Responsibilities

Preliminary construction cost estimates have been prepared for the various options and are summarized in Table 6.

Table 6 - Estimated Construction Costs for Proposed Improvements

|  |  |
| :---: | :---: |
| Proposed Improvement | Estimated Cost |
| Option 1 | $\$ 682,000$ |
| Option 3 | $\$ 55,000$ |
| Option 4 | $\$ 32,000$ |

*Note: A cost for Option 2 is not provided as traffic analyses indicate this option fails.

## PRELIMINARY ENVIRONMENTAL ANALYSIS

Information provided by TDOT and a field level survey conducted of the project area indicates there are no historic properties within the study area.

A review within the Area of Potential Effects (APE) displays there potentially could be some impact due to floodplain in the immediate area. The APE is the geographic area in which an activity may directly or indirectly impact the environment. A floodplain map, as provided by the Federal Emergency Management Agency (FEMA), indicates a portion of the northeast quadrant of the intersection at SR-115 and Louisville Road may lie within the designated zone. The limited improvements should not have any impact or effect to the existing flood zone. A copy of the floodplain map is provided on the following page in Figure 10.

An evaluation of existing environmental conditions was noted during the stakeholder field review and noted in the Preliminary Environmental Evaluation checklist. A copy of the form is also provided on the following pages.


## Preliminary Environmental Evaluation

If preliminary field reviews indicate the presence of any of the following facilities or Economic, Social and Environmental categories (ESE), place the number of facilities in the blank opposite the item. Where more than one location option is to be considered, place its letter designation in the blank.

## Option

1.) Hazardous Material Site or Underground Storage Tanks. $\qquad$
2.) Floodplains $\qquad$
$\qquad$
3.) Historical, archaeological, cultural, or natural landmark, or cemeteries $\qquad$
4.) Airport $\qquad$
5.) Residential establishment.
6.) Urban area, city, town, or community $\qquad$
 (Alcoa , Pop. 7,734)
7.) Commercial area, shopping center $\qquad$
8.) Institutional usages:
a. School or other educational institution
b. Hospital or other medical facility
$\qquad$
$\qquad$
c. Church or other religious institution.
d. Public Building, e.g., fire station.

e. Defense installation. $\qquad$
9.) Agricultural land usage. $\qquad$
$\qquad$
10.) Forested land $\qquad$
11.) Industrial park, factory $\qquad$
$\qquad$
12.) Recreational usages:
a. Park or recreational area, State Natural Area. $\qquad$
$\qquad$
b. Wildlife refuge or wildlife management area. $\qquad$
$\qquad$
13.) Waterway:
a. Lake $\qquad$
$\qquad$
b. Pond $\qquad$
c. River. $\qquad$
d. Stream.
e. Spring. $\qquad$
14.) Railroad Crossings. $\qquad$
15.) Location coordinated with local officials. $\qquad$
$\qquad$
16.) Other. $\qquad$

## SUMMARY

SR-115 in Blount County is a four-lane arterial roadway that serves an increasingly high volume of both commuter, local, and industrial vehicles each day. SR-115 serves as a connector not only from Maryville to Knoxville, but provides access to Interstate 40 which is critical to industry located in the region. The population of Alcoa and other areas within Blount County has continued to grow, increasing between a total of $6-7 \%$ from 2000 to 2005. A large portion of Alcoa still remains undeveloped though, with only $50 \%$ of Alcoa accounted for at the current time. As the City of Alcoa continues to grow and expand, local development will contribute more vehicles to the local transportation system.

The traffic analysis reveals that for the base year 2012, the intersection of SR-115 and Louisville Road operates at LOS E during A.M. and LOS F during P.M. times. Future year 2032 analysis indicates LOS F for both peak hour times. The high traffic volumes accounted for at this intersection also creates a high number of crashes and the crash rate for this intersection is above the Tennessee statewide average crash rate.

Improvements to the intersection of SR-115 and Louisville Road are necessary to achieve the following criteria:

- Address operational and safety concerns at this intersection and adjacent intersections along Louisville Road.

While relief is much needed, efforts at this time to widen the intersection or construct a grade separated interchange would be futile since no corridor improvements are planned or identified along SR-115. Therefore this particular improvement was not considered as an option.

Option 2 is not considered a viable option as analyses indicate this improvement fails. Therefore this option is not recommended.

Four options were considered in addressing the purpose and need of this project. The following is a summary of these options.

## Option 1

- Construct acceleration lanes along SR-115 for the movements of EB Louisville Road to SB SR-115 and WB Louisville Road to NB SR-115.
- Construct a deceleration lane along SR-115 for vehicle turning EB Louisville Road.
- Estimated Cost $=\$ 682,000$.


## Option 3

- Eliminate the left-turn movement for traffic traveling WB Louisville Road to Hunters Crossing. Construct a raised median to prevent left turns.
- Restripe Hunters Crossing for right-in/right-out operation.
- Estimated Cost $=\$ 55,000$.


## Option 4

- Create a double left-turn for vehicles at the intersection of SR-115/Louisville Road traveling WB Louisville Road to SB SR-115.
- Eliminate the left-turn movement for traffic traveling EB Louisville Road to Brenda Drive.
- Estimated Cost $=\$ 32,000$.


## Suggested Option

- Close Hunters Crossing intersection at Louisville Road.
- Close Brenda Drive intersection at Louisville Road.
- Close the Unnamed Business Entrance (adjacent to Cracker Barrel) at Louisville Road.


## Appendix A - Field Review Meeting Minutes

# DEPARTMENT OF TRANSPORTATION BUREAU OF PLANNING AND DEVELOPMENT PLANNING DIVISION 

FIELD REVIEW REPORT

| Region <br> 1 | County <br> Blount | Project No. <br> PIN \#107899.00 | Type of Report: <br> Transportation Planning Report (TPR) |  |
| :--- | :--- | :--- | :--- | :---: |
| Route No. \& Termini: | Date | Date of Inspection |  |  |
| SR-115(US 129)@Louisville Road | $4 / 13 / 07$ | $4 / 12 / 07$ |  |  |

## Inspection Made By:

Gary Webber - TDOT Planning
Mike Biggs - HMB Professional Engineers, Inc.
Chad Toles - HMB Professional Engineers, Inc.
Ken Sperry - HMB Professional Engineers, Inc.
Steve Hylton - TDOT Planning
Gillian Fischbach - Fischbach Transportation Group, Inc.
Mwafaq Mohammed - TDOT Environmental
Mike Conger - Knoxville TPO
Nathan Vatter - TDOT Reg. 1 Traffic
Chris Jenkins - TDOT Reg. Project Manager
Jeff Turner - TDOT Reg. 1 Design
Mike Russell - TDOT Reg. 1
Mark Parrish - TDOT Reg. 1 Design
Kenny Wiggins - City of Alcoa
Andrew Sonner - City of Alcoa
Leigh Ann Tribble - FHWA

## Written Comments Received From:

None

## General Comments:

1. Introductions to all parties attending, HMB explained and discussed the background and scope of project.
2. HMB explained that peak volumes for base year average daily traffic (ADT) is approximately over 40,000, and project ADT in year 2032 is almost 55,000.
3. Updated crash data reveals approximately 200 crashes at this intersection. Almost all were rear-end type crashes with very few injuries and no fatalities. It was asked if this project could be considered as a regional safety audit report (RSAR) rather than TPR due to the number of crashes and focusing just on the intersection. TDOT explained this project does not qualify since the high traffic volumes create a low crash ratio, and no serious injuries or fatalities have
occurred. Representatives from the City of Alcoa pointed out that the majority of crashes occurred in the right-hand turn lane, either from EB Louisville Rd to SB SR-115 or NB Louisville Rd to NB SR-115.
4. A question was asked if the project limits involved only the SR-115/Louisville Road intersection or if it contained any of the SR-115 corridors. HMB explained the projects limits included the signalized intersection near the Wal-Mart entrance, the intersection at Hunters Crossing (a frontage road for the Wal-Mart complex), the SR-115/Louisville Road intersection, and the unsignalized intersection for Brenda Drive/Louisville Road.
5. The City of Alcoa had previously requested to TDOT that the SR-115 corridor as a whole be studied in conjunction with this intersection. This project was to focus only on the intersections discussed in Item \#4.
6. A TDOT representative explained TDOT currently has a SIA project on-going immediately south that will address improvements to Middlesettlements Road @ SR-115.
7. The City of Alcoa has contracted with Kimley-Horn to study the signal phasing along Louisville Road and also for a few signals along SR-115. Design recommendations for synchronizing the timing have been implemented. Attendees who travel the area mentioned this has helped alleviate some congestion.
8. HMB explained because of the extremely large traffic volumes at the main intersection, a grade separation would best alleviate the congestion along SR-115. However, without addressing other issues along the corridor at the same time, a project of this magnitude would inconvenience the public without considering other long term corridor solutions.
9. Limited right-of-way within the immediate area of the main intersection prohibits any major improvements. Retail businesses and the local 911 call center are all built within close proximity of the existing right-of-way.
10. Long-range improvements along SR-115 are not identified in the long range plan for the Knoxville TPO at this time. The City mentioned it is working to have this placed in future plans.
11. It was agreed the best options for improvement would be to address short-term safety and congestion. HMB discussed with TDOT and the City various alternatives. Two of which would be to reduce crashes occurring in the right-turn lanes mentioned in Item \#3. Also, it was discussed to realign Hunters Crossing with the signalized intersection for the Wal-Mart entrance to alleviate the left turn movement from Louisville Road. And also address the left turn movement from Brenda Drive onto Louisville Road.
12. It was mentioned that projected traffic does include traffic estimated for the development under construction in the southeast quadrant of the intersection.
13. Level of Service was discussed for the intersection; currently functions at Level D.
14. The City mentioned updated aerial photography may be available for the area. Current aerial mapping obtained from TDOT does not show recent improvements within the study area. HMB had previously contacted the GIS coordinator for Blount County and will follow up to determine if updated mapping can be obtained.
15. The City provided HMB with a printout of how and where crashes occurred within the main intersection.
16. The review team discussed a variety of options for improvements concerning Brenda Drive and the business entrance (opposite Brenda Drive) along Louisville Road which will be fully discussed in the TPR as a result of this field review.
17. Observed an illegal U-turn on Louisville Road in front of Brenda Drive during review. Also observed several motorists ignoring pavement markings in turning areas.
18. Noticed improperly placed pavement markings currently denoted for the Louisville Road left-turn lane for Brenda Drive. Representatives from the City stated they would correct as soon as possible.

ApRIL 12,2007


## Appendix B - Traffic Report

## TENNESSEE DEPARTMENT OF TRANSPORTATION PROJECT PLANNING DIVISION

PROJECT NO.:
 ROUTE: S.R. 115 @ LOUISVILLE RD. COUNTY: BLOUNT CITY: ALCOA
PROJECT PIN NUMBER: PROJECT DESCRIPTION: INTERSECTION IMPROVEMENT @ S.R. 115 \& LOUISVILLE ROAD.
[1] S.R. 115 TRAFFIC DATA
[2] LOUISVILLE ROAD TRAFFIC DATA

## DIVISION REQUESTING:

MAINTENANCE
PLANNING
PROG. DEVELOPMENT \& ADM. PUBLIC TRANS. \& AERO.

PAVEMENT DESIGN STRUCTURES
SURVEY \& DESIGN
TRAFFIC SIGNAL DESIGN OTHER $\qquad$

YEAR PROJECT PROGRAMMED FOR CONSTRUCTION:
PROJECTED LETTING DATE: $\qquad$
TRAFFIC ASSIGNMENT:
[1]
[2]

| BASE YEAR |  | DESIGN YEAR |  |  |  |  | DESIGN ROADWAY $\%$ TRUCKS |  | DESIGN AVERAGE DAILYLOADS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AADT | YEAR | AADT | DHV | \% | YEAR | DIR.DIST. | DHV | AADT | FLEX | RIGID |
| 47,740 | 2012 | 54,300 | 4,764 | 9 | 2032 | 65-35 | 5 | 7 |  |  |
| 20,010 | 2012 | 24,010 | 1,761 | 7 | 2032 | 55-45 | 2 | 3 |  |  |
|  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |



## COMMENTS:

INCLUDES 4-8 HOUR TURNING MOVEMENT COUNTS DONE BY HMB ENGINEERING.
THIS TRAFFIC BASED ON 2006 CYCLE COUNTS AND THE 4-8 HOUR TURNING MOVEMENT COUNTS DONE BY HMB ENGINEERING DATED 2-20/22-07. THE FUTURE TRAFFIC IS BASED ON GROWTH RATE FROM THE KNOXVILLE MPO COMPUTER ASSIGNMENT MODEL.











Brenda Dr.


Southern Traffic Services, Inc.

2911 Westfield Rd
Louisville Rd @ Brenda Dr
Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-1 Louisville Rd @ Brenda Dr
Site Code : 70490001
Start Date : 2/22/2007
Page No : 1

Groups Printed- Automobiles - Trucks - Buses

|  | Brenda Dr Southbound |  |  |  | Louisville Rd Westbound |  |  |  | Cracker Barrel/Panera Bread <br> Northbound |  |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns |  |
| 07:00 | 0 | 0 | 2 | 0 | 4 | 94 | 4 | 0 | 3 | 1 | 0 | 0 | 16 | 71 | 6 | 0 | 201 |
| 07:15 | 0 | 0 | 4 | 0 | 2 | 118 | 5 | 0 | 3 | 0 | 5 | 0 | 8 | 85 | 11 | 0 | 241 |
| 07:30 | 0 | 0 | 8 | 0 | 3 | 132 | 5 | 0 | 6 | 0 | 4 | 0 | 7 | 118 | 18 | 0 | 301 |
| 07:45 | 0 | 1 | 7 | 0 | 5 | 171 | 6 | 0 | 3 | 0 | 4 | 0 | 10 | 97 | 16 | 0 | 320 |
| Total | 0 | 1 | 21 | 0 | 14 | 515 | 20 | 0 | 15 | 1 | 13 | 0 | 41 | 371 | 51 | 0 | 1063 |
| 08:00 | 0 | 0 | 5 | 0 | 6 | 181 | 4 | 0 | 5 | 0 | 4 | 0 | 10 | 122 | 16 | 0 | 353 |
| 08:15 | 0 | 0 | 6 | 0 | 7 | 131 | 2 | 0 | 12 | 0 | 2 | 0 | 7 | 117 | 13 | 0 | 297 |
| 08:30 | 0 | 0 | 7 | 0 | 8 | 99 | 4 | 0 | 7 | 0 | 4 | 0 | 8 | 132 | 18 | 0 | 287 |
| 08:45 | 1 | 0 | 6 | 0 | 7 | 123 | 1 | 0 | 8 | 0 | 5 | 0 | 17 | 122 | 26 | 0 | 316 |
| Total | 1 | 0 | 24 | 0 | 28 | 534 | 11 | 0 | 32 | 0 | 15 | 0 | 42 | 493 | 73 | 0 | 1253 |

** BREAK ***

| $11: 00$ | 0 | 0 | 16 | 0 | 13 | 157 | 1 | 0 | 6 | 0 | 6 | 0 | 9 | 144 | 33 | 1 | 386 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $11: 15$ | 0 | 0 | 6 | 0 | 3 | 163 | 2 | 0 | 13 | 0 | 11 | 0 | 16 | 127 | 40 | 1 | 382 |
| $11: 30$ | 0 | 0 | 14 | 0 | 10 | 196 | 4 | 0 | 6 | 17 | 10 | 0 | 13 | 113 | 33 | 0 | 416 |
| $11: 45$ | 0 | 1 | 8 | 0 | 17 | 192 | 2 | 0 | 8 | 0 | 11 | 0 | 9 | 150 | 36 | 0 | 434 |
| Total | 0 | 1 | 44 | 0 | 43 | 708 | 9 | 0 | 33 | 17 | 38 | 0 | 47 | 534 | 142 | 2 | 1618 |
| $12: 00$ | 0 | 0 | 10 | 0 | 6 | 195 | 5 | 0 | 15 | 0 | 14 | 0 | 7 | 145 | 37 | 0 | 434 |
| $12: 15$ | 0 | 0 | 5 | 0 | 10 | 195 | 2 | 0 | 16 | 0 | 21 | 0 | 10 | 175 | 37 | 0 | 471 |
| $12: 30$ | 0 | 0 | 7 | 0 | 13 | 226 | 1 | 0 | 29 | 0 | 18 | 0 | 17 | 156 | 41 | 1 | 509 |
| $12: 45$ | 1 | 1 | 11 | 0 | 5 | 178 | 8 | 0 | 14 | 0 | 14 | 0 | 18 | 166 | 37 | 0 | 453 |
| Total | 1 | 1 | 33 | 0 | 34 | 794 | 16 | 0 | 74 | 0 | 67 | 0 | 52 | 642 | 152 | 1 | 1867 |

*** BREAK ***

| $14: 00$ | 0 | 0 | 15 | 0 | 5 | 167 | 2 | 0 | 13 | 0 | 9 | 0 | 13 | 146 | 19 | 0 | 389 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $14: 15$ | 0 | 0 | 5 | 0 | 4 | 189 | 4 | 0 | 13 | 1 | 9 | 0 | 14 | 163 | 22 | 0 | 424 |
| $14: 30$ | 0 | 0 | 12 | 0 | 2 | 153 | 1 | 0 | 19 | 0 | 7 | 0 | 17 | 118 | 21 | 0 | 350 |
| $14: 45$ | 1 | 0 | 18 | 0 | 9 | 168 | 1 | 0 | 12 | 0 | 7 | 0 | 10 | 116 | 25 | 0 | 367 |
| Total | 1 | 0 | 50 | 0 | 20 | 677 | 8 | 0 | 57 | 1 | 32 | 0 | 54 | 543 | 87 | 0 | 1530 |


| $15: 00$ | 0 | 0 | 8 | 0 | 5 | 180 | 5 | 0 | 17 | 0 | 7 | 0 | 11 | 175 | 22 | 1 | 431 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $15: 15$ | 0 | 1 | 8 | 0 | 7 | 177 | 3 | 0 | 15 | 1 | 3 | 0 | 11 | 159 | 24 | 1 | 410 |
| $15: 30$ | 0 | 0 | 3 | 0 | 4 | 189 | 4 | 0 | 15 | 0 | 5 | 0 | 13 | 114 | 22 | 1 | 370 |
| $15: 45$ | 0 | 0 | 13 | 0 | 4 | 174 | 0 | 0 | 12 | 0 | 13 | 0 | 17 | 148 | 33 | 2 | 416 |
| Total | 0 | 1 | 32 | 0 | 20 | 720 | 12 | 0 | 59 | 1 | 28 | 0 | 52 | 596 | 101 | 5 | 1627 |


| $16: 00$ | 0 | 0 | 12 | 0 | 3 | 235 | 4 | 0 | 15 | 0 | 7 | 0 | 10 | 153 | 19 | 1 | 459 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $16: 15$ | 2 | 0 | 14 | 0 | 5 | 191 | 4 | 0 | 11 | 0 | 2 | 0 | 10 | 115 | 26 | 0 | 380 |
| $16: 30$ | 0 | 0 | 10 | 0 | 3 | 203 | 5 | 0 | 7 | 0 | 3 | 0 | 7 | 161 | 21 | 0 | 420 |
| $16: 45$ | 0 | 0 | 10 | 0 | 7 | 192 | 2 | 0 | 6 | 0 | 9 | 0 | 11 | 157 | 40 | 2 | 436 |
| Total | 2 | 0 | 46 | 0 | 18 | 821 | 15 | 0 | 39 | 0 | 21 | 0 | 38 | 586 | 106 | 3 | 1695 |


| 17:00 | 0 | 0 | 11 | 0 | 15 | 213 | 3 | 0 | 11 | 0 | 8 | 0 | 5 | 134 | 43 | 0 | 443 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:15 | 0 | 0 | 4 | 0 | 9 | 200 | 2 | 0 | 6 | 0 | 10 | 0 | 3 | 136 | 35 | 1 | 406 |
| 17:30 | 0 | 0 | 1 | 0 | 11 | 199 | 0 | 0 | 8 | 0 | 16 | 0 | 9 | 116 | 42 | 0 | 402 |
| 17:45 | 0 | 0 | 4 | 0 | 7 | 161 | 3 | 0 | 18 | 1 | 10 | 0 | 11 | 142 | 40 | 0 | 397 |
| Total | 0 | 0 | 20 | 0 | 42 | 773 | 8 | 0 | 43 | 1 | 44 | 0 | 28 | 528 | 160 | 1 | 1648 |
| Grand Total | 5 | 4 | 270 | 0 | 219 | 5542 | 99 | 0 | 352 | 21 | 258 | 0 | 354 | 4293 | 872 | 12 | 12301 |
| Apprch \% | 1.8 | 1.4 | 96.8 | 0.0 | 3.7 | 94.6 | 1.7 | 0.0 | 55.8 | 3.3 | 40.9 | 0.0 | 6.4 | 77.6 | 15.8 | 0.2 |  |
| Total \% | 0.0 | 0.0 | 2.2 | 0.0 | 1.8 | 45.1 | 0.8 | 0.0 | 2.9 | 0.2 | 2.1 | 0.0 | 2.9 | 34.9 | 7.1 | 0.1 |  |

Louisville Rd @ Brenda Dr Blount County, TN

Gulf Breeze, FL 32563 1-800-786-3374

File Name : 7049-1 Louisville Rd @ Brenda Dr Site Code : 70490001
Start Date: 2/22/2007
Page No : 2


Louisville Rd @ Brenda Dr Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-1 Louisville Rd @ Brenda Dr
Site Code : 70490001
Start Date : 2/22/2007
Page No : 3

|  | Brenda Dr Southbound |  |  |  |  | Louisville Rd Westbound |  |  |  |  | Cracker Barrel/Panera BreadNorthbound |  |  |  |  | Louisville Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Left | $\begin{array}{r} \text { Thr } \\ u \end{array}$ | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{aligned} & \text { Utr } \\ & \text { ns } \\ & \hline \end{aligned}$ | App. <br> Total | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |

Peak Hour From 07:00 to 09:45-Peak 1 of 1

| Intersecti on Volume | 07:30 | 1 | 26 | 0 | 27 | 21 | 615 | 17 | 0 | 653 | 26 | 0 | 14 | 0 | 40 | 34 | 454 | 63 | 0 | 551 | 1271 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Percent | 0.0 | 3.7 | $96 .$ $3$ | 0.0 |  | 3.2 | 94. | 2.6 | 0.0 |  | 65. | 0.0 | 35. | 0.0 |  | 6.2 | 82. | 11. | 0.0 |  |  |
| 08:00 Volume Peak Factor | 0 | 0 | 5 | 0 | 5 | 6 | 181 | 4 | 0 | 191 | 5 | 0 | 4 | 0 | 9 | 10 | 122 | 16 | 0 | 148 | $\begin{aligned} & 353 \\ & 0.900 \end{aligned}$ |
| High Int. | 07:30 |  |  |  |  | 08:00 |  |  |  |  | 08:15 |  |  |  |  | 08:00 |  |  |  |  |  |
| Volume | 0 | 0 | 8 | 0 | 8 | 6 | 181 | 4 | 0 | 191 | 12 | 0 | 2 | 0 | 14 | 10 | 122 | 16 | 0 | 148 |  |
| Peak |  |  |  |  | 0.84 |  |  |  |  | 0.85 |  |  |  |  | 0.71 4 |  |  |  |  | 0.93 |  |



Louisville Rd @ Brenda Dr Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-1 Louisville Rd @ Brenda Dr
Site Code : 70490001
Start Date: 2/22/2007
Page No : 4

|  | Brenda Dr Southbound |  |  |  |  | Louisville Rd Westbound |  |  |  |  | Cracker Barrel/Panera Bread Northbound |  |  |  |  | Louisville Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thr u | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \mathrm{ns} \end{gathered}$ | App. Total | Left | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{gathered} \text { Thr } \\ \mathrm{u} \end{gathered}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | $\begin{gathered} \text { Int. } \\ \text { Total } \end{gathered}$ |
| Peak Hour From 10:00 to 13:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersecti on | 12:00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 1 | 1 | 33 | 0 | 35 | 34 | 794 | 16 | 0 | 844 | 74 | 0 | 67 | 0 | 141 | 52 | 642 | 152 | 1 | 847 | 1867 |
| Percent | 2.9 | 2.9 | $\begin{array}{r} 94 . \\ 3 \end{array}$ | 0.0 |  | 4.0 | 94. | 1.9 | 0.0 |  | $\begin{array}{r} 52 . \\ 5 \end{array}$ | 0.0 | $\begin{array}{r} 47 . \\ 5 \end{array}$ | 0.0 |  | 6.1 | $\begin{array}{r} 75 . \\ 8 \end{array}$ | $\begin{array}{r} 17 . \\ 9 \end{array}$ | 0.1 |  |  |
| 12:30 <br> Volume | 0 | 0 | 7 | 0 | 7 | 13 | 226 | 1 | 0 | 240 | 29 | 0 | 18 | 0 | 47 | 17 | 156 | 41 | 1 | 215 | 509 |
| Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 0.917 |
| Factor |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| High Int. | 12:45 |  |  |  |  | 12:30 |  |  |  |  | 12:30 |  |  |  |  | 12:15 |  |  |  |  |  |
| Volume | 1 | 1 | 11 | 0 | 13 | 13 | 226 | 1 | 0 | 240 | 29 | 0 | 18 | 0 | 47 | 10 | 175 | 37 | 0 | 222 |  |
| Peak |  |  |  |  | 0.67 |  |  |  |  | 0.87 |  |  |  |  | 0.75 |  |  |  |  | 0.95 |  |
| Factor |  |  |  |  | 3 |  |  |  |  | 9 |  |  |  |  | 0 |  |  |  |  | 4 |  |



Southern Traffic Services, Inc.

Louisville Rd @ Brenda Dr Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-1 Louisville Rd @ Brenda Dr
Site Code : 70490001
Start Date: 2/22/2007
Page No : 5

|  | Brenda Dr Southbound |  |  |  |  | Louisville Rd Westbound |  |  |  |  | Cracker Barrel/Panera Bread Northbound |  |  |  |  | Louisville Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | Thr u | $\begin{array}{r} \text { Rig } \\ \text { ht } \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{gathered} \text { Thr } \\ \mathrm{u} \end{gathered}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Peak Hour From 14:00 to 17:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersecti on | 16:30 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 0 | 0 | 35 | 0 | 35 | 34 | 808 | 12 | 0 | 854 | 30 | 0 | 30 | 0 | 60 | 26 | 588 | 139 | 3 | 756 | 1705 |
| Percent | 0.0 | 0.0 | 100 .0 | 0.0 |  | 4.0 | 94. 6 | 1.4 | 0.0 |  | 50. | 0.0 | 50. | 0.0 |  | 3.4 | $\begin{array}{r} 77 . \\ 8 \end{array}$ | 18. | 0.4 |  |  |
| 17:00 <br> Volume | 0 | 0 | 11 | 0 | 11 | 15 | 213 | 3 | 0 | 231 | 11 | 0 | 8 | 0 | 19 | 5 | 134 | 43 | 0 | 182 | 443 |
| Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 0.962 |
| Factor |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| High Int. | 17:00 |  |  |  |  | 17:00 |  |  |  |  | 17:00 |  |  |  |  | 16:45 |  |  |  |  |  |
| Volume | 0 | 0 | 11 | 0 | 11 | 15 | 213 | 3 | 0 | 231 | 11 | 0 | 8 | 0 | 19 | 11 | 157 | 40 | 2 | 210 |  |
| Peak |  |  |  |  | 0.79 |  |  |  |  | 0.92 |  |  |  |  | 0.78 |  |  |  |  | 0.90 |  |
| Factor |  |  |  |  | 5 |  |  |  |  | 4 |  |  |  |  | 9 |  |  |  |  | 0 |  |




Southern Traffic Services, Inc.
2911 Westfield Rd

SR 115 @ Louisville Rd
Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-2 SR 115 @ Louisville Rd
Site Code : 40790002
Start Date : 2/20/2007
Page No : 1

Groups Printed- Automobiles - Trucks - Buses

|  | $\begin{gathered} \text { SR } 115 \\ \text { Southbound } \end{gathered}$ |  |  |  | Louisville Rd Westbound |  |  |  | SR 115 <br> Northbound |  |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | $\begin{aligned} & \text { Int. } \\ & \text { Total } \end{aligned}$ |
| 07:00 | 28 | 152 | 9 | 0 | 4 | 35 | 33 | 0 | 16 | 389 | 21 | 0 | 40 | 51 | 18 | 0 | 796 |
| 07:15 | 40 | 208 | 17 | 0 | 3 | 31 | 74 | 0 | 32 | 418 | 7 | 0 | 56 | 62 | 27 | 0 | 975 |
| 07:30 | 32 | 233 | 20 | 1 | 7 | 56 | 118 | 0 | 36 | 396 | 10 | 0 | 72 | 74 | 25 | 0 | 1080 |
| 07:45 | 48 | 297 | 20 | 0 | 16 | 66 | 87 | 0 | 41 | 463 | 11 | 0 | 79 | 70 | 41 | 0 | 1239 |
| Total | 148 | 890 | 66 | 1 | 30 | 188 | 312 | 0 | 125 | 1666 | 49 | 0 | 247 | 257 | 111 | 0 | 4090 |
| 08:00 | 58 | 210 | 18 | 0 | 13 | 76 | 67 | 0 | 37 | 383 | 14 | 0 | 51 | 107 | 34 | 0 | 1068 |
| 08:15 | 45 | 179 | 16 | 0 | 17 | 50 | 45 | 0 | 33 | 424 | 22 | 0 | 35 | 58 | 29 | 0 | 953 |
| 08:30 | 27 | 165 | 20 | 0 | 22 | 71 | 52 | 0 | 37 | 324 | 18 | 0 | 47 | 74 | 28 | 0 | 885 |
| 08:45 | 40 | 182 | 18 | 0 | 17 | 66 | 42 | 0 | 33 | 296 | 23 | 0 | 41 | 68 | 40 | 0 | 866 |
| Total | 170 | 736 | 72 | 0 | 69 | 263 | 206 | 0 | 140 | 1427 | 77 | 0 | 174 | 307 | 131 | 0 | 3772 |

*** BREAK ***

| $11: 00$ | 43 | 208 | 39 | 0 | 47 | 100 | 51 | 0 | 63 | 179 | 29 | 0 | 46 | 81 | 37 | 0 | 923 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $11: 15$ | 37 | 166 | 38 | 1 | 45 | 99 | 53 | 0 | 71 | 170 | 26 | 0 | 47 | 120 | 51 | 0 | 924 |
| $11: 30$ | 51 | 215 | 25 | 0 | 42 | 102 | 64 | 0 | 93 | 183 | 43 | 0 | 48 | 100 | 46 | 0 | 1012 |
| $11: 45$ | 44 | 216 | 28 | 0 | 51 | 119 | 51 | 0 | 58 | 194 | 53 | 0 | 57 | 112 | 68 | 0 | 1051 |
| Total | 175 | 805 | 130 | 1 | 185 | 420 | 219 | 0 | 285 | 726 | 151 | 0 | 198 | 413 | 202 | 0 | 3910 |
| $12: 00$ | 55 | 188 | 38 | 2 | 42 | 126 | 48 | 0 | 53 | 199 | 52 | 0 | 52 | 103 | 55 | 0 | 1013 |
| $12: 15$ | 52 | 203 | 34 | 0 | 52 | 116 | 45 | 0 | 78 | 219 | 29 | 0 | 48 | 113 | 50 | 0 | 1039 |
| $12: 30$ | 51 | 172 | 35 | 0 | 64 | 131 | 64 | 0 | 70 | 203 | 44 | 0 | 53 | 117 | 64 | 0 | 1068 |
| $12: 45$ | 40 | 214 | 24 | 1 | 42 | 98 | 49 | 0 | 55 | 234 | 31 | 0 | 59 | 119 | 49 | 0 | 1015 |
| Total | 198 | 777 | 131 | 3 | 200 | 471 | 206 | 0 | 256 | 855 | 156 | 0 | 212 | 452 | 218 | 0 | 4135 |

*** BREAK ***

| 14:00 | 50 | 256 | 42 | 0 | 51 | 103 | 48 | 0 | 66 | 180 | 24 | 0 | 49 | 136 | 46 | 0 | 1051 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 14:15 | 32 | 237 | 27 | 0 | 46 | 95 | 67 | 0 | 54 | 244 | 25 | 0 | 55 | 115 | 61 | 0 | 1058 |
| 14:30 | 47 | 293 | 38 | 0 | 40 | 114 | 53 | 0 | 63 | 199 | 29 | 0 | 48 | 101 | 69 | 0 | 1094 |
| 14:45 | 30 | 234 | 35 | 0 | 44 | 107 | 40 | 0 | 50 | 205 | 24 | 0 | 46 | 98 | 66 | 0 | 979 |
| Total | 159 | 1020 | 142 | 0 | 181 | 419 | 208 | 0 | 233 | 828 | 102 | 0 | 198 | 450 | 242 | 0 | 4182 |
| 15:00 | 43 | 241 | 40 | 0 | 35 | 132 | 37 | 0 | 71 | 203 | 32 | 0 | 41 | 118 | 62 | 0 | 1055 |
| 15:15 | 45 | 246 | 39 | 1 | 41 | 111 | 55 | 0 | 59 | 229 | 16 | 0 | 73 | 128 | 63 | 0 | 1106 |
| 15:30 | 59 | 266 | 45 | 0 | 55 | 119 | 82 | 0 | 71 | 230 | 17 | 0 | 84 | 114 | 55 | 0 | 1197 |
| 15:45 | 54 | 297 | 32 | 0 | 43 | 110 | 43 | 0 | 88 | 288 | 21 | 0 | 93 | 112 | 47 | 0 | 1228 |
| Total | 201 | 1050 | 156 | 1 | 174 | 472 | 217 | 0 | 289 | 950 | 86 | 0 | 291 | 472 | 227 | 0 | 4586 |
| 16:00 | 47 | 284 | 47 | 0 | 36 | 137 | 43 | 0 | 93 | 251 | 37 | 0 | 59 | 132 | 63 | 0 | 1229 |
| 16:15 | 48 | 295 | 73 | 0 | 49 | 147 | 68 | 0 | 59 | 218 | 24 | 0 | 65 | 137 | 77 | 0 | 1260 |
| 16:30 | 54 | 318 | 44 | 0 | 43 | 100 | 49 | 0 | 56 | 276 | 20 | 0 | 75 | 95 | 96 | 0 | 1226 |
| 16:45 | 26 | 344 | 53 | 0 | 45 | 133 | 62 | 0 | 48 | 241 | 30 | 0 | 66 | 98 | 74 | 0 | 1220 |
| Total | 175 | 1241 | 217 | 0 | 173 | 517 | 222 | 0 | 256 | 986 | 111 | 0 | 265 | 462 | 310 | 0 | 4935 |
| 17:00 | 61 | 344 | 43 | 0 | 45 | 128 | 53 | 0 | 74 | 276 | 16 | 0 | 66 | 144 | 78 | 0 | 1328 |
| 17:15 | 43 | 362 | 32 | 0 | 40 | 123 | 63 | 0 | 70 | 292 | 20 | 0 | 85 | 109 | 68 | 0 | 1307 |
| 17:30 | 44 | 383 | 54 | 0 | 38 | 130 | 53 | 0 | 68 | 272 | 25 | 0 | 59 | 94 | 70 | 0 | 1290 |
| 17:45 | 45 | 350 | 40 | 0 | 36 | 137 | 47 | 0 | 66 | 226 | 35 | 0 | 57 | 102 | 76 | 0 | 1217 |
| Total | 193 | 1439 | 169 | 0 | 159 | 518 | 216 | 0 | 278 | 1066 | 96 | 0 | 267 | 449 | 292 | 0 | 5142 |
| Grand Total | 1419 | 7958 | 1083 | 6 | 1171 | 3268 | 1806 | 0 | 1862 | 8504 | 828 | 0 | 1852 | 3262 | 1733 | 0 | 34752 |
| Apprch \% | 13.6 | 76.0 | 10.3 | 0.1 | 18.8 | 52.3 | 28.9 | 0.0 | 16.6 | 76.0 | 7.4 | 0.0 | 27.0 | 47.6 | 25.3 | 0.0 |  |
| Total \% | 4.1 | 22.9 | 3.1 | 0.0 | 3.4 | 9.4 | 5.2 | 0.0 | 5.4 | 24.5 | 2.4 | 0.0 | 5.3 | 9.4 | 5.0 | 0.0 |  |

SR 115 @ Louisville Rd Blount County, TN

Gulf Breeze, FL 32563 1-800-786-3374

File Name : 7049-2 SR 115 @ Louisville Rd
Site Code : 40790002
Start Date : 2/20/2007
Page No : 2


SR 115 @ Louisville Rd Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-2 SR 115 @ Louisville Rd
Site Code : 40790002
Start Date : 2/20/2007
Page No : 3


|  |  |  |
| :---: | :---: | :---: |
|  | 2/20/2007 7:15:00 AM <br> 2/20/2007 8:00:00 AM <br> Automobiles <br> Trucks <br> Buses |  |
|  |  |  |

SR 115 @ Louisville Rd Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-2 SR 115 @ Louisville Rd
Site Code : 40790002
Start Date : 2/20/2007
Page No : 4



SR 115 @ Louisville Rd Blount County, TN

Gulf Breeze, FL 32563
1-800-786-3374

File Name : 7049-2 SR 115 @ Louisville Rd
Site Code : 40790002 Start Date : 2/20/2007
Page No : 5




Southern Traffic Services, Inc.
2911 Westfield Rd
Louisville Rd @ Hunters Crossing
Gulf Breeze, FL 3256ße Name : 7049-3 LOUISVILLE RD @ HUNTERS CROSSING 1-800-786-3374 Site Code : 70490003

Start Date : 2/22/2007
Page No : 1
Groups Printed- Automobiles - Trucks - Buses

|  | Louisville Rd Westbound |  |  | Hunters Crossing Northbound |  | Louisville Rd Eastbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | Right | Peds | Thru | Right | Peds | Int. Total |
| 07:00 | 34 | 47 | 0 | 52 | 0 | 56 | 10 | 0 | 199 |
| 07:15 | 25 | 57 | 0 | 52 | 0 | 77 | 6 | 0 | 217 |
| 07:30 | 40 | 84 | 0 | 77 | 0 | 94 | 5 | 0 | 300 |
| 07:45 | 35 | 111 | 0 | 89 | 0 | 96 | 12 | 0 | 343 |
| Total | 134 | 299 | 0 | 270 | 0 | 323 | 33 | 0 | 1059 |
| 08:00 | 54 | 81 | 0 | 57 | 0 | 95 | 14 | 0 | 301 |
| 08:15 | 48 | 74 | 0 | 72 | 0 | 72 | 5 | 0 | 271 |
| 08:30 | 39 | 68 | 0 | 52 | 0 | 78 | 10 | 0 | 247 |
| 08:45 | 70 | 67 | 0 | 62 | 0 | 145 | 13 | 0 | 357 |
| Total | 211 | 290 | 0 | 243 | 0 | 390 | 42 | 0 | 1176 |

*** BREAK ***

| $11: 00$ | 80 | 89 | 0 | 72 | 0 | 96 | 11 | 0 | 348 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $11: 15$ | 99 | 92 | 0 | 93 | 0 | 91 | 11 | 0 | 386 |
| $11: 30$ | 80 | 123 | 0 | 95 | 0 | 89 | 8 | 0 | 395 |
| $11: 45$ | 106 | 119 | 0 | 127 | 0 | 100 | 15 | 0 | 467 |
| Total | 365 | 423 | 0 | 387 | 0 | 376 | 45 | 0 | 1596 |
|  |  |  |  |  |  |  |  |  |  |
| $12: 00$ | 106 | 112 | 0 | 106 | 0 | 101 | 20 | 0 | 445 |
| $12: 15$ | 123 | 147 | 0 | 128 | 0 | 89 | 14 | 0 | 501 |
| $12: 30$ | 102 | 128 | 0 | 146 | 0 | 104 | 18 | 0 | 498 |
| $12: 45$ | 119 | 133 | 0 | 149 | 0 | 92 | 17 | 0 | 510 |
| Total | 450 | 520 | 0 | 529 | 0 | 386 | 69 | 0 | 1954 |

*** BREAK ***

| 14:00 | 77 | 126 | 0 | 130 | 0 | 122 | 15 | 0 | 470 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 14:15 | 91 | 113 | 0 | 94 | 0 | 108 | 10 | 0 | 416 |
| 14:30 | 78 | 124 | 0 | 112 | 0 | 121 | 9 | 0 | 444 |
| 14:45 | 64 | 113 | 0 | 95 | 0 | 124 | 16 | 0 | 412 |
| Total | 310 | 476 | 0 | 431 | 0 | 475 | 50 | 0 | 1742 |
| 15:00 | 69 | 121 | 0 | 101 | 0 | 84 | 18 | 0 | 393 |
| 15:15 | 77 | 139 | 0 | 115 | 0 | 98 | 15 | 0 | 444 |
| 15:30 | 62 | 121 | 0 | 104 | 0 | 84 | 9 | 0 | 380 |
| 15:45 | 82 | 159 | 0 | 111 | 0 | 101 | 10 | 0 | 463 |
| Total | 290 | 540 | 0 | 431 | 0 | 367 | 52 | 0 | 1680 |
| 16:00 | 126 | 127 | 0 | 87 | 0 | 96 | 11 | 0 | 447 |
| 16:15 | 77 | 153 | 0 | 114 | 0 | 113 | 13 | 0 | 470 |
| 16:30 | 83 | 177 | 0 | 122 | 0 | 124 | 11 | 0 | 517 |
| 16:45 | 95 | 168 | 0 | 118 | 0 | 140 | 11 | 0 | 532 |
| Total | 381 | 625 | 0 | 441 | 0 | 473 | 46 | 0 | 1966 |
| 17:00 | 104 | 189 | 0 | 174 | 0 | 146 | 14 | 0 | 627 |
| 17:15 | 145 | 215 | 0 | 184 | 0 | 144 | 18 | 0 | 706 |
| 17:30 | 140 | 252 | 0 | 196 | 0 | 102 | 22 | 0 | 712 |
| 17:45 | 128 | 291 | 0 | 158 | 0 | 118 | 21 | 0 | 716 |
| Total | 517 | 947 | 0 | 712 | 0 | 510 | 75 | 0 | 2761 |
| Grand Total | 2658 | 4120 | 0 | 3444 | 0 | 3300 | 412 | 0 | 13934 |
| Apprch \% | 39.2 | 60.8 | 0.0 | 100.0 | 0.0 | 88.9 | 11.1 | 0.0 |  |
| Total \% | 19.1 | 29.6 | 0.0 | 24.7 | 0.0 | 23.7 | 3.0 | 0.0 |  |

Louisville Rd @ Hunters Crossing
Blount County, TN

Gulf Breeze, FL 3256Be Name : 7049-3 LOUISVILLE RD @ HUNTERS CROSSING 1-800-786-3374 Site Code : 70490003 Start Date : 2/22/2007 Page No : 2


## Southern Traffic Services, Inc.

Louisville Rd @ Hunters Crossing
Blount County, TN

Gulf Breeze, FL 3256Be Name : 7049-3 LOUISVILLE RD @ HUNTERS CROSSING 1-800-786-3374 Site Code : 70490003 Start Date : 2/22/2007 Page No : 3

|  | Louisville Rd Westbound |  |  |  | Hunters Crossing Northbound |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | App. Total | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour From 07:00 to 09:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection | 07:30 |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 177 | 350 | 0 | 527 | 295 | 0 | 295 | 357 | 36 | 0 | 393 | 1215 |
| Percent | 33.6 | 66.4 | 0.0 |  | 100.0 | 0.0 |  | 90.8 | 9.2 | 0.0 |  |  |
| 07:45 Volume | 35 | 111 | 0 | 146 | 89 | 0 | 89 | 96 | 12 | 0 | 108 | 343 |
| Peak Factor |  |  |  |  |  |  |  |  |  |  |  | 0.886 |
| High Int. | 07:45 |  |  |  | 07:45 |  |  | 08:00 |  |  |  |  |
| Volume | 35 | 111 | 0 | 146 | 89 | 0 | 89 | 95 | 14 | 0 | 109 |  |
| Peak Factor |  |  |  | 0.902 |  |  | 0.829 |  |  |  | 0.901 |  |



## Southern Traffic Services, Inc.

Louisville Rd @ Hunters Crossing Blount County, TN

Gulf Breeze, FL 3256Be Name : 7049-3 LOUISVILLE RD @ HUNTERS CROSSING 1-800-786-3374 Site Code : 70490003 Start Date : 2/22/2007 Page No : 4

|  | Louisville Rd Westbound |  |  |  | Hunters Crossing Northbound |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | App. Total | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour From 10:00 to 13:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection | 12:00 |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 450 | 520 | 0 | 970 | 529 | 0 | 529 | 386 | 69 | 0 | 455 | 1954 |
| Percent | 46.4 | 53.6 | 0.0 |  | 100.0 | 0.0 |  | 84.8 | 15.2 | 0.0 |  |  |
| 12:45 Volume | 119 | 133 | 0 | 252 | 149 | 0 | 149 | 92 | 17 | 0 | 109 | 510 |
| Peak Factor |  |  |  |  |  |  |  |  |  |  |  | 0.958 |
| High Int. | 12:15 |  |  |  | 12:45 |  |  | 12:30 |  |  |  |  |
| Volume | 123 | 147 | 0 | 270 | 149 | 0 | 149 | 104 | 18 | 0 | 122 |  |
| Peak Factor |  |  |  | 0.898 |  |  | 0.888 |  |  |  | 0.932 |  |



Louisville Rd @ Hunters Crossing Blount County, TN

Gulf Breeze, FL 3256Be Name : 7049-3 LOUISVILLE RD @ HUNTERS CROSSING 1-800-786-3374 Site Code : 70490003 Start Date : 2/22/2007 Page No : 5

|  | Louisville Rd Westbound |  |  |  | Hunters Crossing Northbound |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | App. Total | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour From 14:00 to 17:45-Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection | 17:00 |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 517 | 947 | 0 | 1464 | 712 | 0 | 712 | 510 | 75 | 0 | 585 | 2761 |
| Percent | 35.3 | 64.7 | 0.0 |  | 100.0 | 0.0 |  | 87.2 | 12.8 | 0.0 |  |  |
| 17:45 Volume | 128 | 291 | 0 | 419 | 158 | 0 | 158 | 118 | 21 | 0 | 139 | 716 |
| Peak Factor |  |  |  |  |  |  |  |  |  |  |  | 0.964 |
| High Int. | 17:45 |  |  |  | 17:30 |  |  | 17:15 |  |  |  |  |
| Volume | 128 | 291 | 0 | 419 | 196 | 0 | 196 | 144 | 18 | 0 | 162 |  |
| Peak Factor |  |  |  | 0.874 |  |  | 0.908 |  |  |  | 0.903 |  |



To Catholic Church


Southern Traffic Services, Inc.
2911 Westfield Rd

Louisville Rd @ Catholic Church/Walmart Blount County, TN

Gulf Breeze, FL 32563 File Name : 7049-4 Louisville @ Catholic Church-Walmart 1-800-786-3374 Site Code : 70490004

Start Date : 2/21/2007
Page No : 1

Groups Printed- Automobiles - Trucks - Buses

|  | Catholic Church Southbound |  |  |  | Louisville Rd Westbound |  |  |  | Walmart Northbound |  |  |  | Louisville Rd Eastbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | Left | Thru | Right | Utrns | $\begin{gathered} \text { Int. } \\ \text { Total } \end{gathered}$ |
| 07:00 | 1 | 0 | 0 | 0 | 16 | 31 | 0 | 1 | 14 | 1 | 12 | 0 | 0 | 48 | 2 | 0 | 126 |
| 07:15 | 0 | 0 | 0 | 0 | 12 | 31 | 5 | 1 | 12 | 2 | 12 | 0 | 2 | 77 | 4 | 0 | 158 |
| 07:30 | 0 | 0 | 0 | 0 | 25 | 39 | 14 | 0 | 6 | 0 | 12 | 0 | 1 | 85 | 7 | 0 | 189 |
| 07:45 | 1 | 0 | 0 | 0 | 29 | 34 | 51 | 1 | 8 | 11 | 8 | 0 | 10 | 98 | 7 | 0 | 258 |
| Total | 2 | 0 | 0 | 0 | 82 | 135 | 70 | 3 | 40 | 14 | 44 | 0 | 13 | 308 | 20 | 0 | 731 |
| 08:00 | 0 | 0 | 0 | 0 | 19 | 29 | 26 | 0 | 7 | 4 | 15 | 0 | 6 | 94 | 10 | 0 | 210 |
| 08:15 | 3 | 0 | 1 | 0 | 24 | 44 | 7 | 1 | 11 | 0 | 14 | 0 | 0 | 60 | 11 | 0 | 176 |
| 08:30 | 2 | 0 | 0 | 0 | 29 | 35 | 3 | 0 | 14 | 1 | 9 | 0 | 0 | 74 | 8 | 0 | 175 |
| 08:45 | 5 | 0 | 7 | 0 | 29 | 34 | 2 | 0 | 11 | 0 | 22 | 0 | 1 | 80 | 7 | 0 | 198 |
| Total | 10 | 0 | 8 | 0 | 101 | 142 | 38 | 1 | 43 | 5 | 60 | 0 | 7 | 308 | 36 | 0 | 759 |

*** BREAK ***

| $11: 00$ | 2 | 0 | 1 | 0 | 36 | 43 | 2 | 0 | 15 | 0 | 23 | 0 | 2 | 75 | 14 | 0 | 213 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $11: 15$ | 1 | 3 | 0 | 0 | 41 | 40 | 4 | 1 | 16 | 0 | 33 | 0 | 0 | 70 | 10 | 0 | 219 |
| $11: 30$ | 5 | 0 | 0 | 0 | 52 | 57 | 3 | 1 | 21 | 0 | 31 | 0 | 1 | 60 | 6 | 0 | 237 |
| $11: 45$ | 3 | 0 | 0 | 0 | 51 | 59 | 5 | 1 | 23 | 1 | 40 | 0 | 1 | 76 | 20 | 0 | 280 |
| Total | 11 | 3 | 1 | 0 | 180 | 199 | 14 | 3 | 75 | 1 | 127 | 0 | 4 | 281 | 50 | 0 | 949 |
| $12: 00$ | 8 | 1 | 0 | 0 | 58 | 55 | 1 | 2 | 16 | 0 | 29 | 0 | 1 | 80 | 23 | 0 | 274 |
| $12: 15$ | 3 | 0 | 1 | 0 | 74 | 71 | 1 | 0 | 30 | 1 | 34 | 0 | 0 | 66 | 19 | 0 | 300 |
| $12: 30$ | 1 | 1 | 1 | 0 | 67 | 52 | 4 | 1 | 38 | 1 | 36 | 0 | 0 | 87 | 13 | 0 | 302 |
| $12: 45$ | 5 | 1 | 1 | 0 | 67 | 56 | 3 | 0 | 25 | 1 | 31 | 0 | 1 | 71 | 11 | 0 | 273 |
| Total | 17 | 3 | 3 | 0 | 266 | 234 | 9 | 3 | 109 | 3 | 130 | 0 | 2 | 304 | 66 | 0 | 1149 |

*** BREAK ***

| 14:00 | 7 | 2 | 1 | 0 | 56 | 68 | 2 | 2 | 23 | 0 | 38 | 0 | 0 | 79 | 20 | 0 | 298 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 14:15 | 1 | 0 | 0 | 0 | 33 | 61 | 0 | 2 | 27 | 0 | 32 | 0 | 0 | 73 | 16 | 0 | 245 |
| 14:30 | 5 | 0 | 0 | 0 | 55 | 69 | 1 | 0 | 22 | 0 | 40 | 0 | 1 | 85 | 14 | 0 | 292 |
| 14:45 | 3 | 0 | 2 | 0 | 52 | 57 | 3 | 2 | 25 | 1 | 40 | 0 | 1 | 93 | 9 | 0 | 288 |
| Total | 16 | 2 | 3 | 0 | 196 | 255 | 6 | 6 | 97 | 1 | 150 | 0 | 2 | 330 | 59 | 0 | 1123 |
| 15:00 | 0 | 0 | 0 | 0 | 36 | 80 | 1 | 0 | 27 | 1 | 24 | 0 | 0 | 76 | 10 | 0 | 255 |
| 15:15 | 7 | 0 | 1 | 0 | 45 | 72 |  | 0 | 30 | 0 | 38 | 0 | 1 | 65 | 14 | 0 | 274 |
| 15:30 | 1 | 0 | 0 | 0 | 42 | 59 | 0 | 1 | 20 | 0 | 27 | 0 | 0 | 66 | 13 | 0 | 229 |
| 15:45 | 3 | 0 | 1 | 0 | 46 | 94 | 2 | 3 | 27 | 0 | 28 | 0 | 1 | 71 | 18 | 0 | 294 |
| Total | 11 | 0 | 2 | 0 | 169 | 305 | 4 | 4 | 104 | 1 | 117 | 0 | 2 | 278 | 55 | 0 | 1052 |
| 16:00 | 8 | 0 | 1 | 0 | 44 | 80 | 0 | 2 | 27 | 1 | 27 | 0 | 0 | 65 | 10 | 0 | 265 |
| 16:15 | 5 | 0 | 1 | 0 | 65 | 78 | 2 | 1 | 18 | 0 | 40 | 0 | 0 | 73 | 19 | 0 | 302 |
| 16:30 | 7 | 0 | 0 | 0 | 53 | 91 | 3 | 0 | 27 | 1 | 29 | 0 | 1 | 88 | 20 | 0 | 320 |
| 16:45 | 5 | 0 | 0 | 0 | 64 | 93 | 2 | 0 | 27 | 1 | 43 | 0 | 1 | 97 | 18 | 0 | 351 |
| Total | 25 | 0 | 2 | 0 | 226 | 342 | 7 | 3 | 99 | 3 | 139 | 0 | 2 | 323 | 67 | 0 | 1238 |
| 17:00 | 6 | 0 | 0 | 0 | 60 | 94 | 5 | 0 | 32 | 1 | 44 | 0 | 1 | 105 | 17 | 0 | 365 |
| 17:15 | 0 | 0 | 0 | 0 | 41 | 95 | 11 | 2 | 19 | 1 | 45 | 0 | 5 | 109 | 19 | 0 | 347 |
| 17:30 | 0 | 1 | 2 | 0 | 45 | 94 | 37 | 2 | 25 | 7 | 24 | 0 | 5 | 91 | 9 | 0 | 342 |
| 17:45 | 2 | 2 | 0 | 0 | 44 | 68 | 122 | 0 | 39 | 7 | 23 | 0 | 20 | 100 | 21 | 0 | 448 |
| Total | 8 | 3 | 2 | 0 | 190 | 351 | 175 | 4 | 115 | 16 | 136 | 0 | 31 | 405 | 66 | 0 | 1502 |
| Grand Total | 100 | 11 | 21 | 0 | 1410 | 1963 | 323 | 27 | 682 | 44 | 903 | 0 | 63 | 2537 | 419 | 0 | 8503 |
| Apprch \% | 75.8 | 8.3 | 15.9 | 0.0 | 37.9 | 52.7 | 8.7 | 0.7 | 41.9 | 2.7 | 55.4 | 0.0 | 2.1 | 84.0 | 13.9 | 0.0 |  |
| Total \% | 1.2 | 0.1 | 0.2 | 0.0 | 16.6 | 23.1 | 3.8 | 0.3 | 8.0 | 0.5 | 10.6 | 0.0 | 0.7 | 29.8 | 4.9 | 0.0 |  |

Louisville Rd @ Catholic Church/Walmart Blount County, TN

File Name : 7049-4 Louisville @ Catholic Church-Walmart Site Code : 70490004 Start Date : 2/21/2007
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Southern Traffic Services, Inc.

Louisville Rd @ Catholic Church/Walmart Blount County, TN

Gulf Breeze, FL 32563 File Name : 7049-4 Louisville @ Catholic Church-Walmart 1-800-786-3374 Site Code : 70490004 Start Date : 2/21/2007
Page No : 3

|  | Catholic Church Southbound |  |  |  |  | Louisville Rd Westbound |  |  |  |  | Walmart Northbound |  |  |  |  | Louisville Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{gathered} \mathrm{Rig} \\ \mathrm{ht} \end{gathered}$ | $\begin{aligned} & \text { Utr } \\ & \text { ns } \end{aligned}$ | App. Total | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{gathered} \mathrm{Rig} \\ \mathrm{ht} \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | Left | $\begin{gathered} \text { Thr } \\ \mathrm{u} \end{gathered}$ | $\begin{gathered} \text { Rig } \\ \mathrm{ht} \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |

Peak Hour From 07:00 to 09:45-Peak 1 of 1

| Intersecti Volume | 07:30 | 0 | 1 | 0 | 5 | 97 | 146 | 98 | 2 | 343 | 32 | 15 | 49 | 0 | 96 | 17 | 337 | 35 | 0 | 389 | 833 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Percent | $\begin{array}{r} 80 . \\ 0 \end{array}$ | 0.0 | $\begin{array}{r} 20 . \\ 0 \end{array}$ | 0.0 |  | $\begin{array}{r} 28 . \\ 3 \end{array}$ | $\begin{array}{r} 42 . \\ 6 \end{array}$ | $28 .$ | 0.6 |  | 33. | $15 .$ | $51 .$ | 0.0 |  | 4.4 | 86. 6 | 9.0 | 0.0 |  |  |
|  | 1 | 0 | 0 | 0 | 1 | 29 | 34 | 51 | 1 | 115 | 8 | 11 | 8 | 0 | 27 | 10 | 98 | 7 | 0 | 115 | $\begin{aligned} & 258 \\ & 0.807 \end{aligned}$ |
| High Int. | 08:15 |  |  |  |  | 07:45 |  |  |  |  | 07:45 |  |  |  |  | 07:45 |  |  |  |  |  |
| Volume | 3 | 0 | 1 | 0 | 4 | 29 | 34 | 51 | 1 | 115 | 8 | 11 | 8 | 0 | 27 | 10 | 98 | 7 | 0 | 115 |  |
| Peak |  |  |  |  | 0.31 3 |  |  |  |  | 0.74 6 |  |  |  |  | 0.88 9 |  |  |  |  | 0.84 |  |



Southern Traffic Services, Inc.

Louisville Rd @ Catholic Church/Walmart Blount County, TN

Gulf Breeze, FL 32563 File Name : 7049-4 Louisville @ Catholic Church-Walmart 1-800-786-3374 Site Code : 70490004 Start Date : 2/21/2007
Page No : 4

|  | Catholic Church Southbound |  |  |  |  | Louisville Rd Westbound |  |  |  |  | Walmart Northbound |  |  |  |  | Louisville Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | Utr ns | App. Total | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \end{array}$ | Utr ns | App. Total | Left | $\begin{array}{r} \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{aligned} & \text { Rig } \\ & \text { ht } \end{aligned}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. <br> Total | Left | $\begin{array}{r} \mathrm{Thr} \\ \mathrm{u} \end{array}$ | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{gathered} \text { Utr } \\ \text { ns } \end{gathered}$ | App. Total | $\begin{aligned} & \text { Int. } \\ & \text { Total } \end{aligned}$ |

Peak Hour From 10:00 to 13:45-Peak 1 of 1

| Intersecti on Volume | $11: 45$ 15 | 2 | 2 | 0 | 19 | 250 | 237 | 11 | 4 | 502 | 107 | 3 | 139 | 0 | 249 | 2 | 309 | 75 | 0 | 386 | 1156 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Percent | $\begin{array}{r} 78 . \\ 9 \end{array}$ | $\begin{array}{r} 10 . \\ 5 \end{array}$ | $\begin{array}{r} 10 . \\ 5 \end{array}$ | 0.0 |  | $49 .$ | $47 .$ | 2.2 | 0.8 |  | $43 .$ | 1.2 | $55 .$ $8$ | 0.0 |  | 0.5 | 80. | 19. | 0.0 |  |  |
| 12:30 Volume Peak Factor | 1 | 1 | 1 | 0 | 3 | 67 | 52 | 4 | 1 | 124 | 38 | 1 | 36 | 0 | 75 | 0 | 87 | 13 | 0 | 100 | $\begin{aligned} & 302 \\ & 0.957 \end{aligned}$ |
| High Int. | 12:00 |  |  |  |  | 12:15 |  |  |  |  | 12:30 |  |  |  |  | 12:00 |  |  |  |  |  |
| Volume | 8 | 1 | 0 | 0 | 9 | 74 | 71 | 1 | 0 | 146 | 38 | 1 | 36 | 0 | 75 | 1 | 80 | 23 | 0 | 104 |  |
| Peak |  |  |  |  | 0.52 |  |  |  |  | 0.86 |  |  |  |  | 0.83 |  |  |  |  | 0.92 |  |
| Factor |  |  |  |  | 8 |  |  |  |  | 0 |  |  |  |  | 0 |  |  |  |  | 8 |  |



Southern Traffic Services, Inc.

Louisville Rd @ Catholic Church/Walmart Blount County, TN

Gulf Breeze, FL 32563 File Name : 7049-4 Louisville @ Catholic Church-Walmart 1-800-786-3374 Site Code : 70490004 Start Date : 2/21/2007
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## Appendix C - Capacity Analysis Worksheets

HCS+: Freeway Weaving Release 5.21
Operational Analysis $\qquad$

```
Analyst: FTG
Agency/Co.: FTG
Date Performed: June 2007
Analysis Time Period: AM Peak Hour
Freeway/Dir of Travel: SB SR 115
Weaving Location: btw Louisville Road and retail
Jurisdiction: Alcoa, TN
Analysis Year: Year 2012
```

FTG
FTG
June 2007
AM Peak Hour
SB SR 115
btw Louisville Road and retail
Alcoa, TN
Year 2012
Description: 10360

Description: 10360
Inputs

| Freeway free-flow speed, SFF | 55 | mph |
| :--- | :--- | :--- |
| Weaving number of lanes, N | 3 |  |
| Weaving segment length, L | 660 | Level |
| Terrain type |  | $\%$ |
| Grade | A | mi |
| Length | 0.25 |  |
| Weaving type | 0.46 |  |

Conversion to pc/h Under Base Conditions $\qquad$

|  | Non-Weaving |  | Weaving |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | V | V | V | V |  |
|  | A-C | B-D | A-D | B-C |  |
| Volume, V | 1015 | 0 | 179 | 154 | veh/h |
| Peak-hour factor, PHF | 0.95 | 0.95 | 0.95 | 0.95 |  |
| Peak 15-min volume, v15 | 267 | 0 | 47 | 41 | v |
| Trucks and buses | 4 | 4 | 4 | 4 | \% |
| Recreational vehicles | 1 | 1 | 1 | 1 | \% |
| Trucks and buses PCE, ET | 1.5 | 1.5 | 1.5 | 1.5 |  |
| Recreational vehicle PCE, ER | 1.2 | 1.2 | 1.2 | 1.2 |  |
| Heavy vehicle adjustment, fHV | 0.978 | 0.978 | 0.978 | 0.978 |  |
| Driver population adjustment, fP | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flow rate, v | 1091 | 0 | 192 | 165 | $\mathrm{pc} / \mathrm{h}$ |

Weaving and Non-Weaving Speeds $\qquad$

|  | Weaving | Non-Weaving |
| :--- | :--- | :--- |
| a (Exhibit 24-6) | 0.15 | 0.0035 |
| b (Exhibit 24-6) | 2.20 | 4.00 |
| c (Exhibit 24-6) | 0.97 | 1.30 |
| d (Exhibit 24-6) | 0.80 | 0.75 |
| Weaving intensity factor, Wi | 0.54 | 0.20 |
| Weaving and non-weaving speeds, Si | 44.18 | 52.50 |
| Number of lanes required for |  |  |
| unconstrained operation, Nw (Exhibit 24-7) | 0.87 |  |
| Maximum number of lanes, Nw (max) (Exhibit 24-7) | 1.40 |  |
| Type of operation is |  | Unconstrained |

$\qquad$

| Weaving segment speed, S | 50.17 | mph |
| :--- | :--- | :--- |
| Weaving segment density, D | 9.62 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |
| Level of service, LOS | A |  |
| Capacity of base condition, cb | 4668 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a 15-minute flow rate, c | 4568 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a full-hour volume, ch | 4340 | $\mathrm{pc} / \mathrm{h}$ |

Limitations on Weaving Segments $\qquad$

Weaving flow rate, Vw
Average flow rate (pcphpl)
Volume ratio, VR
Analyzed

| If Max Exceeded | See |
| :--- | :---: |
| Maximum | Note |
| 2800 | a |
| 2250 | b |
| 0.45 | c |
| N/A | d |
| 2500 | e |


| Weaving ratio, R | 0.46 | N/A |
| :--- | :--- | ---: |
| Weaving length (ft) | 660 | 2500 |

Notes:
a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
b. Capacity constrained by basic freeway capacity.
c. Capacity occurs under constrained operating conditions.
d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
i. Type $C$ weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS+: Freeway Weaving Release 5.21
Operational Analysis $\qquad$

```
Analyst: FTG
Agency/Co.: FTG
Date Performed: June 2007
Analysis Time Period: PM Peak Hour
Freeway/Dir of Travel: SB SR 115
Weaving Location: btw Louisville Road and retail
Jurisdiction: Alcoa, TN
Analysis Year: Year 2012
```

FTG
FTG
June 2007
PM Peak Hour
SB SR 115
btw Louisville Road and retail
Alcoa, TN
Year 2012
Description: 10360

|  | Inputs_ |  |
| :--- | :--- | :--- |
| Freeway free-flow speed, SFF | 55 | mph |
| Weaving number of lanes, N | 3 |  |
| Weaving segment length, L | 660 | Level |
| Terrain type |  | $\%$ |
| Grade | A | mi |
| Length | 0.28 |  |
| Weaving type | 0.45 |  |

Conversion to pc/h Under Base Conditions $\qquad$

|  | Non-Weaving |  | Weaving |  | veh/h |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | V | V | V | V |  |
|  | A-C | B-D | A-D | B-C |  |
| Volume, V | 1646 | 0 | 290 | 351 |  |
| Peak-hour factor, PHF | 0.95 | 0.95 | 0.95 | 0.95 |  |
| Peak 15-min volume, v15 | 433 | 0 | 76 | 92 | v |
| Trucks and buses | 4 | 4 | 4 | 4 | \% |
| Recreational vehicles | 1 | 1 | 1 | 1 | \% |
| Trucks and buses PCE, ET | 1.5 | 1.5 | 1.5 | 1.5 |  |
| Recreational vehicle PCE, ER | 1.2 | 1.2 | 1.2 | 1.2 |  |
| Heavy vehicle adjustment, fHV | 0.978 | 0.978 | 0.978 | 0.978 |  |
| Driver population adjustment, fP | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flow rate, v | 1770 | 0 | 311 | 377 | $\mathrm{pc} / \mathrm{h}$ |

Weaving and Non-Weaving Speeds $\qquad$

|  | Weaving | Non-Weaving |
| :--- | :--- | :--- |
| a (Exhibit 24-6) | 0.15 | 0.0035 |
| b (Exhibit 24-6) | 2.20 | 4.00 |
| c (Exhibit 24-6) | 0.97 | 1.30 |
| d (Exhibit 24-6) | 0.80 | 0.75 |
| Weaving intensity factor, Wi | 0.96 | 0.44 |
| Weaving and non-weaving speeds, Si | 37.96 | 46.20 |
| Number of lanes required for |  |  |
| unconstrained operation, Nw (Exhibit 24-7) | 1.00 |  |
| Maximum number of lanes, Nw (max) (Exhibit 24-7) | 1.40 |  |
| Type of operation is |  | Unconstrained |

$\qquad$

| Weaving segment speed, S | 43.55 | mph |
| :--- | :--- | :--- |
| Weaving segment density, D | 18.81 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |
| Level of service, LoS | B |  |
| Capacity of base condition, cb | 4548 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a 15-minute flow rate, c | 4450 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a full-hour volume, ch | 4227 | $\mathrm{pc} / \mathrm{h}$ |

Limitations on Weaving Segments $\qquad$

Weaving flow rate, Vw
Average flow rate (pcphpl)
Volume ratio, VR
Weaving ratio, R

|  | If Max Exceeded See Note |  |
| :--- | :--- | :---: |
| Analyzed | Maximum | Note |
| 688 | 2800 | a |
| 819 | 2250 | b |
| 0.28 | 0.45 | c |
| 0.45 | N/A | d |
| 660 | 2500 | e |

Notes:
a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
b. Capacity constrained by basic freeway capacity.
c. Capacity occurs under constrained operating conditions.
d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
i. Type $C$ weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS+: Freeway Weaving Release 5.21
Operational Analysis $\qquad$

```
Analyst: FTG
Agency/Co.: FTG
Date Performed: June 2007
Analysis Time Period: AM Peak Hour
Freeway/Dir of Travel: SB SR 115
Weaving Location: btw Louisville Road and retail
Jurisdiction: Alcoa, TN
Analysis Year: Year 2032
```

FTG
FTG
June 2007
AM Peak Hour
SB SR 115
btw Louisville Road and retail
Alcoa, TN
Year 2032
Description: 10360

Description: 10360
Inputs

| Freeway free-flow speed, SFF | 55 | mph |
| :--- | :--- | :--- |
| Weaving number of lanes, N | 3 |  |
| Weaving segment length, L | 660 | ft |
| Terrain type |  | $\%$ |
| Grade | A | mi |
| Length | 0.25 |  |
| Weaving type | 0.46 |  |
| Volume ratio, VR |  |  |

Conversion to pc/h Under Base Conditions $\qquad$

|  | Non-Weaving |  | Weaving |  | veh/h |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | V | V | V | V |  |
|  | A-C | B-D | A-D | B-C |  |
| Volume, V | 1217 | 0 | 215 | 185 |  |
| Peak-hour factor, PHF | 0.95 | 0.95 | 0.95 | 0.95 |  |
| Peak 15-min volume, v15 | 320 | 0 | 57 | 49 | v |
| Trucks and buses | 4 | 4 | 4 | 4 | \% |
| Recreational vehicles | 1 | 1 | 1 | 1 | \% |
| Trucks and buses PCE, ET | 1.5 | 1.5 | 1.5 | 1.5 |  |
| Recreational vehicle PCE, ER | 1.2 | 1.2 | 1.2 | 1.2 |  |
| Heavy vehicle adjustment, fHV | 0.978 | 0.978 | 0.978 | 0.978 |  |
| Driver population adjustment, fP | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flow rate, v | 1309 | 0 | 231 | 199 | $\mathrm{pc} / \mathrm{h}$ |

Weaving and Non-Weaving Speeds $\qquad$

|  | Weaving | Non-Weaving |
| :--- | :--- | :--- |
| a (Exhibit 24-6) | 0.15 | 0.0035 |
| b (Exhibit 24-6) | 2.20 | 4.00 |
| c (Exhibit 24-6) | 0.97 | 1.30 |
| d (Exhibit 24-6) | 0.80 | 0.75 |
| Weaving intensity factor, Wi | 0.65 | 0.25 |
| Weaving and non-weaving speeds, Si | 42.30 | 50.88 |
| Number of lanes required for |  |  |
| unconstrained operation, Nw (Exhibit 24-7) | 0.89 |  |
| Maximum number of lanes, Nw (max) (Exhibit 24-7) | 1.40 |  |
| Type of operation is |  | Unconstrained |

$\qquad$

| Weaving segment speed, S | 48.45 | mph |
| :--- | :--- | :--- |
| Weaving segment density, D | 11.96 | $\mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |
| Level of service, LoS | B |  |
| Capacity of base condition, cb | 4666 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a 15-minute flow rate, c | 4566 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a full-hour volume, ch | 4338 | $\mathrm{pc} / \mathrm{h}$ |

Limitations on Weaving Segments $\qquad$

Weaving flow rate, Vw
Average flow rate (pcphpl)
Volume ratio, VR
Weaving ratio, R

|  | If Max Exceeded See Note |  |
| :--- | :--- | :---: |
| Analyzed | Maximum | Note |
| 430 | 2800 | a |
| 579 | 2250 | b |
| 0.25 | 0.45 | c |
| 0.46 | N/A | d |
| 660 | 2500 | e |

heaving length (ft)
Notes:
a. Weaving segments longer than 2500 ft. are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
b. Capacity constrained by basic freeway capacity.
c. Capacity occurs under constrained operating conditions.
d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
i. Type $C$ weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

HCS+: Freeway Weaving Release 5.21
Operational Analysis $\qquad$

```
Analyst: FTG
Agency/Co.: FTG
Date Performed: June 2007
Analysis Time Period: PM Peak Hour
Freeway/Dir of Travel: SB SR 115
Weaving Location: btw Louisville Road and retail
Jurisdiction: Alcoa, TN
Analysis Year: Year 2032
```

FTG
FTG
June 2007
PM Peak Hour
SB SR 115
btw Louisville Road and retail
Alcoa, TN
Year 2032
Description: 10360

Description: 10360
Inputs

| Freeway free-flow speed, SFF | 55 | mph |
| :--- | :--- | :--- |
| Weaving number of lanes, N | 3 |  |
| Weaving segment length, L | 660 | ft |
| Terrain type |  | $\%$ |
| Grade | A | mi |
| Length | 0.28 |  |
| Weaving type | 0.45 |  |
| Volume ratio, VR |  |  |

Conversion to pc/h Under Base Conditions $\qquad$

|  | Non-Weaving |  | Weaving |  | veh/h |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | V | V | V | V |  |
|  | A-C | B-D | A-D | B-C |  |
| Volume, V | 1975 | 0 | 349 | 421 |  |
| Peak-hour factor, PHF | 0.95 | 0.95 | 0.95 | 0.95 |  |
| Peak 15-min volume, v15 | 520 | 0 | 92 | 111 | v |
| Trucks and buses | 4 | 4 | 4 | 4 | \% |
| Recreational vehicles | 1 | 1 | 1 | 1 | \% |
| Trucks and buses PCE, ET | 1.5 | 1.5 | 1.5 | 1.5 |  |
| Recreational vehicle PCE, ER | 1.2 | 1.2 | 1.2 | 1.2 |  |
| Heavy vehicle adjustment, fHV | 0.978 | 0.978 | 0.978 | 0.978 |  |
| Driver population adjustment, fP | 1.00 | 1.00 | 1.00 | 1.00 |  |
| Flow rate, v | 2124 | 0 | 375 | 452 | $\mathrm{pc} / \mathrm{h}$ |

Weaving and Non-Weaving Speeds $\qquad$

|  | Weaving | Non-Weaving |
| :--- | :--- | :--- |
| a (Exhibit 24-6) | 0.15 | 0.0035 |
| b (Exhibit 24-6) | 2.20 | 4.00 |
| c (Exhibit 24-6) | 0.97 | 1.30 |
| d (Exhibit 24-6) | 0.80 | 0.75 |
| Weaving intensity factor, Wi | 1.15 | 0.56 |
| Weaving and non-weaving speeds, Si | 35.96 | 43.82 |
| Number of lanes required for |  |  |
| unconstrained operation, Nw (Exhibit 24-7) | 1.02 |  |
| Maximum number of lanes, Nw (max) (Exhibit 24-7) | 1.40 |  |
| Type of operation is |  | Unconstrained |

$\qquad$

| Weaving segment speed, S | 41.29 | mph |
| :--- | :--- | :--- |
| Weaving segment density, D | $23.82 \mathrm{pc} / \mathrm{mi} / \mathrm{ln}$ |  |
| Level of service, LoS | C |  |
| Capacity of base condition, cb | 4547 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a 15-minute flow rate, c | 4449 | $\mathrm{pc} / \mathrm{h}$ |
| Capacity as a full-hour volume, ch | 4227 | $\mathrm{pc} / \mathrm{h}$ |

Limitations on Weaving Segments $\qquad$

Weaving flow rate, Vw
Average flow rate (pcphpl)
Volume ratio VR
Weaving ratio, R 0.45
Weaving length (ft)
Notes:

|  | If Max Exceeded See Note |  |
| :--- | :--- | :---: |
| Analyzed | Maximum | Note |
| 827 | 2800 | a |
| 983 | 2250 | b |
| 0.28 | 0.45 | c |
| 0.45 | N/A | d |
| 660 | 2500 | e |

a. Weaving segments longer than 2500 ft . are treated as isolated merge and diverge areas using the procedures of Chapter 25, "Ramps and Ramp Junctions".
b. Capacity constrained by basic freeway capacity.
c. Capacity occurs under constrained operating conditions.
d. Three-lane Type A segments do not operate well at volume ratios greater than 0.45. Poor operations and some local queuing are expected in such cases.
e. Four-lane Type A segments do not operate well at volume ratios greater than 0.35. Poor operations and some local queuing are expected in such cases.
f. Capacity constrained by maximum allowable weaving flow rate: 2,800 pc/h (Type A), 4,000 (Type B), 3,500 (Type C).
g. Five-lane Type A segments do not operate well at volume ratios greater than 0.20. Poor operations and some local queuing are expected in such cases.
h. Type B weaving segments do not operate well at volume ratios greater than 0.80. Poor operations and some local queuing are expected in such cases.
i. Type $C$ weaving segments do not operate well at volume ratios greater than 0.50. Poor operations and some local queuing are expected in such cases.

CAPACITY ANALYSIS WORKSHEETS
OPTION 2

EXISTING LANEAGE OPTION 2

HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: April 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs

N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY


| uration $0.25 \quad$ Area Type: All other areas |
| ---: | ---: |
| Signal Operations |

$\qquad$

Green 10.010 .0

| Yellow | 4.0 | 4.0 |
| :--- | :--- | :--- |


| 10.0 | 66.0 |
| :--- | :--- |
| 4.0 | 4.0 |
| 2.0 | 2.0 |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | ( s ) | v/c | g/C | Delay LOS | Delay LOS |

E


BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: April 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs

N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T |  | L | T |  | L | T |  | L | T |  |  |
| Volume | \| 334 | 742 |  | \| 203 | 952 |  | \| 314 | 1308 |  | \| 210 | 1733 |  |  |
| Lane Width | 12.0 | 12.0 |  | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  |  |
| RTOR Vol |  |  |  |  |  |  |  |  |  |  |  |  |  |

uration $0.25 \quad$ Area Type: All other areas
Signal Operations
$\qquad$


Green $10.0 \quad 12.0$
$\begin{array}{lll}\text { Yellow } & 4.0 & 4.0\end{array}$

| 10.0 | 64.0 |
| :--- | :--- |
| 4.0 | 4.0 |
| 2.0 | 2.0 |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios | Lane Group | Approach |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{c}$ | $\mathrm{g} / \mathrm{C}$ |  | $\overline{\text { Delay LOS }}$ |

E


BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: April 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs

N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  |  | tbound |  | We | tbou |  |  | thbour |  | Sou | uthboun |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T |  | L |  |  |  | T |  | L | T |  |  |
| Volume | 374 | 379 |  | \| 56 | 428 |  | \| 212 | 2410 |  | \| 258 | 1376 |  |  |
| Lane Width | 12.0 | 12.0 |  | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  |  |
| RTOR Vol |  |  |  |  |  |  |  |  |  |  |  |  |  |


| uration $0.25 \quad$ Area Type: All other areas |
| ---: | ---: |
| Signal Operations |

$\qquad$


Green 10.010 .0
$\begin{array}{lll}\text { Yellow } & 4.0 & 4.0\end{array}$

| 10.0 | 66.0 |
| :--- | :--- |
| 4.0 | 4.0 |
| 2.0 | 2.0 |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lane | Group | Flow Rate |  |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{c}$ | $\mathrm{g} / \mathrm{C}$ |  | Delay LOS |  |



BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: April 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs

N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY


| uration $0.25 \quad$ Area Type: All other areas |
| ---: | ---: |
| Signal Operations |

$\qquad$

| Phase Combination | 1 | 2 | 3 | 4 |
| :--- | :--- | :--- | :--- | :--- |
| EB Left | A |  |  |  |

Thru Right Peds
WB Left Thru Right Peds
NB Right
SB Right
Green Yellow All Red
$10.0 \quad 12.0$
$4.0 \quad 4.0$
2.02 .0


Intersection Performance Summary

| 10.0 | 64.0 |
| :--- | :--- |
| 4.0 | 4.0 |
| 2.0 | 2.0 |

Cycle Length: 120.0 secs
y

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lane | Group | Flow Rate |  |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | v/c | g/C |  | Delay LOS | Delay LOS |

E


BACK OF QUEUE WORKSHEET


WITH EASTBOUND AND WESTBOUND RIGHT TURNS SIGNALIZED OPTION 2

HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 1 | 1 | 2 | 1 | 2 | 2 | $\bigcirc$ | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | \| 312 | 316 | 154 | \| 47 | 357 | 419 | \|177 | 2008 |  | \| 215 | 1147 |  |  |
| Lane Width | \|12.0 | 12.0 | 12.0 | \|12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \| 12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  | \| |  |  |  |

uration 0.25 Area Type: All other areas

Signal Operations $\qquad$

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 10.0 |  |  |  | 10.0 | 66.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{c}$ | $\mathrm{g} / \mathrm{C}$ |  | Delay LOS |

E


BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 1 | 1 | 2 | 1 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | 334 | 742 | 351 | 203 | 952 | 279 | \| 314 | 1308 |  | \| 210 | 1733 |  |  |
| Lane Width | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  |  |  |  |  |

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 12.0 |  |  |  | 10.0 | 64.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | ( s ) | v/c | g/C | Delay LOS | Delay LOS |

E


BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY $\qquad$

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 1 | 1 | 2 | 1 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | \| 374 | 379 | 185 | 56 | 428 | 503 | \| 212 | 2410 |  | \| 258 | 1376 |  |  |
| Lane Width | \|12.0 | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \| 12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  |  |  |  |  |

uration 0.25 Area Type: All other areas Signal Operations $\qquad$

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 10.0 |  |  |  | 10.0 | 66.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lane | Group | Flow Rate |  |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{C}$ | $\mathrm{g} / \mathrm{C}$ |  | Delay LOS | Delay LOS |

E

| astbound |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L | 292 | 3505 | 1.35 | 0.08 | 233.2 | F |  |  |
| T | 302 | 3618 | 1.32 | 0.08 | 220.9 | F | 239.8 | F |
| R | 135 | 1615 | 1.44 | 0.08 | 291.6 | F |  |  |
| Westbound |  |  |  |  |  |  |  |  |
| L | 150 | 1805 | 0.39 | 0.08 | 53.8 | D |  |  |
| T | 302 | 3618 | 1.49 | 0.08 | 293.8 | F | 836.3 | F |
| R | 135 | 1615 | 3.92 | 0.08 | 1386 | F |  |  |
| Northbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.76 | 0.08 | 65.2 | E |  |  |
| T | 1990 | 3618 | 1.27 | 0.55 | 154.8 | F | 147.5 | F |
| Southbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.93 | 0.08 | 89.7 | F |  |  |
| T | 1990 | 3618 | 0.73 | 0.55 | 21.6 | C | 32.4 | C |
| Intersection Delay $=241.1$ (sec/veh) Intersection LOS $=$ F |  |  |  |  |  |  |  |  |

BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right |  |  |  |  |
| SB Right |  |  |  | WB | Right |  |  |  |  |
| Green | 10.0 | 12.0 |  |  |  | 10.0 | 64.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lane | Group | Flow Rate |  |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{c}$ | $\mathrm{g} / \mathrm{C}$ |  | Delay LOS | Delay LOS |

E


BACK OF QUEUE WORKSHEET

|  | Eastbound |  |  | Westbound |  |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LaneGroup | \|L | T | R | \|L | T | R | \|L | T | \| L | T |
| Init Queue | \| 0.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | 0.0 |
| Flow Rate | \| 217 | 492 | 443 | \| 257 | 631 | 353 | \| 204 | 868 | 136 | 1149 |
| So | \|1900 | 1900 | 1900 | \|1900 | 1900 | 1900 | 1900 | 1900 | \|1900 | 1900 |
| No.Lanes | \| 2 | 2 | 1 | \|1 | 2 | 1 | \| 2 | 20 | \| 2 | 20 |
| SL | \|1805 | 1900 | 1615 | \|1805 | 1900 | 1615 | \|1805 | 1900 | \|1805 | 1900 |
| LnCapacity | \|150 | 190 | 0 | \| 150 | 190 | 0 | \| 150 | 1013 | \|150 | 1013 |
| Flow Ratio | \|0.1 | 0.3 | 0.3 | \| 0.1 | 0.3 | 0.2 | \| 0.1 | 0.5 | 0.1 | 0.6 |
| v/c Ratio | \|1.45 | 2.59 |  | \|1.71 | 3.32 |  | \|1.36 | 0.86 | \|0.91 | 1.13 |
| Grn Ratio | \|0.08 | 0.10 | 0.00 | \|0.08 | 0.10 | 0.00 | \|0.08 | 0.53 | 0.08 | 0.53 |
| I Factor |  | 1.000 |  |  | 1.000 |  |  | 1.000 |  | 1.000 |
| AT or PVG | \| 3 | 3 | 3 | 13 | 3 | 3 | \|3 | 3 | \| 3 | 3 |
| Pltn Ratio | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |
| PF2 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |
| Q1 | 17.2 | 16.4 |  | \| 8.6 | 21.0 |  | 16.8 | 24.9 | 14.5 | 38.3 |
| kB | \|0.3 | 0.3 |  | \| 0.3 | 0.3 |  | \| 0.3 | 0.8 | \| 0.3 | 0.8 |
| Q2 | \| 9.2 | 38.2 |  | \|14.0 | 55.6 |  | \| 7.6 | 4.0 | \|1.4 | 22.3 |
| Q Average | \|16.4 | 54.6 |  | \| 22.5 | 76.6 |  | \|14.4 | 28.9 | \| 5.9 | 60.6 |
| Q Spacing | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | \| 25.0 | 25.0 |
| Q Storage | \| 0 | 0 | 0 | 10 | 0 | 0 | \| 0 | 0 | \| 0 | 0 |
| Q S Ratio |  |  |  |  |  |  |  |  |  |  |
| 70th Percentile Output: |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.2 | 1.1 |  | \|1.2 | 1.1 |  | 11.2 | 1.1 | 11.2 | 1.1 |
| B0Q | \|19.1 | 61.5 |  | \| 26.1 | 85.4 |  | \|16.9 | 33.2 | 17.0 | 68.0 |
| QSRatio |  |  |  |  |  |  |  |  |  |  |
| 85th Percentile Output: |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.5 | 1.3 |  | \|1.4 | 1.3 |  | \|1.5 | 1.4 | \|1.5 | 1.3 |
| B0Q | \| 24.2 | 73.7 |  | \| 32.5 | 101 |  | \| 21.4 | 40.9 | \|9.1 | 81.2 |
| QSRatio |  |  |  |  |  |  |  |  |  |  |
| 90th Percentile Output: |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.6 | 1.4 |  | \|1.5 | 1.4 |  | \|1.6 | 1.5 | 11.7 | 1.4 |
| B0Q | \| 25.8 | 77.9 |  | \|34.5 | 108 |  | \| 23.0 | 43.2 | \|10.0 | 86.0 |
| QSRatio |  |  |  |  |  |  |  |  |  |  |
| 95th Percentile Output: |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.7 | 1.5 |  | \|1.7 | 1.5 |  | \|1.8 | 1.6 | \|1.9 | 1.5 |
| B0Q | \| 28.5 | 83.5 |  | \| 37.7 | 116 |  | \| 25.5 | 46.8 | \|11.4 | 92.2 |
| QSRatio |  |  |  |  |  |  |  |  |  |  |
| 98th Percentile Output: |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|2.0 | 1.7 |  | 11.9 | 1.7 |  | \|2.0 | 1.8 | \| 2.3 | 1.7 |
| B0Q | \| 32.5 | 93.7 |  | \| 42.3 | 130 |  | \| 29.3 | 52.3 | \|13.8 | 104 |
| QSRatio | 1 |  |  |  |  |  |  |  |  |  |

WITH DOUBLE EASTBOUND AND WESTBOUND RIGHT TURNS SIGNALIZED OPTION 2

HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 2 | 1 | 2 | 2 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | 312 | 316 | 154 | \| 47 | 357 | 419 | \|177 | 2008 |  | \| 215 | 1147 |  |  |
| Lane Width | 12.0 | 12.0 | 12.0 | \|12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  |  |  |  |  |

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 10.0 |  |  |  | 10.0 | 66.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | ( s ) | v/c | g/C | Delay LOS | Delay LOS |

E

| astbound |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L | 292 | 3505 | 1.12 | 0.08 | 145.1 | F |  |  |
| T | 302 | 3618 | 1.10 | 0.08 | 137.2 | F | 125.4 | F |
| R | 238 | 2859 | 0.68 | 0.08 | 61.1 | E |  |  |
| Westbound |  |  |  |  |  |  |  |  |
| L | 150 | 1805 | 0.33 | 0.08 | 53.1 | D |  |  |
| T | 302 | 3618 | 1.25 | 0.08 | 190.0 | F | 317.0 | F |
| R | 238 | 2859 | 1.85 | 0.08 | 454.6 | F |  |  |
| Northbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.64 | 0.08 | 57.8 | E |  |  |
| T | 1990 | 3618 | 1.06 | 0.55 | 66.1 | E | 65.4 | E |
| Southbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.77 | 0.08 | 66.1 | E |  |  |
| T | 1990 | 3618 | 0.61 | 0.55 | 18.8 | B | 26.2 | C |
|  | Intersection Delay $=104.3$ ( sec/veh) Intersection LOS = F |  |  |  |  |  |  |  |

BACK OF QUEUE WORKSHEET

|  | Eastbound |  |  | Westbound |  |  | Northbound |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LaneGroup | \|L | T | R | L | T | R | \| L | T | \|L | T |  |
| Init Queue | \| 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Flow Rate | \|168 | 174 | 91 | 149 | 197 | 249 | \| 95 | 1110 | \|116 | 633 |  |
| So | \|1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | \|1900 | 1900 |  |
| No. Lanes | \| 2 | 2 | 2 | 1 | 2 | 2 | 2 | 20 | 2 | 20 |  |
| SL | \|1805 | 1900 | 1615 | 1805 | 1900 | 1615 | 1805 | 1900 | 1805 | 1900 |  |
| LnCapacity | \|150 | 158 | 134 | 150 | 158 | 134 | 150 | 1045 | \|150 | 1045 |  |
| Flow Ratio | \| 0.1 | 0.1 | 0.1 | 0.0 | 0.1 | 0.2 | 0.1 | 0.6 | 0.1 | 0.3 |  |
| v/c Ratio | \|1.12 | 1.10 | 0.68 | 0.33 | 1.25 | 1.86 | 0.63 | 1.06 | 0.77 | 0.61 |  |
| Grn Ratio | \| 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.08 | 0.55 | 0.08 | 0.55 |  |
| I Factor |  | 1.000 |  |  | 1.000 |  |  | 1.000 |  | 1.000 |  |
| AT or PVG | \| 3 | 3 | 3 | 3 | 3 | 3 | \| 3 | 3 | \| 3 | 3 |  |
| Pltn Ratio | \|1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |  |
| PF2 | \|1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | \| 1.00 | 1.00 | \|1.00 | 1.00 |  |
| Q1 | \| 5.6 | 5.8 | 2.9 | 1.5 | 6.6 | 8.3 | \| 3.1 | 37.0 | \|3.8 | 14.2 |  |
| kB | \| 0.3 | 0.3 | 0.2 | \|0.3 | 0.3 | 0.2 | \|0.3 | 0.8 | \|0.3 | 0.8 |  |
| Q2 | \| 3.7 | 3.6 | 0.5 | 0.1 | 6.0 | 14.9 | 0.4 | 15.6 | \|0.8 | 1.3 |  |
| Q Average | \| 9.3 | 9.4 | 3.4 | \|1.7 | 12.6 | 23.2 | 3.5 | 52.6 | \| 4.5 | 15.5 |  |
| Q Spacing | \| 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 |  |
| Q Storage | \| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 |  |
| Q S Ratio |  |  |  |  |  |  |  |  |  |  |  |
| 70th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.2 | 1.2 | 1.2 | \|1.2 | 1.2 | 1.2 | \|1.2 | 1.1 | \|1.2 | 1.2 |  |
| B0Q | \|11.0 | 11.1 | 4.1 | \| 2.0 | 14.7 | 26.8 | \| 4.2 | 59.3 | \| 5.4 | 18.1 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 85th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.5 | 1.5 | 1.6 | \|1.6 | 1.5 | 1.4 | 11.6 | 1.4 | \|1.6 | 1.5 |  |
| B0Q | \|14.2 | 14.3 | 5.4 | 2.6 | 18.8 | 33.4 | 5.5 | 71.1 | \| 7.1 | 22.9 |  |
| QSRatio |  |  |  |  |  |  | \| |  | 1 |  |  |
| 90th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.7 | 1.6 | 1.7 | \|1.8 | 1.6 | 1.5 | \|1.7 | 1.4 | \|1.7 | 1.6 |  |
| B0Q | \|15.4 | 15.6 | 6.0 | \| 2.9 | 20.3 | 35.4 | 6.1 | 75.2 | \| 7.8 | 24.6 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 95th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.9 | 1.9 | 2.0 | \| 2.0 | 1.8 | 1.7 | \| 2.0 | 1.5 | \| 2.0 | 1.8 |  |
| B0Q | \|17.3 | 17.5 | 6.8 | \| 3.4 | 22.6 | 38.6 | 7.0 | 80.6 | 8.9 | 27.2 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 98th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \| 2.2 | 2.2 | 2.5 | \| 2.6 | 2.1 | 1.9 | \|2.5 | 1.7 | 12.4 | 2.0 |  |
| B0Q | \| 20.4 | 20.6 | 8.5 | \| 4.3 | 26.1 | 43.3 | \| 8.6 | 90.4 | 10.9 | 31.1 |  |
| QSRatio |  |  |  |  |  |  |  |  | \| |  |  |

HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2012 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 2 | 1 | 2 | 2 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | 334 | 742 | 351 | 203 | 952 | 279 | \| 314 | 1308 |  | \| 210 | 1733 |  |  |
| Lane Width | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \|12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  |  |  |  |  |

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 12.0 |  |  |  | 10.0 | 64.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | ( s ) | v/c | g/C | Delay LOS | Delay LOS |

E


BACK OF QUEUE WORKSHEET

|  | Eastbound |  |  | Westbound |  |  | Northbound |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LaneGroup | \|L | T | R | \|L | T | R | \|L | T | \|L | T |  |
| Init Queue | \| 0.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | 0.0 |  |
| Flow Rate | \|181 | 410 | 208 | \| 214 | 526 | 166 | \|170 | 723 | \|113 | 957 |  |
| So | \|1900 | 1900 | 1900 | \|1900 | 1900 | 1900 | \|1900 | 1900 | \|1900 | 1900 |  |
| No. Lanes | \|2 | 2 | 2 | \|1 | 2 | 2 | \|2 | 20 | \|2 | 20 |  |
| SL | \|1805 | 1900 | 1615 | \|1805 | 1900 | 1615 | \|1805 | 1900 | \|1805 | 1900 |  |
| LnCapacity | \|150 | 190 | 134 | \| 150 | 190 | 134 | \|150 | 1013 | \|150 | 1013 |  |
| Flow Ratio | \|0.1 | 0.2 | 0.1 | \| 0.1 | 0.3 | 0.1 | \| 0.1 | 0.4 | \|0.1 | 0.5 |  |
| v/c Ratio | \|1.21 | 2.16 | 1.55 | \|1.43 | 2.77 | 1.24 | \|1.13 | 0.71 | \|0.75 | 0.94 |  |
| Grn Ratio | \|0.08 | 0.10 | 0.08 | \| 0.08 | 0.10 | 0.08 | \| 0.08 | 0.53 | 0.08 | 0.53 |  |
| I Factor | 1.000 |  |  | 1.000 |  |  |  | 1.000 |  | 1.000 |  |
| AT or PVG | \| 3 | 3 | 3 | \| 3 | 3 | 3 | \| 3 | 3 | \| 3 | 3 |  |
| Pltn Ratio | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |  |
| PF2 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |  |
| Q1 | 16.0 | 13.7 | 6.9 | \| 7.1 | 17.5 | 5.5 | \| 5.7 | 18.2 | \|3.7 | 30.0 |  |
| kB | 10.3 | 0.3 | 0.2 | \| 0.3 | 0.3 | 0.2 | \| 0.3 | 0.8 | \|0.3 | 0.8 |  |
| Q2 | \| 5.1 | 28.1 | 9.9 | \| 8.8 | 42.5 | 5.0 | \| 3.9 | 2.0 | \|0.7 | 7.0 |  |
| Q Average | \|11.1 | 41.7 | 16.8 | \|15.9 | 60.0 | 10.6 | \| 9.6 | 20.1 | \| 4.4 | 37.0 |  |
| Q Spacing | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | \| 25.0 | 25.0 |  |
| Q Storage | \| 0 | 0 | 0 | \| 0 | 0 | 0 | \| 0 | 0 | 0 | 0 |  |
| Q S Ratio |  |  |  |  |  |  |  |  |  |  |  |
| 70th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.2 | 1.1 | 1.2 | \|1.2 | 1.1 | 1.2 | \|1.2 | 1.2 | \|1.2 | 1.1 |  |
| B0Q | \|13.0 | 47.4 | 19.6 | \|18.6 | 67.3 | 12.4 | \|11.3 | 23.3 | \| 5.2 | 42.2 |  |
| QSRatio |  |  |  | \| |  |  | \| |  |  |  |  |
| 85th Percentile Output: 11.5 |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.5 | 1.4 | 1.5 | \|1.5 | 1.3 | 1.5 | 11.5 | 1.5 | 11.6 | 1.4 |  |
| B0Q | \|16.7 | 57.4 | 24.8 | \| 23.5 | 80.4 | 15.9 | \|14.6 | 29.2 | \| 6.8 | 51.4 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 90th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.6 | 1.4 | 1.6 | \|1.6 | 1.4 | 1.6 | 11.6 | 1.5 | 11.7 | 1.5 |  |
| B0Q | \|18.1 | 60.5 | 26.5 | \| 25.2 | 85.2 | 17.3 | \|15.8 | 31.1 | \| 7.5 | 54.2 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 95th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | 11.8 | 1.6 | 1.7 | \|1.7 | 1.5 | 1.8 | 11.9 | 1.7 | \|2.0 | 1.6 |  |
| B0Q | \| 20.2 | 65.0 | 29.2 | \| 27.8 | 91.3 | 19.3 | \|17.8 | 34.1 | 18.6 | 58.4 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 98th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|2.1 | 1.7 | 2.0 | \|2.0 | 1.7 | 2.1 | \|2.2 | 1.9 | \|2.4 | 1.8 |  |
| B0Q | \| 23.6 | 72.6 | 33.2 | \| 31.8 | 103 | 22.6 | \| 20.9 | 38.5 | \|10.6 | 65.1 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |

HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: AM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs
N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

|  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | L | T | R | L | T | R | L | T | R |  |
| No. Lanes | 2 | 2 | 2 | 1 | 2 | 2 | 2 | 2 | 0 | 2 | 2 | 0 |  |
| LGConfig | L | T | R | L | T | R | L | T |  | L | T |  |  |
| Volume | 374 | 379 | 185 | \| 56 | 428 | 503 | \| 212 | 2410 |  | \| 258 | 1376 |  |  |
| Lane Width | 12.0 | 12.0 | 12.0 | \|12.0 | 12.0 | 12.0 | \|12.0 | 12.0 |  | \| 12.0 | 12.0 |  |  |
| RTOR Vol |  |  | 0 |  |  | 0 |  |  |  |  |  |  |  |

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right | A |  |  |  |
| SB Right |  |  |  | WB | Right | A |  |  |  |
| Green | 10.0 | 10.0 |  |  |  | 10.0 | 66.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  | Lane Group | Approach |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Group | Flow Rate |  |  |  |  |
| Grp | Capacity | ( s ) | v/c | g/C | Delay LOS | Delay LOS |

E

| astbound |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L | 292 | 3505 | 1.35 | 0.08 | 233.2 | F |  |  |
| T | 302 | 3618 | 1.32 | 0.08 | 220.9 | F | 196.8 | F |
| R | 238 | 2859 | 0.82 | 0.08 | 73.9 | E |  |  |
| Westbound |  |  |  |  |  |  |  |  |
| L | 150 | 1805 | 0.39 | 0.08 | 53.8 | D |  |  |
| T | 302 | 3618 | 1.49 | 0.08 | 293.8 | F | 445.6 | F |
| R | 238 | 2859 | 2.22 | 0.08 | 618.6 | F |  |  |
| Northbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.76 | 0.08 | 65.2 | E |  |  |
| T | 1990 | 3618 | 1.27 | 0.55 | 154.8 | F | 147.5 | F |
| Southbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.93 | 0.08 | 89.7 | F |  |  |
| T | 1990 | 3618 | 0.73 | 0.55 | 21.6 | C | 32.4 | C |
|  | Intersection Delay $=172.2$ ( sec/veh) Intersection LOS $=\mathrm{F}$ |  |  |  |  |  |  |  |

BACK OF QUEUE WORKSHEET


HCS+: Signalized Intersections Release 5.21

Analyst: FTG
Agency: FTG
Date: June 2007
Period: PM Peak Hour
Project ID: 10360
E/W St: Louisville Road

Inter.: SR 115 and Louisville Road
Area Type: All other areas
Jurisd: Alcoa, TN
Year : 2032 DHVs

N/S St: SR 115 (US 29)

SIGNALIZED INTERSECTION SUMMARY

uration 0.25 Area Type: All other areas

| Phase Comb | 1 | 2 | 34 |  |  | 5 | 6 | 7 | 8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | A |  |  | NB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| WB Left | A |  |  | SB | Left | A |  |  |  |
| Thru |  | A |  |  | Thru |  | A |  |  |
| Right |  |  |  |  | Right |  |  |  |  |
| Peds |  |  |  |  | Peds |  |  |  |  |
| NB Right |  |  |  | EB | Right |  |  |  |  |
| SB Right |  |  |  | WB | Right |  |  |  |  |
| Green | 10.0 | 12.0 |  |  |  | 10.0 | 64.0 |  |  |
| Yellow | 4.0 | 4.0 |  |  |  | 4.0 | 4.0 |  |  |
| All Red | 2.0 | 2.0 |  |  |  | 2.0 | 2.0 |  |  |

Cycle Length: 120.0 secs
Intersection Performance Summary

| Appr/ | Lane | Adj Sat | Ratios |  |  | Lane Group | Approach |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Lane | Group | Flow Rate |  |  |  |  |  |
| Grp | Capacity | $(\mathrm{s})$ | $\mathrm{v} / \mathrm{C}$ | $\mathrm{g} / \mathrm{C}$ |  | Delay LOS |  |

E

| astbound |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L | 292 | 3505 | 1.45 | 0.08 | 273.7 | F |  |  |
| T | 362 | 3618 | 2.59 | 0.10 | 776.8 | F |  |  |
| R | 0 | 2859 |  | 0.00 |  |  |  |  |
| Westbound |  |  |  |  |  |  |  |  |
| L | 150 | 1805 | 1.71 | 0.08 | 402.6 | F |  |  |
| T | 362 | 3618 | 3.32 | 0.10 | 1107 | F |  |  |
| R | 0 | 2859 |  | 0.00 |  |  |  |  |
| Northbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 1.36 | 0.08 | 237.5 | F |  |  |
| T | 1930 | 3618 | 0.86 | 0.53 | 28.1 | C | 68.7 | E |
| Southbound |  |  |  |  |  |  |  |  |
| L | 292 | 3505 | 0.91 | 0.08 | 84.6 | F |  |  |
| T | 1930 | 3618 | 1.13 | 0.53 | 95.4 | F | 94.3 | F |
| Intersection Delay |  |  |  | (sec/veh) Intersection |  |  |  | S |

BACK OF QUEUE WORKSHEET

|  | Eastbound |  |  | Westbound |  |  | Northbound |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LaneGroup | \|L | T | R | \|L | T | R | \|L | T | \|L | T |  |
| Init Queue | 10.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | \| 0.0 | 0.0 | 0.0 | 0.0 |  |
| Flow Rate | \| 217 | 492 | 250 | \| 257 | 631 | 199 | \| 204 | 868 | \|136 | 1149 |  |
| So | \|1900 | 1900 | 1900 | \|1900 | 1900 | 1900 | \| 1900 | 1900 | \|1900 | 1900 |  |
| No.Lanes | \|2 | 2 | 2 | \|1 | 2 | 2 | \| 2 | 20 | \|2 | 20 |  |
| SL | \|1805 | 1900 | 1615 | \|1805 | 1900 | 1615 | \|1805 | 1900 | \|1805 | 1900 |  |
| LnCapacity | \|150 | 190 | 0 | \| 150 | 190 | 0 | \| 150 | 1013 | \|150 | 1013 |  |
| Flow Ratio | \| 0.1 | 0.3 | 0.2 | \| 0.1 | 0.3 | 0.1 | \| 0.1 | 0.5 | \|0.1 | 0.6 |  |
| v/c Ratio | \|1.45 | 2.59 |  | \|1.71 | 3.32 |  | \|1.36 | 0.86 | \|0.91 | 1.13 |  |
| Grn Ratio | \| 0.08 | 0.10 | 0.00 | 0.08 | 0.10 | 0.00 | 0.08 | 0.53 | 0.08 | 0.53 |  |
| I Factor |  | 1.000 |  |  | 1.000 |  |  | 1.000 |  | 1.000 |  |
| AT or PVG | \| 3 | 3 | 3 | \| 3 | 3 | 3 | \| 3 | 3 | \| 3 | 3 |  |
| Pltn Ratio | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |  |
| PF2 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | 1.00 | \|1.00 | 1.00 | \|1.00 | 1.00 |  |
| Q1 | \| 7.2 | 16.4 |  | \| 8.6 | 21.0 |  | \| 6.8 | 24.9 | \| 4.5 | 38.3 |  |
| kB | 10.3 | 0.3 |  | \| 0.3 | 0.3 |  | \| 0.3 | 0.8 | \| 0.3 | 0.8 |  |
| Q2 | 19.2 | 38.2 |  | \| 14.0 | 55.6 |  | \| 7.6 | 4.0 | 11.4 | 22.3 |  |
| Q Average | \|16.4 | 54.6 |  | \| 22.5 | 76.6 |  | \|14.4 | 28.9 | \| 5.9 | 60.6 |  |
| Q Spacing | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | 25.0 | \| 25.0 | 25.0 | \| 25.0 | 25.0 |  |
| Q Storage | \| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | \| 0 | 0 |  |
| Q S Ratio |  |  |  |  |  |  |  |  |  |  |  |
| 70th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.2 | 1.1 |  | \|1.2 | 1.1 |  | \|1.2 | 1.1 | 11.2 | 1.1 |  |
| B0Q | \|19.1 | 61.5 |  | \| 26.1 | 85.4 |  | \|16.9 | 33.2 | \| 7.0 | 68.0 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 85th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.5 | 1.3 |  | \|1.4 | 1.3 |  | \|1.5 | 1.4 | \|1.5 | 1.3 |  |
| B0Q | \| 24.2 | 73.7 |  | \| 32.5 | 101 |  | \| 21.4 | 40.9 | \|9.1 | 81.2 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 90th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.6 | 1.4 |  | \|1.5 | 1.4 |  | \|1.6 | 1.5 | 11.7 | 1.4 |  |
| B0Q | \| 25.8 | 77.9 |  | \| 34.5 | 108 |  | \| 23.0 | 43.2 | \|10.0 | 86.0 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |
| 95th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|1.7 | 1.5 |  | \|1.7 | 1.5 |  | \|1.8 | 1.6 | \|1.9 | 1.5 |  |
| B0Q | \| 28.5 | 83.5 |  | \| 37.7 | 116 |  | \| 25.5 | 46.8 | \|11.4 | 92.2 |  |
| QSRatio |  |  |  | \| |  |  |  |  |  |  |  |
| 98th Percentile Output: |  |  |  |  |  |  |  |  |  |  |  |
| fB\% | \|2.0 | 1.7 |  | \|1.9 | 1.7 |  | \|2.0 | 1.8 | \|2.3 | 1.7 |  |
| B0Q | \| 32.5 | 93.7 |  | \| 42.3 | 130 |  | \| 29.3 | 52.3 | \|13.8 | 104 |  |
| QSRatio |  |  |  |  |  |  |  |  |  |  |  |

