

# Bio-Tiger Model Training

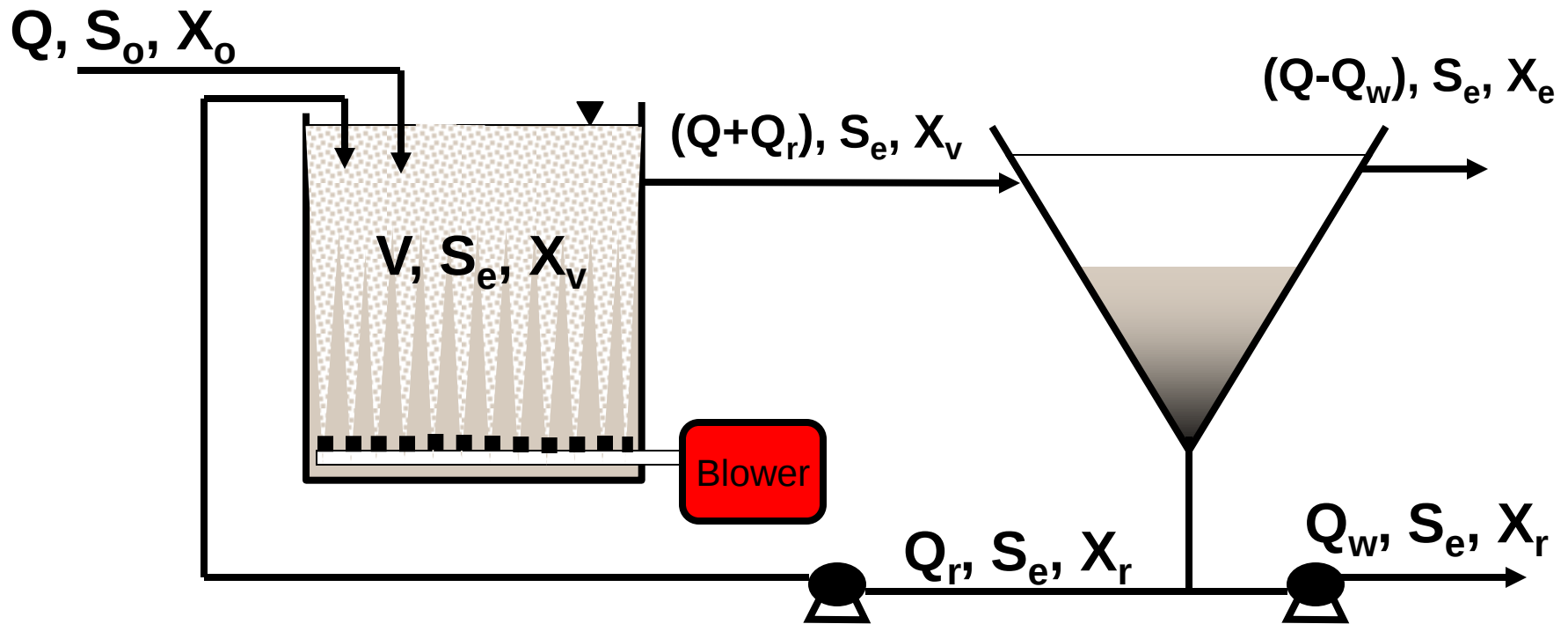
## Introduction to Activated Sludge Biokinetics and to Dr. Moore's Model

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Fleming Training Center

# **Predicting Performance of the Activated Sludge Process Using Biokinetic Relationships**

# Activated Sludge: Basics of Design



Activated Sludge



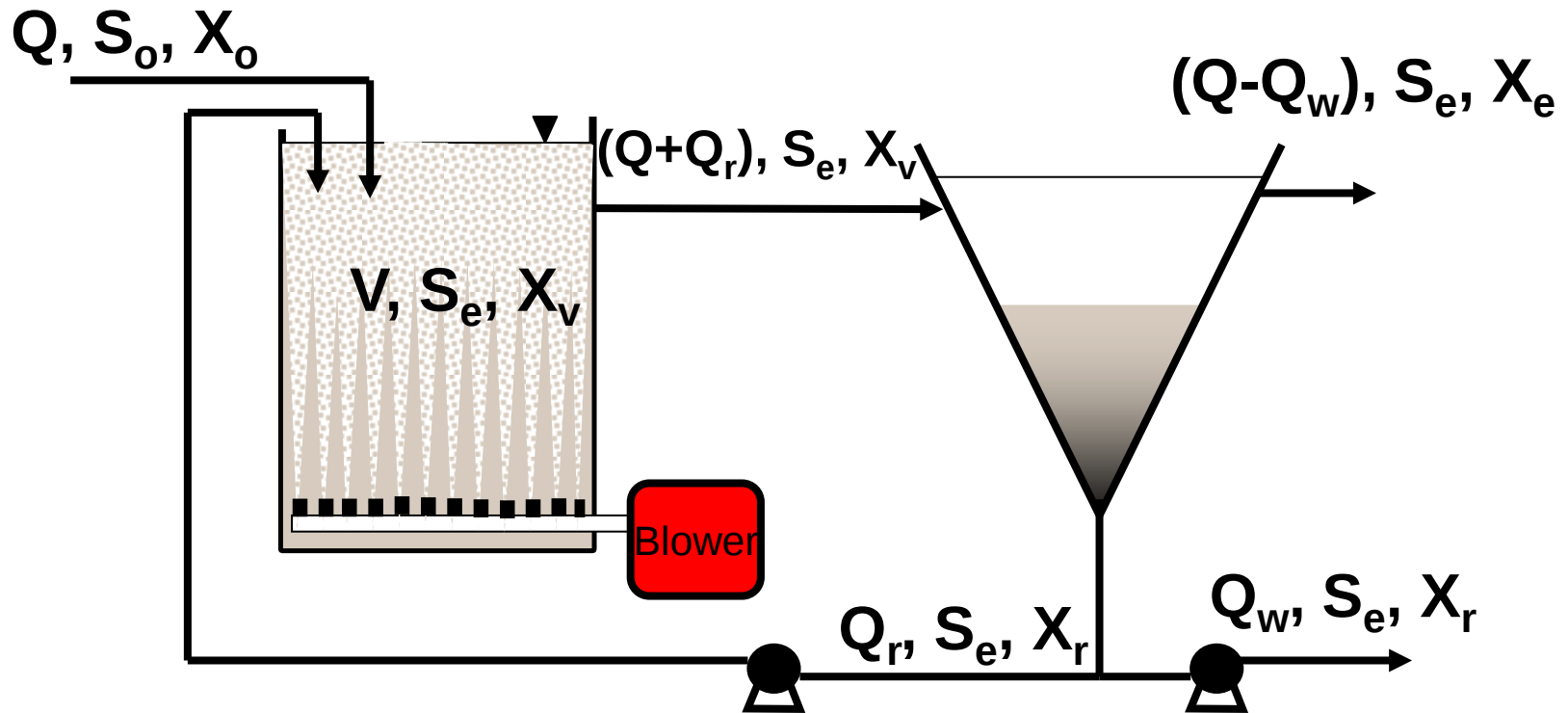
## Biological Reactor with Aerated Mixed Liquor (diffused aeration)

# Remember:

$$\theta_c = \text{MCRT} = \text{SRT} = \text{sludge age}$$

It is how long in days (on average) the biomass stays in the activated sludge system until the biomass exits the system as waste activated sludge solids or as TSS in the effluent.

# Determining $\theta_c$ Using Plant Data



$$\theta_c = \frac{XV}{Q_w X_r + (Q - Q_w) X_e} \approx \frac{XV}{Q_w X_r}$$

Activated Sludge

# The Activated Sludge Process

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$$\mu = \frac{1}{\theta_c} + k_e$$

$\theta_c$  = mean cell residence time  
or sludge age

$\mu$  = specific growth rate of  
biomass

# Activated Sludge Design

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$\mu_{\max}$  = maximum specific growth rate

$K_s$  = saturation constant

$k_e$  = microbial decay coefficient ( $k_e = k_d$ )

$Y$  = biomass yield constant

$k$  = maximum specific substrate utilization rate

$\mu_{\max} = Yk$

# Determining $S_e$ Using Biokinetic Approach

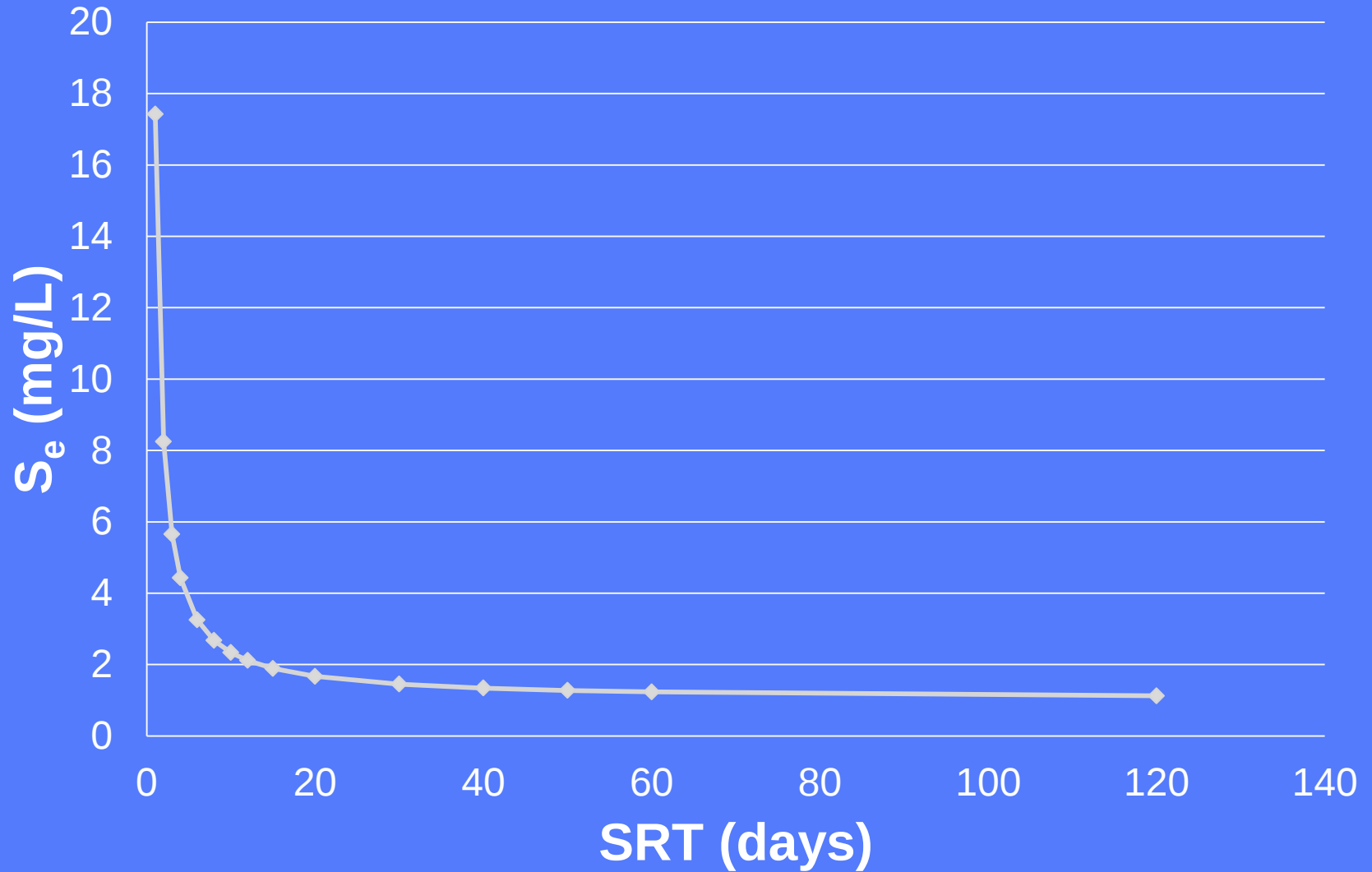
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$$S_e = \frac{K_s(1 + k_e \theta_c)}{\theta_c(\mu_{\max} - k_e) - 1}$$

$S_e$  does not include  $\text{CBOD}_5$  contributed by solids.

This equation is only valid for *Monod kinetics*.

## $S_e$ versus SRT



# Activated Sludge Design

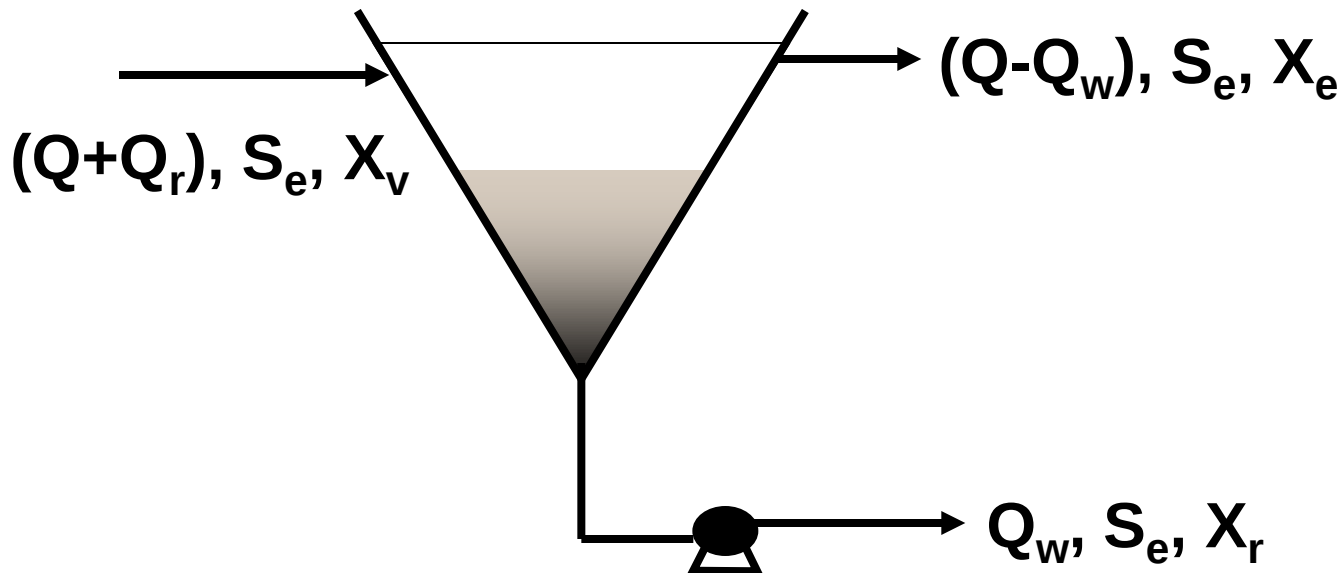
Step 1: Determine effluent requirement

$$\text{CBOD}_{5\text{eff}} = S_e + f X_e$$

where  $X_e$  = TSS in final effluent

$S_e$  = soluble  $\text{CBOD}_5$

$f$  = g  $\text{CBOD}_5$ /g TSS = 0.3 to 0.6



Activated Sludge

# Activated Sludge Design

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## Step 2: Determine kinetic coefficients

| <u>Coef.</u> | <u>Range and units</u>          | <u>Typical Value</u>     |
|--------------|---------------------------------|--------------------------|
| $\mu_{m/20}$ | 2 to 10 day <sup>-1</sup>       | 5.0 day <sup>-1</sup>    |
| $K_s$        | 25 to 100 mg/l BOD <sub>5</sub> | 60 mg/L                  |
| $k_e$        | 0.05 to 0.15 day <sup>-1</sup>  | 0.08 day <sup>-1</sup>   |
| $Y_{x/s}$    | 0.4 to 0.8 VSS/BOD <sub>5</sub> | 0.6 VSS/BOD <sub>5</sub> |

# Activated Sludge Design

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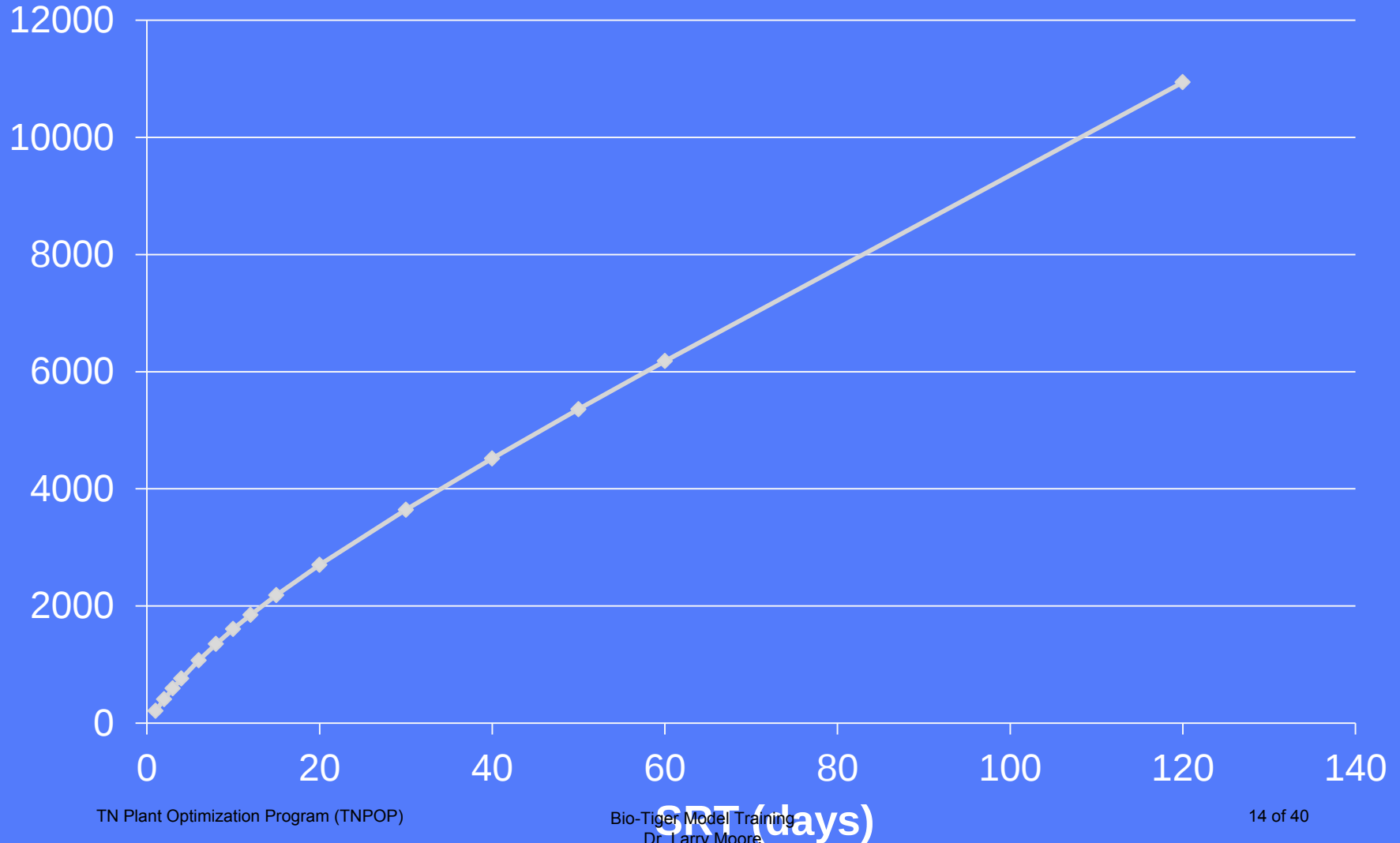
**Step 3:** Select MLVSS concentration in aeration basin.

$X_v = 1500$  to  $3000$  mg/L for complete mix  
 $1500$  to  $4000$  mg/L for extended air

**Step 4:** Determine aeration basin volume

$$V = \frac{QY_{x/s}\theta_c(S_o - S_e)}{X_v(1 + k_e\theta_c)}$$

# MLSS versus SRT – 1.0 mgd Extended Aeration Act. Sludge



# Activated Sludge Design

**Step 5:** Determine the mass of volatile solids to be wasted ( $P_{XVSS}$ )

$$P_{XVSS} = A + B + C$$

$$A + B = \text{biomass production} = \text{VSW}$$

A = heterotrophic biomass

B = cell debris

C = nonbiodegradable VSS in influent

$$P_{XVSS} = \frac{QY(S_o - S_e)}{1 + k_d\theta_c} + \frac{f_d(k_d)YQ(S_o - S_e)\theta_c}{1 + k_d\theta_c} + QX_{oi}$$

# Activated Sludge Design

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**Step 6:** Determine the mass of **total** solids to be wasted (Px)

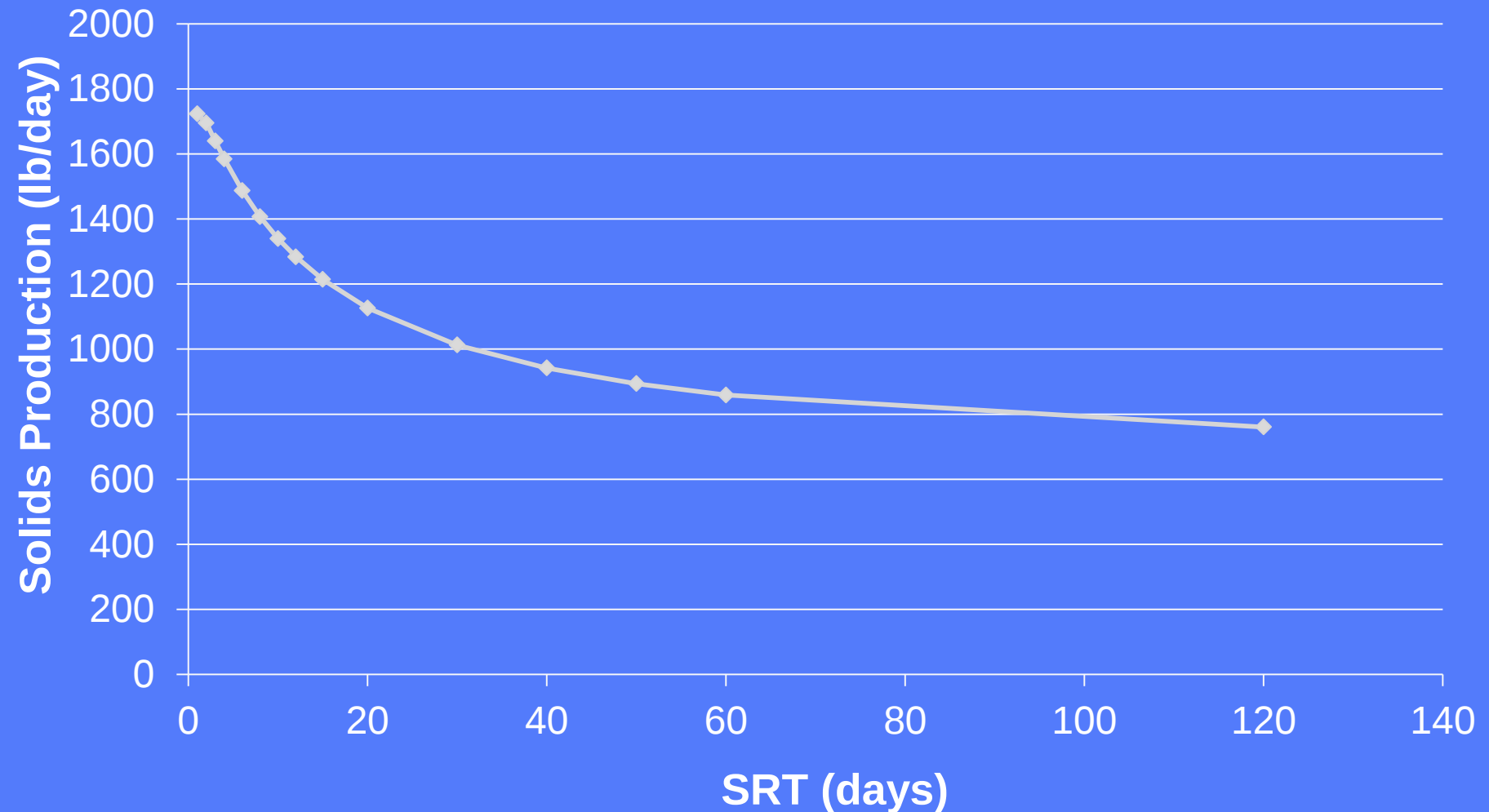
$$P_{XTSS} = A/0.85 + B/0.85 + C + Q(TSS_o - VSS_o)$$

where  $P_{XTSS}$  = net waste activated sludge produced each day, mass/day

$TSS_o$  = influent TSS concentration

$VSS_o$  = influent VSS concentration

# Sludge Production (TSS, lb/d) vs SRT - 1.0 mgd Extended Aeration Act. Sludge



# Activated Sludge Design

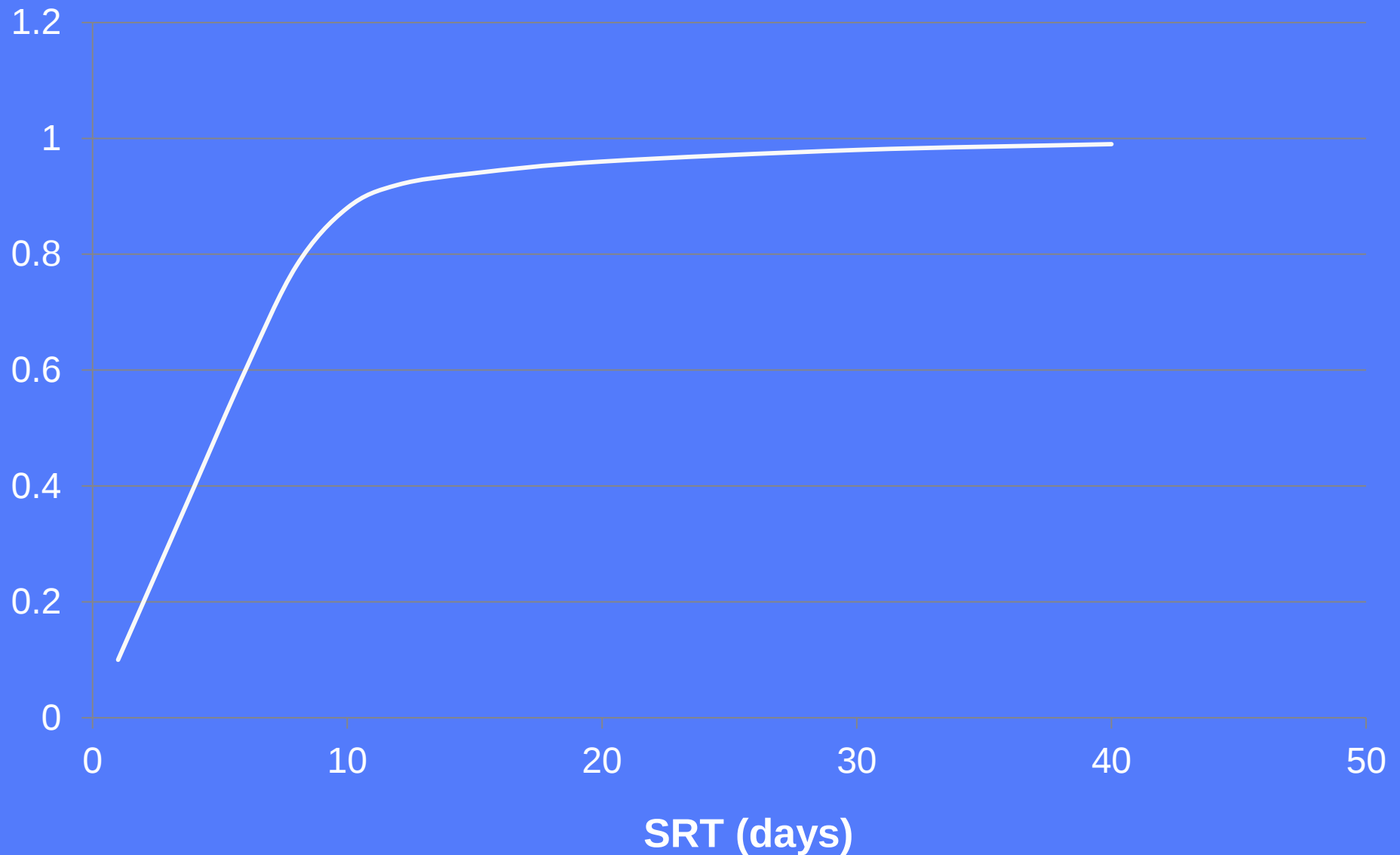
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**Step 7:** Determine the oxygen requirements (CBOD and NBOD)

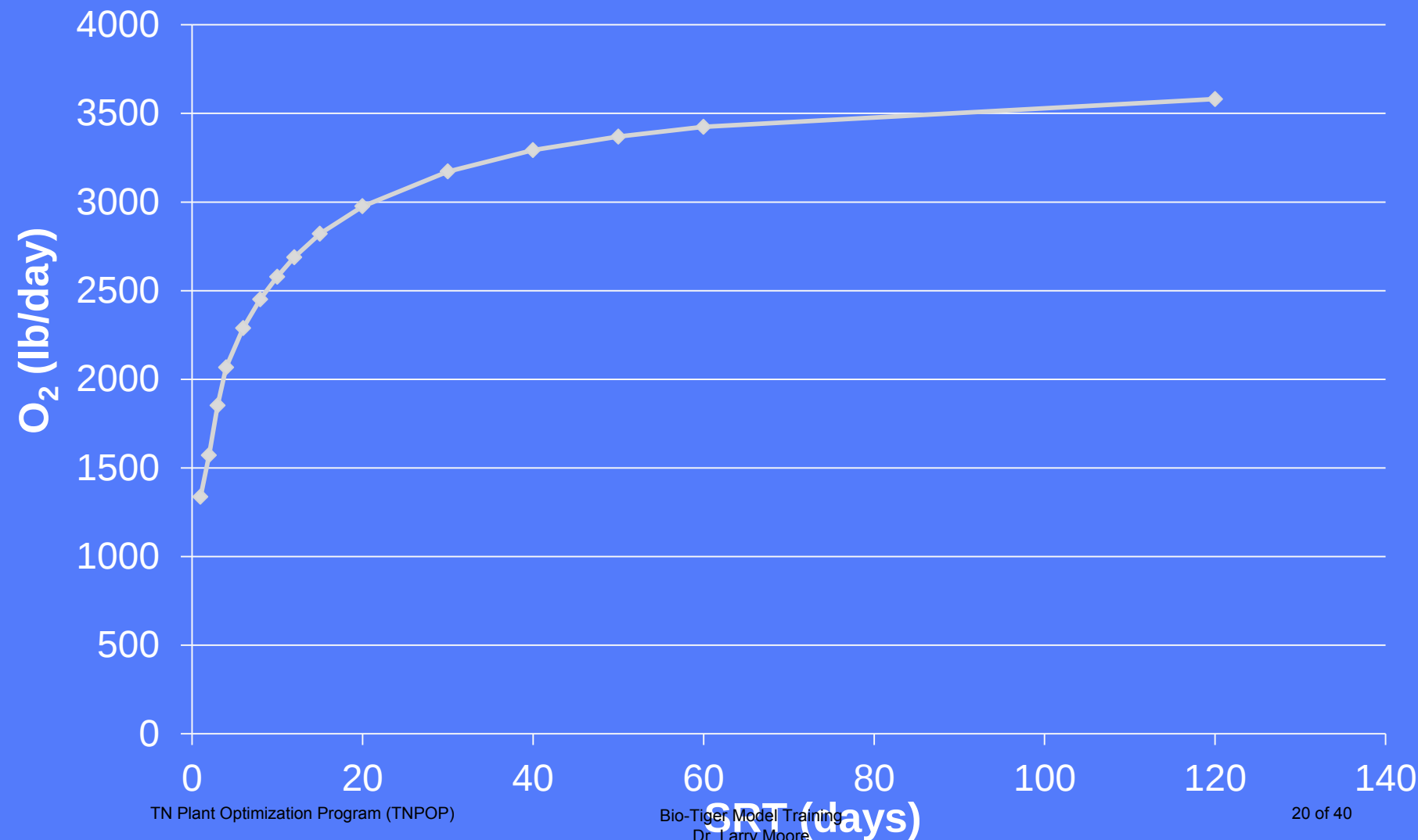
$$\begin{aligned} \text{O}_2(\text{lb/day}) = & 8.34Q \left[ \frac{S_o - S_e}{0.67} \right] - 1.42(\text{VSW}) \\ & + 4.33(\text{N}_{\text{ox}})(Q)(8.34) \end{aligned}$$

\*Note: VSW = biomass production = A + B in previous equations  
1.42(VSW) = ultimate CBOD that goes to cell growth

# Fraction $N_{ox}$ Oxidized at 20°C



# Oxygen Required (Carb+Nit) vs SRT – 1.0 mgd Extended Aeration Act. Sludge



# Understanding the Oxygen Transfer Capability of Various Types of Aeration Equipment

# Typical Standard O<sub>2</sub> Transfer Rates

- Pump type aerators
  - 2.3 to 3.3 lb O<sub>2</sub>/(HP-hr)
- Aspirating aerators
  - 1.7 to 2.3 lb O<sub>2</sub>/(HP-hr)
- Horizontal rotor aerators
  - 2.3 to 3.3 lb O<sub>2</sub>/(HP-hr)
- Nonporous diffusers
  - 1.8 to 2.5 lb O<sub>2</sub>/(HP-hr)
- Porous diffusers
  - 2.8 to 3.8 lb O<sub>2</sub>/(HP-hr)

# Standard Conditions for Mechanical Aerators

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**Elevation = sea level**

**Temperature = 20°C**

**Initial DO concentration = zero mg/L**

**Tap water**

# Determine Field O<sub>2</sub> Transfer Rate for Mechanical Aerators

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$$\text{OTR} = \text{OTR}_{\text{standard}} \alpha \frac{(\beta \rho C_s - C)}{9.2} 1.024^{(T - 20)}$$

OTR<sub>standard</sub> = oxygen transfer rate at 20°C (lb O<sub>2</sub> hp<sup>-1</sup> hour<sup>-1</sup>),  
1 atm, tap water, and initial DO = zero mg/L

C = dissolved oxygen level in basin (typically 1.5 to 2 mg/L)

C<sub>s</sub> = saturated dissolved oxygen level in mg/L

α = (K<sub>L</sub>a of wastewater)/(K<sub>L</sub>a of tap water); use α = 0.80 to 0.90  
unless specified otherwise.

β = C<sub>s</sub> wastewater/C<sub>s</sub> tap water = 0.92 for municipal wastewater

ρ = factor that corrects for elevation differences

# Approximate Field O<sub>2</sub> Transfer Rates

- Pump type aerators
  - 1.4 to 2.0 lb O<sub>2</sub>/(HP-hr)
- Aspirating aerators
  - 1.0 to 1.4 lb O<sub>2</sub>/(HP-hr)
- Horizontal rotor aerators
  - 1.4 to 2.0 lb O<sub>2</sub>/(HP-hr)

**$\alpha = 0.84$ ,  $\beta = 0.92$ ,  $\rho = 1$ , DO = 2 mg/L,  
Elevation < 500 ft**

# **Additional Assumptions for Diffused Aeration**

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**Compressor efficiency = 75%**

**Tank depth = 15 ft**

**Diffusers located 1.5 ft above tank bottom**

# **Approximate Field O<sub>2</sub> Transfer Rates**

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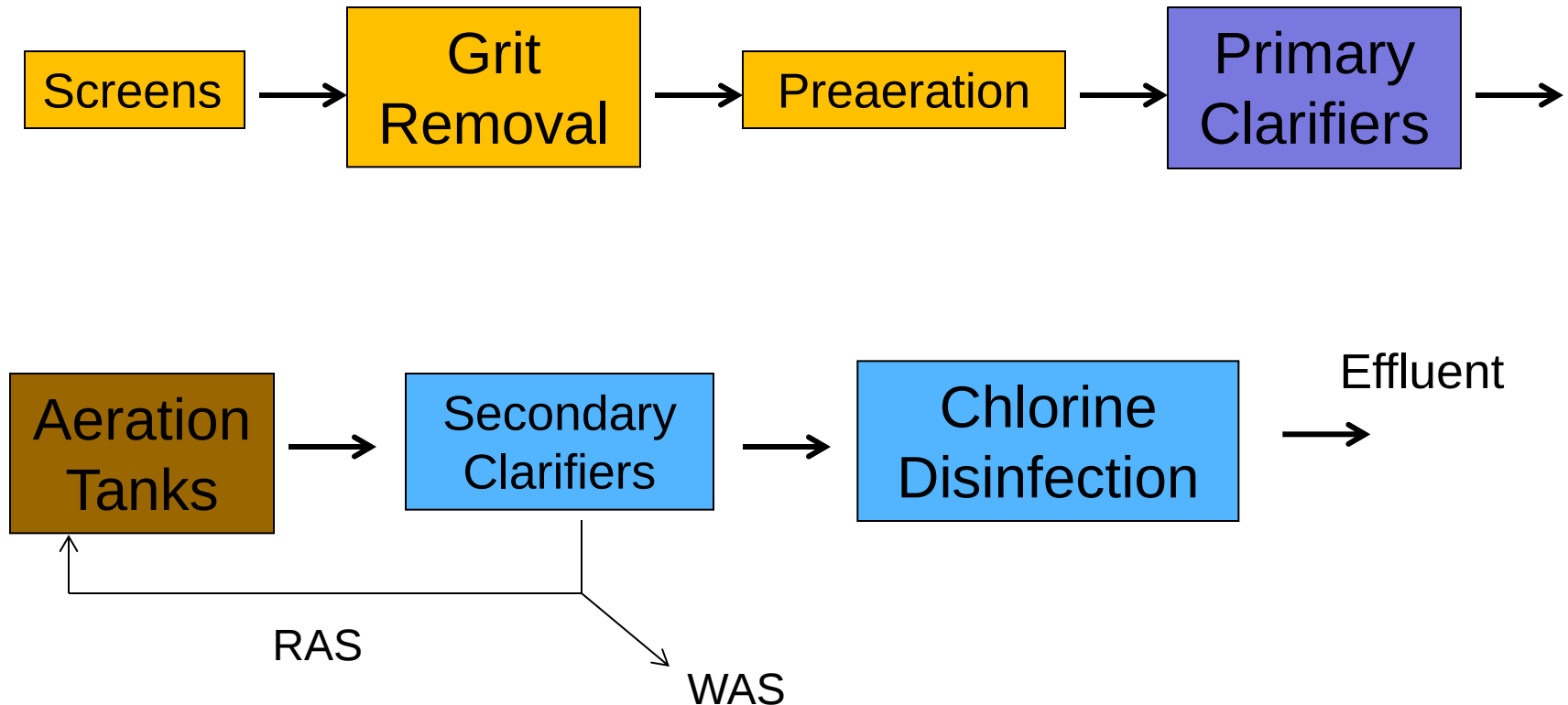
- Nonporous diffusers
  - 1.1 to 1.5 lb O<sub>2</sub>/(HP-hr)
- Porous diffusers
  - 1.7 to 2.3 lb O<sub>2</sub>/(HP-hr)

**$\alpha = 0.84$ ,  $\beta = 0.92$ ,  $\rho = 1$ , DO = 2 mg/L**

**Elevation < 500 ft, Compressor efficiency = 75%**

**Tank depth = 15 ft, Diffusers located 1.5 ft above tank bottom**

# Case Study: Fort Rucker WWTP



\*Aeration basin diffusers are flexible-membrane, fine-bubble diffusers.

# Design Parameters for Fort Rucker WWTP

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**Flow rate = 2.5 mgd (ave. daily)**

**CBOD<sub>5</sub> = 250 mg/L (ave)**

**TSS = 350 mg/L (ave)**

**TKN = 45 mg/L (ave)**

**Ammonia-N = 25 mg/L (ave)**

# Summer NPDES Limits for Fort Rucker WWTP

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**CBOD<sub>5</sub> = 12 mg/L (mo. ave)**

**TSS = 30 mg/L (mo. ave)**

**Ammonia-N = 3 mg/L (mo. ave)**

# Existing Conditions: Fort Rucker WWTP

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|                                              |                                  |
|----------------------------------------------|----------------------------------|
| Total average daily flow rate                | 0.58 mgd (half to each aer tank) |
| Aeration volume in service                   | 0.66 mil gal (0.33 mil gal each) |
| Sec. influent BOD <sub>5</sub> concentration | 90 mg/L                          |
| Sec. influent BOD <sub>5</sub> mass loading  | 435 lb/day (total)               |
| Biomass inventory (MLVSS)                    | 14,300 lb (in aeration tanks)    |

**\*See Bio-Tiger Model for details of this case study!**

# Existing Conditions: Fort Rucker WWTP

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|                          |                                           |
|--------------------------|-------------------------------------------|
| Biomass inventory (MLSS) | 19,300 lb (in aeration tanks)             |
| F/M ratio                | 0.031 lb BOD <sub>5</sub> /(lb MLVSS-day) |
| Solids Retention Time    | 160 days                                  |
| MLSS                     | 3500 mg/L                                 |
| MLVSS                    | 2600 mg/L                                 |



# Existing Conditions: Fort Rucker WWTP

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|                                                                |                                                                 |
|----------------------------------------------------------------|-----------------------------------------------------------------|
| TSS Sludge Production                                          | 99 lb/day (intentional wastage)<br>1050 lb/day (primary solids) |
| TSS in activated sludge effluent                               | 19 lb/day (unintentional wastage)                               |
| Oxygen Requirements (actual)                                   | 1300 lb/day                                                     |
| Oxygen required for aerobic digestion<br>= 2.3 x VSS destroyed | 900 lb/day                                                      |

**One 60-hp PD blower runs 22 hrs/day for aeration basin**

**One 75-hp PD blower runs 24 hrs/day for aerobic digesters**

# Existing Conditions: Fort Rucker WWTP

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|                                                   |                     |
|---------------------------------------------------|---------------------|
| <b>Total Oxygen Supplied by aer. basin blower</b> | <b>1,400 lb/day</b> |
| Mixing intensity in aeration tanks with 60 hp     | 91 hp/mil gal       |
| DO in aeration basins                             | 4.5 mg/L            |
| RAS flow rate                                     | 0.66 mgd (total)    |
| WAS flow rate                                     | 0.0018 mgd          |
| RAS TSS concentration                             | 6500 mg/L           |

# New Conditions: Use Only One Aer. Basin and Run the Blower Only 16 Hours/Day

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|                                              |                                |
|----------------------------------------------|--------------------------------|
| Total average daily flow rate                | 0.58 mgd (all to one aer tank) |
| Aeration volume in service                   | 0.33 mil gal (one basin)       |
| Sec. influent BOD <sub>5</sub> concentration | 90 mg/L                        |
| Sec. influent BOD <sub>5</sub> mass loading  | 435 lb/day (total)             |
| Biomass inventory (MLVSS)                    | 7,200 lb (in aeration tank)    |

# New Conditions: Use Only One Aer. Basin and Run the Blower Only 16 Hours/Day

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|                          |                                           |
|--------------------------|-------------------------------------------|
| Biomass inventory (MLSS) | 9,600 lb (in aeration tanks)              |
| F/M ratio                | 0.060 lb BOD <sub>5</sub> /(lb MLVSS-day) |
| Solids Retention Time    | 70 days                                   |
| MLSS                     | 3500 mg/L                                 |
| MLVSS                    | 2600 mg/L                                 |

# New Conditions: Use Only One Aer. Basin and Run the Blower Only 16 Hours/Day

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|                                                                |                                   |
|----------------------------------------------------------------|-----------------------------------|
| TSS Sludge Production                                          | 117 lb/day (intentional wastage)  |
| TSS in activated sludge effluent                               | 19 lb/day (unintentional wastage) |
| Oxygen Requirements (actual)                                   | 1270 lb/day                       |
| Oxygen required for aerobic digestion<br>= 2.3 x VSS destroyed | 900 lb/day                        |

**One 60-hp PD blower runs 16 hrs/day for aeration basin**

**One 75-hp PD blower runs 12 hrs/day for aerobic digesters**

# New Conditions: Use Only One Aer. Basin and Run the Blower Only 16 Hours/Day

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|                                                   |                     |
|---------------------------------------------------|---------------------|
| <b>Total Oxygen Supplied by aer. basin blower</b> | <b>1,200 lb/day</b> |
| Mixing intensity in aeration tanks with 60 hp     | 182 hp/mil gal      |
| DO in aeration basins                             | 4 mg/L              |
| RAS flow rate                                     | 0.67 mgd (total)    |
| WAS flow rate                                     | 0.0022 mgd          |
| RAS TSS concentration                             | 6500 mg/L           |

# Fort Rucker WWTP Estimated Energy Savings

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Energy savings for aeration basin blower:

$$\begin{aligned} &= 60 \text{ hp} \times 6 \text{ hr/day} \times 30 \text{ day/mo} \times 0.75 \text{ kWh/hp-hr} \times 0.85 \\ &\approx 6,900 \text{ kWh per month} \end{aligned}$$

Energy savings for aerobic digester blower:

$$\begin{aligned} &= 75 \text{ hp} \times 12 \text{ hr/day} \times 30 \text{ day/mo} \times 0.75 \text{ kWh/hp-hr} \times 0.85 \\ &\approx 17,200 \text{ kWh per month} \end{aligned}$$

$$\begin{aligned} \text{Energy cost savings} &= 24,000 \text{ kWh per month} \times \$0.053/\text{kWh} \\ &= \$1,270 \text{ per month (20\% savings)} \end{aligned}$$