GENERAL

1. THESE GENERAL NOTES REPRESENT AND/OR SUMMARIZE KEY PROJECT INFORMATION FOR THE DRAWING USER'S CONVENIENCE. HOWEVER, ALL CONSTRUCTION DOCUMENTS SHOULD BE REVIEWED FOR FURTHER DETAILS AND REQUIREMENTS.
2. WHERE REFERENCE IS MADE TO VARIOUS CODES, STANDARDS, AND TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE AS SPECIFIED IN SECTION **CODES AND STANDARDS**.
3. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
4. TYPICAL DETAILS ARE NOT CUT ON DRAWINGS, BUT APPLY UNLESS NOTED OTHERWISE.
5. WHERE ANY DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENT SHALL GOVERN.
6. ELEVATIONS SHOWN ARE RELATIVE TO THE TOP OF BRIDGE DECK ELEVATION (T/BRIDGE = 0' – 0"). FOR PROJECT DIMENSIONAL CONTROL, SEE CIVIL DRAWINGS.
7. WORK THESE DRAWINGS WITH THOSE PREPARED BY OTHER DISCIPLINES: CIVIL, RETAINING WALL DESIGN ENGINEER, ETC. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION. DO NOT PENETRATE ANY STRUCTURAL ELEMENTS (BEAMS, COLUMNS, WALLS, SLABS, STEEL DECKS, ETC.) WITHOUT PRIOR WRITTEN APPROVAL OF STRUCTURAL ENGINEER UNLESS SPECIFIC PENETRATION DETAILS ARE SHOWN ON DRAWINGS.
8. THIS PROJECT IS WITHIN THE JURISDICTION OF THE CITY OF JACKSON, TENNESSEE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL BUILDING PERMITS AND SCHEDULING/ACCOMMODATING ALL REQUIRED INSPECTIONS PERTAINING TO THE BUILDING PERMITS.
9. SUBMIT SHOP DRAWINGS, PROJECT DATA AND SAMPLES AS SPECIFIED ON CONSTRUCTION DOCUMENTS.
10. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, WHICH IS DESIGNED TO BE SELF–SUPPORTING AND STABLE AFTER THE CONSTRUCTION IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, ETC. THAT MAY BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.
11. THIS ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ACTS OR OMISSIONS OF THE CONTRACTOR, THEIR SUBCONTRACTORS, AND ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF THESE PERSONS TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
12. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK THAT CONFORMS WITH THE REGULATIONS OF THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY.
13. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IN SUCH A MANNER TO NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT OF THE AREA.
14. OPTIONS AND ALTERNATES ARE FOR THE CONTRACTOR'S CONVENIENCE. IF AN OPTION OR ALTERNATE IS CHOSEN, CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY AND SHALL COORDINATE ALL DETAILS.

CODES & STANDARDS

1.	IBC 2018:	INTERNATIONAL BUILDING CODE
2.	ASCE 7–16:	MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
3.	AISC 360–16:	SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
4.	AWS D1.1–15:	AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE
5.	MNL–66–20:	ACI DETAILING MANUAL
6.	ACI 117–10:	SPECIFICATION FOR TOLERANCES FOR CONCRETE
7.	ACI 301–10:	STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE
8.	ACI 318–14:	BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
9.	ACI 347–04:	STANDARD SPECIFICATION FOR FORMWORK FOR CONCRETE
10.	CRSI–18:	MANUAL OF STANDARD PRACTICE
11.	ASTM A615–15ae1:	STANDARD SPECIFICATION FOR DEFORMED AND PLAIN CARBON STEEL BARS FOR CONCRETE REINFORCEMENT
12.	ASTM C150–15:	STANDARD SPECIFICATION FOR PORTLAND CEMENT

DESIGN CRITERIA

RISK CATEGORY		II
A.	DEAD LOAD	SELF–WEIGHT
B.	LIVE LOADS	
	A. PEDESTRIAN TRAFFIC	100 PSF
	B. MAINTENANCE VEHICLE	5 KIP, 2 AXLES
C.	SNOW LOAD	
	A. GROUND SNOW LOAD	10 PSF
	B. FLAT ROOF SNOW LOAD	8.4 PSF
D.	WIND LOADS	
	A. ULTIMATE WIND SPEED (3–SECOND GUST)	105 MILES PER HOUR
	B. EXPOSURE	C
E.	SEISMIC LOADS	
	A. SITE CLASS	D
	B. SEISMIC DESIGN CATEGORY	D
	a. S_S	0.741
	b. S_1	0.271
	c. S_{DS}	0.596
	d. S_{D1}	N/A *
	C. SEISMIC FORCE RESISTING SYSTEM	STEEL ORDINARY CONCENTRICALLY BRACED FRAMES
	a. REDUNDANCY COEFFICIENT, ρ	1.3
	b. RESPONSE MODIFICATION FACTOR, R	3.25
	c. SEISMIC IMPORTANCE FACTOR, I_e	1.0
	d. OVERSTRENGTH FACTOR, Ω_0	2.0
	e. SEISMIC RESPONSE COEFFICIENT, C_s	0.183
	f. TOTAL BASE SHEAR, $V=C_sW$	7.8 KIPS
D.	ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PROCEDURE

* SITE SPECIFIC GROUND MOTION ANALYSIS CONDITION ASSESSED – IN ACCORDANCE WITH ASCE 7–16, SECTION 11.4.8, THE STRUCTURAL SYSTEM MEETS THE EXCEPTION CRITERIA #2 WHERE THE Cs VALUE WAS DETERMINED ACCORDING TO EQ. 12.8.2 WHERE T<1.5Ts.

EXCAVATIONS

1. EXCAVATIONS SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE PROJECT GEOTECHNICAL REPORT.
2. THE CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE SHORING OR SLOPING OF EXCAVATIONS TO MEET OSHA REQUIREMENTS. DESIGN OF SHORING IS THE CONTRACTORS RESPONSIBILITY. EXCAVATIONS SHALL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND FEDERAL REGULATIONS.
3. BLASTING IS NOT PERMITTED.
4. ANY SURFACES DISTURBED DURING CONSTRUCTION SHALL BE GRADED TO MATCH SITE GRADING DRAWINGS.
5. POSITIVE SITE DRAINAGE SHALL BE PROVIDED AT ALL TIMES SO THAT WATER IS NOT ALLOWED TO POND IN EXCAVATIONS OR NEAR FOUNDATIONS
6. ANY SOFT SPOTS ENCOUNTERED ON SITE SHALL BE REMOVED AND REPLACED WITH ENGINEERED FILL. REFER TO GEOTECHNICAL REPORT FOR ADDITIONAL REQUIREMENTS.
7. EXCAVATIONS SHALL BE FREE OF STANDING WATER, ICE, DEBRIS, OR OTHER DELETERIOUS MATERIALS PRIOR TO PLACING CONCRETE.

FOUNDATION NOTES

1.	FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH THE FOLLOWING GEOTECHNICAL REPORT	
	A. PREPARER	GOODWYN MILLS CAWOOD, LLC, OF NASHVILLE TN
	B. PROJECT NUMBER	GNAS250061
	C. REPORT DATE	MARCH 10, 2026
2.	THE DESIGN PARAMETERS WERE PROVIDED FOR SOIL PROPERTIES AS NOTED BELOW:	
	A. FROST DEPTH	1.5 FT
	B. BEARING CAPACITY	1,000 PSF (NET ALLOWABLE)
	C. DENSITY OF SOIL	120 PCF (ABOVE WATER TABLE) 57.6 PCF (BELOW WATER TABLE)
	D. MODULUS OF SUBGRADE REACTION	N/A (0–5 FT, SOFT CLAY) 20 PCI (5–9 FT, LOOSE SAND) 30 PCI (9–17 FT, SOFT CLAY) 20 PCI (17–35 FT, LOOSE SAND)
	E. EFF ANGLE OF INTERNAL FRICTION, Φ	29 DEGREES
	F. AT–REST LATERAL SOIL PRESSURE COEFF, Ko	0.48
	G. ACTIVE LATERAL SOIL PRESSURE COEFF, Ka	0.32
	H. PASSIVE LATERAL SOIL PRESSURE COEFF, Kp	2.0
	I. PASSIVE RESISTANCE	240 PCF (NEGLECT TOP 1 FOOT OF SOIL)
	J. COEFFICIENT OF FRICTION	0.40 (BETWEEN BASE AND STIFF/MEDIUM DENSE SOIL) 0.50 (BETWEEN BASE AND COMPACTED SHOT ROCK)
3.	GEOTECHNICAL ENGINEER TO BE RETAINED BY OWNER TO PROVIDE OBSERVATION AND TESTING SERVICES DURING THE GRADING AND FOUNDATION PHASE OF CONSTRUCTION. INSPECTION AND TESTING REPORT SHALL BE SUBMITTED TO THE ENGINEER OF RECORD.	
4.	PREPARE FOUNDATION SUBGRADE IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT.	
5.	IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE WHETHER OR NOT ADDITIONAL GEOTECHNICAL INFORMATION IS REQUIRED AND TO PROVIDE SUCH INFORMATION AS HE/SHE DEEMS NECESSARY.	
6.	PRIOR TO PLACEMENT OF CONCRETE, CONTRACTOR SHALL CONFIRM ALL FOUNDATION EMBED ITEMS – PIPES SLEEVES, STEEL PLATES, INSERTS, ETC., ARE IN PLACE.	

1

ABBREVIATIONS

THE FOLLOWING ABBREVIATIONS MAY BE USED ON THE DRAWINGS:

A ROD	ANCHOR ROD	LLH	LONG LEG HORIZONTAL (ANGLE)
A/E	ARCHITECT & ENGINEER	LLV	LONG LEG VERTICAL (ANGLE)
AB	ANCHOR BOLT	LSH	LONG SIDE HORIZONTAL (HSS)
ACI	AMERICAN CONCRETE INSTITUTE	LSV	LONG SIDE VERTICAL (HSS)
ADDL	ADDITIONAL	LB	POUND
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LF	LINEAR FEET
ALT	ALTERNATE	LL	LIVE LOAD
ALUM	ALUMINUM	LOC	LOCATION
APPROX	APPROXIMATE	LONG	LONGITUDINAL
APVD	APPROVED	LP	LOW POINT
ARCH	ARCHITECTURAL/ARCHITECT	LSH	LONG SLOTTED HOLE
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS		
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS	M	MOMENT
AWS	AMERICAN WELDING SOCIETY	MATL	MATERIAL
		MAX	MAXIMUM
B/	BOTTOM OF	MC	MOMENT CONNECTION
BLDG	BUILDING	MCJ	MASONRY CONTROL JOINT
BM	BEAM	MECH	MECHANICAL
BOT	BOTTOM	MFD	MANUFACTURED
BRG	BEARING	MFR	MANUFACTURER
BTWN	BETWEEN	MIN	MINIMUM
		MISC	MISCELLANEOUS
CALC	CALCULATION(S)	MSNRY	MASONRY
CHKD	CHECKED	MTD	MOUNTED
CHKRD	CHECKERED	MTL	METAL
CIP	CAST–IN–PLACE CONCRETE		
CJ	CONSTRUCTION/CONTROL JOINT	N&F	NEAR AND FAR
CJP	COMPLETE JOINT PENETRATION	N/A	NOT APPLICABLE
CL	CENTERLINE	NIC	NOT IN CONTRACT
CLR	CLEAR/CLEARANCE	NO	NUMBER
CMU	CONCRETE MASONRY UNIT	NOM	NOMINAL
COL	COLUMN	NS	NEAR SIDE
CONC	CONCRETE	NTS	NOT TO SCALE
CONN	CONNECTION		
CONT	CONTINUOUS	OC	ON CENTER
COORD	COORDINATE	OD	OUTSIDE DIAMETER
CP	CONCRETE PIER	OF	OUTSIDE FACE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	OPNG	OPENING
CTR	CENTER	OPP	OPPOSITE
CTRD	CENTERED	OSH	OVERSIZED HOLE
CU	CUBIC	OWJ	OPEN WEB JOIST
DBA	DEFORMED BAR ANCHOR	P/T	POST TENSIONED(ING)
DBL	DOUBLE	PART	PARTITION
DEG	DEGREES	PCF	POUNDS PER CUBIC FOOT
DET	DETAIL	PCI	POUNDS PER CUBIC INCH
DIA	DIAMETER	PE	PROFESSIONAL ENGINEER
DIAG	DIAGONAL	PENE	PENETRATION
DIR	DIRECTION	PERIM	PERIMETER
DL	DEAD LOAD	PL	PLATE
DP	DRILLED PIER	PLCS	PLACES
DWG	DRAWING	PLF	POUNDS PER LINEAR FOOT
		PREFAB	PREFABRICATED
(E)	EXISTING	PROJ	PROJECTION
EA	EACH	PSF	POUNDS PER SQUARE FOOT
EF	EACH FACE	PSI	POUNDS PER SQUARE INCH
EJ	EXPANSION JOINT	PT	POINT
EL	ELEVATION	PVC	POLYVINYL CHLORIDE
ELEC	ELECTRICAL		
ENGR	ENGINEER	QTY	QUANTITY
EOD	EDGE OF DECK		
EOG	EDGE OF GRATING	R	RADIUS
EOR	ENGINEER OF RECORD	RCP	REINFORCED CONCRETE PIPE
EOS	EDGE OF SLAB	RD	ROOF DRAIN
EQ	EQUAL	REF	REFERENCE
EQUIP	EQUIPMENT	REINF	REINFORCING
EW	EACH WAY	REQD	REQUIRED
EXP	EXPANSION	RET	RETURN
EXT	EXTERIOR	REV	REVISION
		RO	ROUGH OPENING
FB	FLAT BAR		
FD	FLOOR DRAIN	SC	SLIP CRITICAL
FDN	FOUNDATION	SCHED	SCHEDULE
FF	FINISHED FLOOR	SECT	SECTION
FFE	FINISHED FLOOR ELEVATION	SHT	SHEET
FG	FINISHED GRADE	SIB	STRUCTURAL ISOLATION BREAK
FIN	FINISH	SIM	SIMILAR
FLG	FLANGE	SIP	STRUCTURAL INSULATED PANEL
FLR	FLOOR	SJI	STEEL JOIST INSTITUTE
FND	FOUNDATION	SL	SLOPE
FRMG	FRAMING	SPCS	SPACES
FRP	FIBER REINFORCED PLASTIC	SPEC(S)	SPECIFICATION(S)
FS	FAR SIDE	SQ	SQUARE
FT	FOOT	SR	SAG ROD
FTG	FOOTING	SS	STAINLESS STEEL
		SSH	SHORT SLOTTED HOLE
GA	GAGE, GAUGE	STD	STANDARD
GALV	GALVANIZED (HOT DIP)	STIF	STIFFENER
GL	GIRT LINE	STL	STEEL
GR	GUARDRAIL	STRUCT	STRUCTURAL
GRTG	GRATING	SYM	SYMMETRICAL
(H)	HORIZONTAL BEAM ORIENTATION	T/	TOP OF
HCA	HEADED CONCRETE ANCHOR	T&B	TOP AND BOTTOM
HDR	HEADER	T&G	TONGUE AND GROOVE
HGR	HANGER	THK	THICK
HORIZ	HORIZONTAL	THRU	THROUGH
HP	HIGH POINT	TJ	TRUSS JOIST
HR	HANDRAIL	TP	TOE PLATE
HSB	HIGH STRENGTH BOLT	TRANS	TRANSVERSE
		TYP	TYPICAL
IF	INSIDE FACE		
IBC	INTERNATIONAL BUILDING CODE	UL	UNDERWRITERS LABORATORIES
ICC	INTERNATIONAL CODE COUNCIL	UNO	UNLESS NOTED OTHERWISE
ICF	INSULATED CONCRETE FORM		
ID	INSIDE DIAMETER	VERT	VERTICAL
IN	INCH	VIF	VERIFY IN FIELD
INT	INTERIOR		
		W/	WITH
JST	JOIST	W/O	WITHOUT
JT	JOINT	WCJ	WALL CONTROL JOINT
		WP	WORKING POINT
K	KIP(S)	WS	WATERSTOP
KB	KNEE BRACE	WWR	WELDED WIRE REINFORCEMENT
KSF	KIPS PER SQUARE FOOT		
KSI	KIPS PER SQUARE INCH	YD	YARD

SHOP DRAWINGS & SUBMITTALS

1. THE GENERAL CONTRACTOR WILL REVIEW AND STAMP ALL SHOP DRAWINGS AND PRODUCT DATA SUBMITTALS FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS PRIOR TO SUBMISSION. ANY SHOP DRAWINGS OF PRODUCT DATA NOT REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR WILL BE RETURNED WITHOUT REVIEW.
2. ANY SHOP DRAWINGS NOT CHECKED AND INITIALED BY THE SUPPLIER/DETAILER PRIOR TO SUBMITTING FOR ARCHITECTURAL AND ENGINEERING REVIEW, WILL BE RETURNED WITHOUT REVIEW.
3. THE CEC CONSTRUCTION DOCUMENTS MAY NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS, UNLESS APPROVED BY THE ENGINEER OF RECORD.
4. ELECTRONIC FILES OF CONSTRUCTION DOCUMENTS WILL NOT BE MADE AVAILABLE FOR USE AS SHOP DRAWINGS.
5. REFER TO INDIVIDUAL NOTES SECTIONS FOR ADDITIONAL SUBMITTAL REQUIREMENTS.

STRUCTURAL CONCRETE

1. CONCRETE WORK SHALL FOLLOW THE PRACTICES AND STANDARDS SPECIFIED IN SECTION **CODES AND STANDARDS**.
2. CONCRETE PROPERTIES:
   A. ALL CONCRETE SHALL CONFORM TO ASTM C94
   B. ALL AGGREGATES SHALL CONFORM TO ASTM C33 (NORMAL–WEIGHT) & ACI 302.1
   C. ALL CONCRETE SHALL BE CURED IN CONFORMANCE WITH ACI 301
   D. ALL CONCRETE FORMWORK SHALL CONFORM WITH ACI 347
3. ADDITION OF WATER TO THE BATCH FOR MATERIAL WITH INSUFFICIENT SLUMP WILL NOT BE PERMITTED, UNLESS THE SUPPLIER HAS SPECIFICALLY WITHHELD WATER FROM THE BATCH AT THE PLANT. IN SUCH CASE, THE MIX DESIGN AND TRUCK TICKET MUST CLEARLY STATE THE MAXIMUM AMOUNT OF WATER THAT CAN BE ADDED TO THE BATCH ON SITE. IN NO CASE SHALL THE DESIGN WATER TO CEMENTITIOUS MATERIAL RATIO BE EXCEEDED.
4. AGGREGATES DETERMINED TO BE ALKALI–CARBONATE REACTIVE (ACR), IN ACCORDANCE WITH ASTM C1778, ARE NOT PERMITTED.
5. CONTRACTOR TO SUBMIT ALL PROPOSED CONCRETE ADMIXTURES TO EOR FOR APPROVAL.
6. CONTRACTOR AND READI–MIX SUPPLIER TO SELECT A TARGET SLUMP BASED ON ASTM C143/C143M AS PERMITTED UNDER ACI 301 FOR THE POINT OF DELIVERY FOR CONCRETE MIXTURES USED FOR THIS PROJECT. SELECTED TARGET SLUMP SHALL NOT EXCEED 9 INCHES. CONCRETE SHALL NOT SHOW VISIBLE SIGNS OF SEGREGATION. THE TARGET SLUMP INDICATED ON THE SUBMITTAL SHALL BE USED AS THE BASIS FOR ACCEPTANCE DURING THE PROJECT. SLUMP TOLERANCES SHALL BE IN ACCORDANCE WITH ACI 117.
7. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED TO BE VIBRATED ONLY AROUND UNDER–FLOOR DUCTS, SLAB EDGES, REINFORCING, KEYS, ETC. MECHANICALLY VIBRATE ONLY THE TOP 5 FEET OF DRILLED PIER CONCRETE. REVIBRATE TOP OF DRILLED PIER 15 MINUTES AFTER PLACING CONCRETE.
8. VERTICAL CONCRETE SURFACES ARE TO BE FORMED.
9. ALL EXPOSED CONCRETE CORNERS (BOTH VERTICAL & HORIZONTAL) SHALL HAVE A 3/4" CHAMFER OR ROUNDED TOOLED EDGE.
10. ALL CONCRETE SHALL CURE A MINIMUM OF 7 DAYS AFTER POURING, AND SHALL REMAIN UNLOADED UNTIL CONCRETE HAS REACHED 80% OF THE 28 DAY DESIGN STRENGTH, UNLESS APPROVED BY THE ENGINEER OF RECORD.
11. UNLESS OTHERWISE SPECIFIED, CONCRETE TEMPERATURE AS DELIVERED SHALL NOT EXCEED 95 DEGREES FAHRENHEIT NOR BE LESS THAN 55 DEGREES FAHRENHEIT FOR SECTION SIZE <12", 50 DEGREES FAHRENHEIT FOR 12" – 36". TEMPERATURE OF CONCRETE AS PLACED SHALL NOT EXCEED THESE VALUES BY MORE THAN 20 DEGREES FAHRENHEIT.
12. ALKALI–SILICA REACTION – COMPLY WITH ONE OF THE FOLLOWING:
   A. EXPANSION RESULT OF AGGREGATE: NOT MORE THAN 0.04 PERCENT AT ONE–YEAR WHEN TESTED IN ACCORDANCE WITH ASTM 1293.
   B. EXPANSION RESULTS OF AGGREGATE AND CEMENTITIOUS MATERIALS IN COMBINATION: NOT MORE THAN 0.10 PERCENT AT AN AGE OF 16 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C1567.
   C. ALKALI CONTENT IN CONCRETE: NOT MORE THAN 4 LB/YD³ FOR MODERATELY REACTIVE AGGREGATE OR 3 LB/YD³ FOR HIGHLY REACTIVE AGGREGATE, WHEN TESTED IN ACCORDANCE WITH ASTM C1293 AND CATEGORIZED IN ACCORDANCE WITH ASTM C1778, BASED ON ALKALI CONTENT BEING CALCULATED IN ACCORDANCE WITH ACI 301 (ACI 301M).

CONCRETE MIX REQUIREMENTS

CONCRETE TYPE	28 DAY COMPRESSIVE STRENGTH (f'_c, PSI)	AIR CONTENT (+/– 1.5%)	MAX W/C RATIO	MAX NOM AGG SIZE (IN)	CEMENT TYPE	FLYASH % (OF TOTAL CEMENT)	EXPOSURE CLASSES
ALL CONCRETE (UNO)	4,500	6.0%	0.45	3/4	I/II	10–20	F2, S0, W1, C1

REINFORCING STEEL

1. REINFORCING STEEL SHALL FOLLOW THE PRACTICES AND STANDARDS SPECIFIED IN SECTION **CODES AND STANDARDS**.
2. TYPICAL REINFORCING BAR STRENGTHS
   A. REINFORCING (NON–WELDABLE) — ASTM A615, DEFORMED, Fy= 60 KSI (420 MPa)
   B. REINFORCING (WELDABLE) — ASTM A706, DEFORMED, Fy= 60 KSI (420 MPa)
3. CONCRETE COVER FOR REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 OR ACI 350, UNLESS OTHERWISE NOTED.
4. DETAILING, FABRICATION, SUPPORT, AND PLACING OF ALL BARS SHALL BE PER CRSI SPECIFICATIONS/HANDBOOKS AND THE LATEST ACI CODES AND DETAILING MANUALS.
5. ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONCRETE. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE.
6. REINFORCING BAR SPACINGS GIVEN ARE MAXIMUM ON CENTERS.
7. REINFORCING BAR HOOKS AND BENDS SHALL BE ACI STANDARD DIMENSIONS, UNLESS NOTED OTHERWISE.
8. REINFORCING BARS SHALL NOT BE BENT OR STRAIGHTENED IN A MANNER THAT WILL DAMAGE THE MATERIAL. BARS WITH KINKS OR IMPROPER BENDS SHALL NOT BE USED. NO BARS PARTIALLY EMBEDDED IN CONCRETE SHALL BE FIELD BENT, EXCEPT AS SHOWN ON THE DRAWINGS OR AS PERMITTED BY THE ENGINEER.
9. NO REINFORCING STEEL SHALL BE FIELD BENT WITHOUT THE APPROVAL OF THE ENGINEER OF RECORD. FIELD BENDING OF PLAIN REINFORCEMENT, IF PERMITTED, SHALL BE PERFORMED USING AN APPROVED AND APPROPRIATE SIZED PORTABLE HYDRAULIC DEVICE THAT MAKES ACI STANDARD RADIUS BENDS. NO OTHER FIELD BENDING METHOD SHALL BE PERMITTED.
10. REINFORCING LAP SPLICES IN CONCRETE SHALL BE PER TYPICAL DETAILS UNLESS NOTED OTHERWISE. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF FOOTINGS AND WALLS.
11. ALL SPLICING OF REINFORCING BARS SHALL BE DONE BY LAPPING UNLESS OTHERWISE APPROVED. BARS SPLICED BY NON CONTACT LAP SPLICES IN FLEXURAL MEMBERS SHALL NOT BE SPACED TRANSVERSELY FARTHER APART THAN ONE–FIFTH THE REQUIRED SPLICE LENGTH NOR 6 INCHES.
12. LAP SPLICE LENGTHS FOR DEFORMED BARS SHALL BE ACI 318 CLASS B TENSION LAP SPLICES UNLESS OTHERWISE NOTED ON DESIGN DRAWINGS.
13. LAP SPLICES FOR ADJACENT BARS SHALL BE STAGGERED. STAGGERED SPLICE REQUIREMENTS DO NOT APPLY TO BEAMS AND COLUMNS.
14. ALL SPLICES NOT DETAILED ON DESIGN DRAWINGS SHALL BE TAKEN AS TENSION LAP SPLICES.
15. DIMENSIONS GIVEN FOR BENT OR HOOKED BARS ARE OUT TO OUT.

REINFORCING CLEAR COVER TABLE

TYPE	CONCRETE CLEAR COVER (IN)
PERMANENTLY EXPOSED TO OR CAST AGAINST EARTH	3
EXPOSED TO EARTH OR WEATHER (#5 OR SMALLER)	1 1/2
EXPOSED TO EARTH OR WEATHER (#6 OR LARGER)	2
NOT EXPOSED TO EARTH OR WEATHER (#11 OR SMALLER)	1
NOT EXPOSED TO EARTH OR WEATHER (#12 OR LARGER)	1 1/2

CONCRETE DEVELOPMENT/LAP SPLICE SCHEDULE ($f'c$ = 4.5 KSI)

BAR SIZE	DEVELOPMENT LENGTH (IN)	LAP SPLICE LENGTH, CLASS B (IN)
	BAR TYPE 1*	BAR TYPE 1*
4	18	24
5	23	30
6	27	36
7	40	52
8	45	59
9	51	67

CLEAR SPACING OF BARS BEING DEVELOPED OR LAP SPLICED NOT LESS THAN Db, CONCRETE COVER NOT LESS THAN Db, AND STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN THE CODE MINIMUM.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL SHALL FOLLOW THE PRACTICES AND STANDARDS SPECIFIED IN SECTION **CODES AND STANDARDS**.
2. ALL STRUCTURAL STEEL SHALL BE FABRICATED BY A FABRICATOR WITH ANY ONE OF THE FOLLOWING MINIMUM QUALIFICATIONS. QUALIFICATIONS SHALL BE IN EFFECT AT TIME OF BID.
   A. INTERNATIONAL ACCREDITATION SERVICE, INC. (IAS) APPROVED FABRICATOR.
   B. AISC CERTIFIED FABRICATOR (STD).
   C. FABRICATOR SHALL SUBMIT DOCUMENTATION OF THEIR CERTIFICATION WITH THE FIRST SHOP DRAWING SUBMITTAL.
3. UNLESS OTHERWISE NOTED, THE SHOP DRAWING REVIEW TIME WILL BE IN ACCORDANCE WITH SECTION 4.4 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. THE WORD "APPROVED" IN SECTION 4.4 IS REDEFINED AS "REVIEWED".
4. STEEL PROPERTIES (UNLESS NOTED OTHERWISE):
   A. WIDE FLANGE COLUMNS, BEAMS, CHANNELS AND TEES: ASTM A992 (FY = 50 KSI)
5. PROVIDE POSITIVE CAMBER AS NOTED ON DRAWINGS. WHERE NO CAMBER IS NOTED, RESIDUAL MILL CAMBER IS TO BE UPWARDS.
6. STANDARD AISC "USUAL GAGE" DIMENSIONS SHALL BE USED FOR LOCATING HOLES FOR BOLTS, EXPANSION ANCHORS, ETC. IN ALL ANGLES, BEAM FLANGES, ETC., UNLESS NOTED OTHERWISE.
7. WELDS SHALL BE MADE WITH E–70XX ELECTRODES. MINIMUM FILLET WELD SIZE SHALL BE 3/16" OR IN ACCORDANCE WITH AISC MANUAL TABLE J2.8, WHICHEVER IS GREATER. USE PREQUALIFIED WELDED JOINTS PER AISC AND THE AMERICAN WELDING SOCIETY (AWS). NON–PREQUALIFIED JOINTS SHALL BE SUBMITTED FOR APPROVAL PRIOR TO FABRICATION.
8. EVERY STRUCTURAL PIECE IS TO INCLUDE IDENTIFICATION AS FOLLOWS:
   A. UNIQUE PERMANENT PIECE IDENTIFICATION MARKING SUCH AS PUNCH OR WELD – ON TWO FACES.
   B. UNIQUE PIECE IDENTIFICATION 2D BARCODED LABEL – ON TWO FACES.
   C. IDENTIFIER OF PIECE ORIENTATION TO ASSIST STEEL ERECTOR.
9. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT–DIPPED GALVANIZED AND PAINTED. WELDS FOR EXTERIOR STEEL EXPOSED TO WEATHER SHALL BE CONTINUOUS TO SEAL CONTACT SURFACES AND SHALL BE SEAL WELDED WHERE STRUCTURAL WELDS ARE NOT SPECIFIED.
10. WHERE STEEL IS GALVANIZED, ALL WELDED JOINTS ARE TO BE SEAL WELDED CONTINUOUSLY. SEAL WELD BETWEEN INTERMEDIATE WELDS SHOWN AND BETWEEN WELDED CONNECTION ANGLES AT BEAM ENDS.
11. SUBSTITUTION OF MATERIAL SIZE OR SHAPE IS PROHIBITED WITHOUT THE APPROVAL OF THE ENGINEER OF RECORD.
12. THE STEEL ERECTOR SHALL CORRECT MINOR MISALIGNMENTS BY APPLYING MODERATE AMOUNTS OF REAMING, CHIPPING, OR CUTTING AND BY DRAWING MEMBERS INTO LINE WITH THE USE OF DRIFT PINS. MISALIGNMENTS WHICH CANNOT BE CORRECTED BY THE FORGOING MEANS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD.
13. TEMPORARY SUPPORTS SUCH AS BRACES, GUYS, FALSE WORK, AND CRIBBING SHALL BE DESIGNED, SUPPLIED, AND INSTALLED BY THE STEEL ERECTOR TO ENSURE A SAFE ERECTION OPERATION.
14. ANY DISCREPANCIES OR INTERFERENCES FOUND BY THE STRUCTURAL STEEL DETAILER SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD PRIOR TO DETAILING STEEL.

REVISION RECORD

NO	DATE	DESCRIPTION
1	08/21/26	REV 1 - FOUNDATION CHANGE TO SHALLOW FOOTING

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STATEMENT OF SPECIAL INSPECTIONS

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT, AS WELL AS THE NAME OF THE SPECIAL INSPECTOR TO BE BE RETAINED FOR CONDUCTING THESE INSPECTIONS AND TESTS. THIS STATEMENT OF SPECIAL INSPECTIONS ENCOMPASSES THE FOLLOWING DISCIPLINES:

☐ ARCHITECTURAL ☒ STRUCTURAL
☐ MECHANICAL/ELECTRICAL/PLUMBING ☐ OTHER:_____

THE SPECIAL INSPECTOR SHALL KEEP A RECORD OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITIES.

INTERIM REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING THE COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS, TESTING, AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. 1

THIS STATEMENT OF SPECIAL INSPECTIONS INCLUDES THE FOLLOWING BUILDING SYSTEMS:

☐ ARCHITECTURAL	☒ SOILS	☒ CAST–IN–PLACE FOUNDATION ELEMENTS
☐ DRIVEN DEEP FOUNDATION ELEMENTS	☐ HELICAL PILE FOUNDATIONS	☐ CAST–IN–PLACE DEEP FOUNDATION ELEMENTS
☒ CONCRETE CONSTRUCTION	☐ MASONRY CONSTRUCTION – LEVEL A	☐ MASONRY CONSTRUCTION – LEVEL B
☐ STRUCTURAL STEEL CONSTRUCTION	☐ STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	
☐ WOOD CONSTRUCTION	☐ SPRAY FIRE–RESISTANT MATERIALS	☐ MASTIC AND INTUMESCENT FIRE–RESISTANT COATINGS
☐ EXTERIOR INSULATION AND FINISH SYSTEMS (EIFS)	☐ FIRE–RESISTANT PENETRATIONS AND JOINTS	☐ SMOKE CONTROL
☐ WIND RESISTANCE	☐ SEISMIC RESISTANCE	

THE FOLLOWING COMPONENTS ARE WIND–RESISTING COMPONENTS OR PART OF THE MAIN WIND–FORCE RESISTING SYSTEM AND ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH THE SPECIAL INSPECTION SCHEDULE:

THE FOLLOWING COMPONENTS ARE DESIGNATED SEISMIC SYSTEMS OR PART OF THE SEISMIC–FORCE RESISTING SYSTEM THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH THE SPECIAL INSPECTION SCHEDULE:

SPECIAL INSPECTION AGENCY:

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION (IBC 2018 TABLE 1705.3)

	VERIFICATION AND INSPECTION	REQUIRED	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	YES	–	X	ACI 318: CH 20, 25.2, 25.3, 26.6.1–26.6.3	1908.4
2.	REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706; b. INSPECT SINGLE–PASS FILLET WELDS, MAXIMUM 5/16"; AND c. INSPECT ALL OTHER WELDS	YES	– X	X X	AWS D1.4 ACI 318: 26.6.4	–
3.	INSPECT ANCHORS CAST IN CONCRETE.	NO	–	X	ACI 318: 17.8.2	–
4.	INSPECT ANCHORS POST–INSTALLED IN HARDENED CONCRETE MEMBERS.° a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	YES	X	 X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	–
5.	VERIFYING USE OF REQUIRED DESIGN MIX.	YES	–	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6.	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	YES	X	–	ASTM C172; ASTM C31; ACI 318: 26.4.5, 26.12	1908.10
7.	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	YES	X	–	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8.	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	YES	–	X	ACI 318: 26.4.7–26.4.9	1908.9
9.	INSPECT PRESTRESSED CONCRETE: a. APPLICATION OF PRESTRESSING FORCES. b. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE–RESISTING SYSTEM.	NO	X X	– –	ACI 318: 26.10	–
10.	INSPECT ERECTION OF PRECAST CONCRETE MEMBERS	NO	–	X	ACI 318: Ch. 26.9	–
11.	VERIFY IN–SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST–TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	NO	–	X	ACI 318: 26.11.2	–
12.	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	YES	–	X	ACI 318: 26.11.1.2(b)	–

° SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.

REQUIRED VERIFICATION AND INSPECTION OF SOILS (IBC 2018 TABLE 1705.6)

	VERIFICATION AND INSPECTION	REQUIRED	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	YES	–	X	–	–
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	YES	–	X	–	–
3.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	YES	–	X	–	–
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	YES	X	–	–	–
5.	PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	YES	–	X	–	–

REQUIRED VERIFICATION AND INSPECTION OF CAST-IN-PLACE FOUNDATION ELEMENTS (IBC 2018 SECTION 1705.3)

	VERIFICATION AND INSPECTION	REQUIRED	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE FOUNDATION CONSTRUCTION IN ACCORDANCE WITH THE SPECIAL INSPECTION SCHEDULE: CAST–IN–PLACE CONCRETE FOR THE FOLLOWING FOUNDATION ELEMENTS:					
	A. ISOLATED SPREAD CONCRETE FOOTINGS	YES	–	–	–	–
	B. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS	YES	–	–	–	–
	C. CONCRETE FOUNDATION WALLS	YES	–	–	–	–

1

REVISION RECORD

NO	DATE	DESCRIPTION
1	08/21/26	REV 1 - FOUNDATION CHANGE TO SHALLOW FOOTING

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Civil & Environmental Consultants, Inc.

WEST TENNESSEE
RIVER BASIN AUTHORITY URBAN
STREAM RESTORATION
CENTRAL CREEK
JACKSON, TENNESSEE

STRUCTURAL SPECIAL INSPECTIONS

DATE:	APRIL 2026
DRAWN BY:	SCM
DWG SCALE:	NTS
CHECKED BY:	RTJ/GRG
PROJECT NO:	343-075
APPROVED BY:	BCK

DRAWING NO.:
S101
SHEET 2 OF 4

ELIZABETH CRISCO-KERR
REGISTERED ENGINEER
No. 116005
STATE OF TENNESSEE
8/21/26

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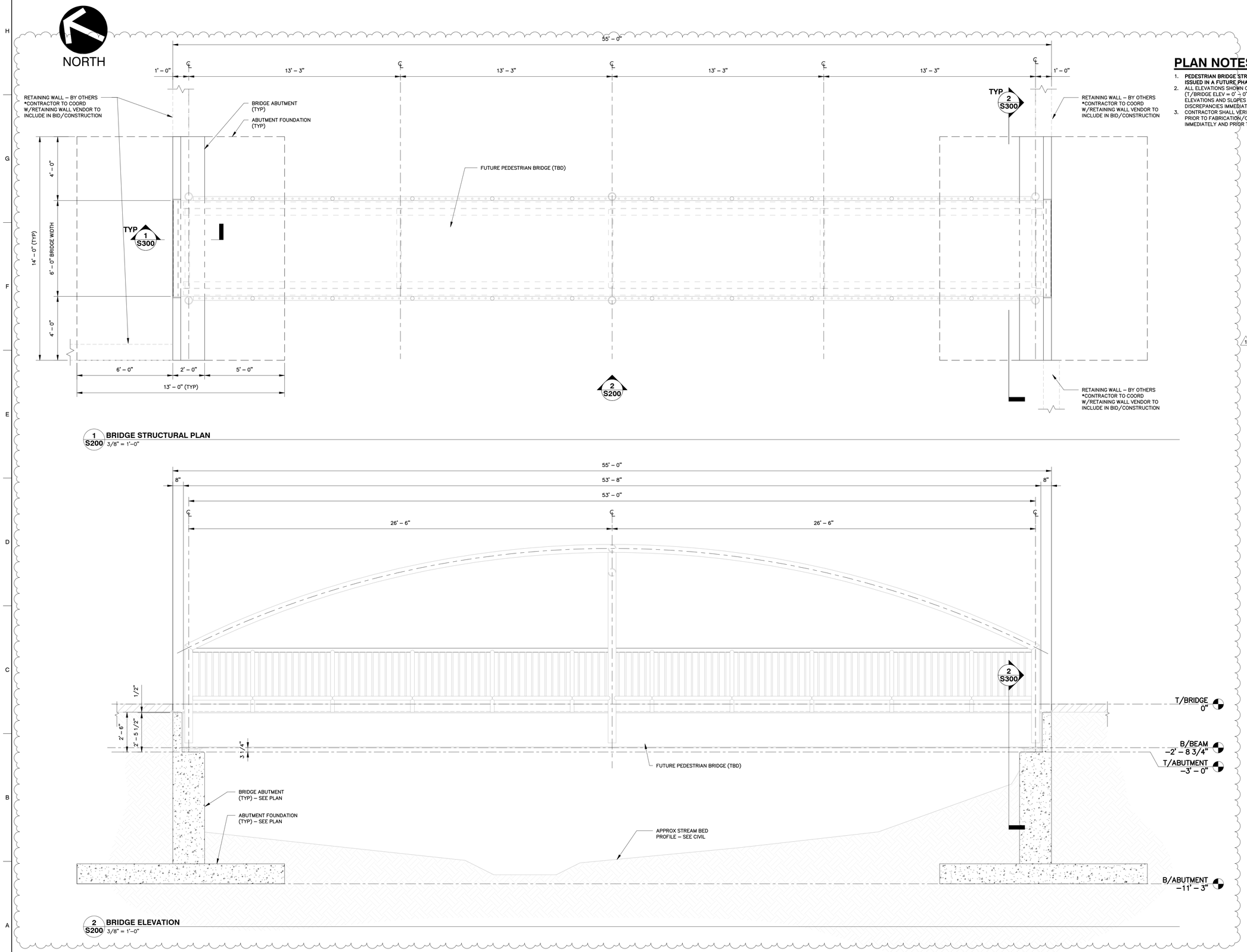

PLAN NOTES

1. **PEDESTRIAN BRIDGE STRUCTURE SHOWN WILL BE DESIGNED AND ISSUED IN A FUTURE PHASE.**
2. ALL ELEVATIONS SHOWN ON PLAN ARE RELATIVE TO THE TOP OF BRIDGE (T/BRIDGE ELEV = 0' – 0"). CONTRACTOR SHALL COORDINATE ACTUAL ELEVATIONS AND SLOPES WITH CIVIL DRAWINGS. NOTIFY EOR OF DISCREPANCIES IMMEDIATELY AND PRIOR TO CONSTRUCTION.
3. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY EOR OF DISCREPANCIES IMMEDIATELY AND PRIOR TO FABRICATION/CONSTRUCTION.

REVISION RECORD

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WEST TENNESSEE
RIVER BASIN AUTHORITY URBAN STREAM RESTORATION
CENTRAL CREEK
JACKSON, TENNESSEE

STRUCTURAL PLAN & ELEVATION

DATE:	APRIL 2026	DRAWN BY:	SCM
DWG SCALE:	3/8" = 1'-0"	CHECKED BY:	RTJ/GRG
PROJECT NO:			343-075
APPROVED BY:			BCK

ELIZABETH CRISCO-KEITNER
REGISTERED ENGINEER
AGRICULTURE
COMMERCE
No. 116695
STATE OF TENNESSEE
8/21/26

DRAWING NO.: **S200**

SHEET 3 OF 4

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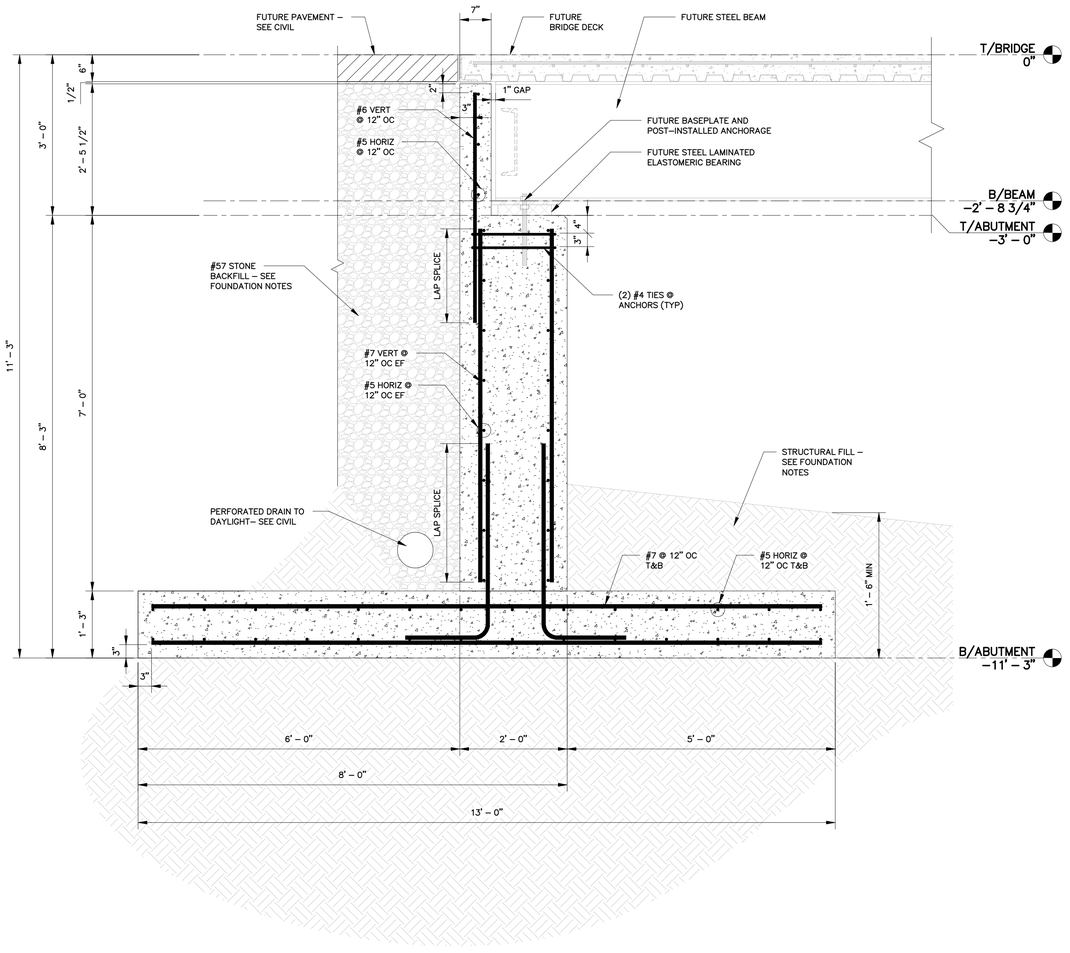
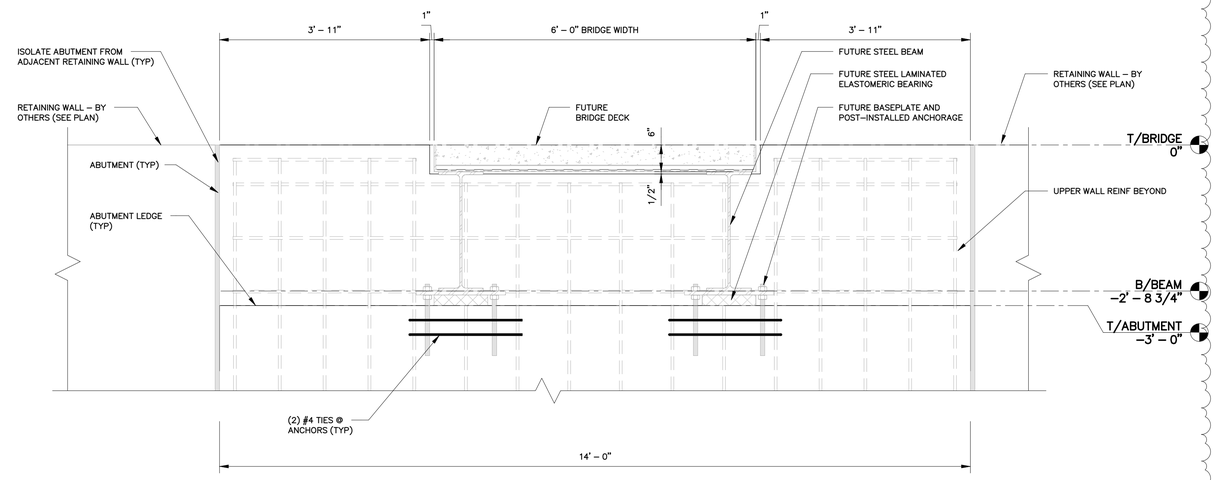


1 S300 REVISED ABUTMENT SECTION
3/4" = 1'–0"



2 S300 REVISED ABUTMENT ELEVATION
3/4" = 1'–0"

REVISION RECORD

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RIVER BASIN AUTHORITY URBAN STREAM RESTORATION
CENTRAL CREEK
JACKSON, TENNESSEE

STRUCTURAL SECTIONS AND DETAILS

DATE:	APRIL 2026	DRAWN BY:	SCM
DWG SCALE:	AS SHOWN	CHECKED BY:	RTJ/GRG
PROJECT NO:			343-075
APPROVED BY:			BCK

ELIZABETH CRISCO-KEATNER
REGISTERED ENGINEER
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No. 116695
STATE OF TENNESSEE
8/21/26

DRAWING NO.: S300
SHEET 4 OF 4

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